APPENDIX M

Engineering Reports

M.1 | SIGNATURE EAST – ENGINEER'S REPORT

M.2 | SIGNATURE EAST – SITE DRAWINGS

M.3 | SIGNATURE SOUTH – ENGINEER'S REPORT

M.4 | SIGNATURE SOUTH – SITE DRAWINGS

APPENDIX M.3

Signature South – Engineer's Report

APPENDIX D
Post-Development Quantity Hydrographs

Project	**Teterboro Airport - South Hanger**		**By:** **AM**	**Date:**	**7/29/2025**
Location	**Teterboro, NJ**		**Checked By:** **LM**	**Date Checked:**	**7/29/2025**
Circle one:	**Present**	(Developed)	**Proposed Pervious Watershed A2**		

Include a map, schematic, or description of coverage areas.

Runoff Curve Number (CN)

Soil Name and hydrologic group (Appendix A)	Cover description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN*2 Table 2-2	CN*2 Fig. 2-3	CN*2 Fig. 2-4	Area [x] acres / [ ] mi^2 / [ ] %	Product of CN x Area
D	**Impervious**	**98**			**1.15**	**113.06**
			Totals =		**1.15**	**113.06**

CN (weighted) = $\frac{\text{total product}}{\text{total area}} = \frac{113.06}{1.15} =$ 98.00 Use CN = **98**

Instructions:
1. Update project information and dates for date created and date checked/reviewed.
2. Use only one CN and area per source line.

Project	**Teterboro Airport - South Hanger**		**By:** **AM**	**Date:**	**7/29/2025**
Location	**Teterboro, NJ**		**Checked By:** **LM**	**Date Checked:**	**7/29/2025**
Circle one:	**Present**	(Developed)	**Proposed Impervious Watershed A3**		

Include a map, schematic, or description of coverage areas.

Runoff Curve Number (CN)

Soil Name and hydrologic group (Appendix A)	Cover description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN*2 Table 2-2	CN*2 Fig. 2-3	CN*2 Fig. 2-4	Area [x] acres / [ ] mi^2 / [ ] %	Product of CN x Area
D	**Impervious**	**98**			**2.34**	**229.04**
			Totals =		**2.34**	**229.04**

CN (weighted) = $\frac{\text{total product}}{\text{total area}} = \frac{229.04}{2.34} =$ 98.00 Use CN = **98**

Instructions:
1. Update project information and dates for date created and date checked/reviewed.
2. Use only one CN and area per source line.

Project	**Teterboro Airport - South Hanger**		**By:** **AM**	**Date:**	**7/29/2025**
Location	**Teterboro, NJ**		**Checked By:** **LM**	**Date Checked:**	**7/29/2025**
Circle one:	**Present**	(Developed)	**Proposed Impervious Watershed A4**		

Include a map, schematic, or description of coverage areas.

Runoff Curve Number (CN)

Soil Name and hydrologic group (Appendix A)	Cover description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN*2			Area	Product of CN x Area
		Table 2-2	Fig. 2-3	Fig. 2-4	[x] acres [ ] mi^2 [ ] %	
D	**Impervious**	**98**			**2.11**	**206.78**
				Totals =	**2.11**	**206.78**

CN (weighted) = $\frac{\text{total product}}{\text{total area}} = \frac{206.78}{2.11} =$ 98.00 Use CN = **98**

Instructions:
1. Update project information and dates for date created and date checked/reviewed.
2. Use only one CN and area per source line.

Project	**Teterboro Airport - South Hanger**		**By:** **AM**	**Date:**	**7/29/2025**
Location	**Teterboro, NJ**		**Checked By:** **LM**	**Date Checked:**	**7/29/2025**
Circle one:	**Present**	(Developed)	**Proposed Impervious Watershed A5**		

Include a map, schematic, or description of coverage areas.

Runoff Curve Number (CN)

Soil Name and hydrologic group (Appendix A)	Cover description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN*2 Table 2-2	CN*2 Fig. 2-3	CN*2 Fig. 2-4	Area [x] acres / [ ] mi^2 / [ ] %	Product of CN x Area
D	**Impervious**	**98**			**0.77**	**75.84**
			Totals =		**0.77**	**75.84**

CN (weighted) = $\frac{\text{total product}}{\text{total area}} = \frac{75.84}{0.77} =$ 98.00 Use CN = **98**

Instructions:
1. Update project information and dates for date created and date checked/reviewed.
2. Use only one CN and area per source line.

Project	**Teterboro Airport - South Hanger**		**By: AM**	**Date:**	**7/29/2025**
Location	**Teterboro, NJ**		**Checked By: LM**	**Date Checked:**	**7/29/2025**
Circle one:	**Present**	(Developed)	**Proposed Pervious Watershed A5**		

Include a map, schematic, or description of coverage areas.

Runoff Curve Number (CN)

Soil Name and hydrologic group (Appendix A)	Cover description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN*2 Table 2-2	CN*2 Fig. 2-3	CN*2 Fig. 2-4	Area [x] acres / [ ] mi^2 / [ ] %	Product of CN x Area
D	**Open Space, Good Cond.**	**80**			**0.05**	**3.90**
			Totals =		**0.05**	**3.90**

CN (weighted) = $\frac{\text{total product}}{\text{total area}} = \frac{3.90}{0.05} =$ 80.00 Use CN = **80**

Instructions:
1. Update project information and dates for date created and date checked/reviewed.
2. Use only one CN and area per source line.

Project	**Teterboro Airport - South Hanger**		**By:** **AM**	**Date:**	**7/29/2025**
Location	**Teterboro, NJ**		**Checked By:** **LM**	**Date Checked:**	**7/29/2025**
Circle one:	**Present**	(Developed)	**Proposed Impervious Watershed A6**		

Include a map, schematic, or description of coverage areas.

Runoff Curve Number (CN)

Soil Name and hydrologic group (Appendix A)	Cover description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN*2 Table 2-2	CN*2 Fig. 2-3	CN*2 Fig. 2-4	Area [x] acres / [ ] mi^2 / [ ] %	Product of CN x Area
D	**Impervious**	**98**			**0.41**	**40.22**
			Totals =		**0.41**	**40.22**

CN (weighted) = $\frac{\text{total product}}{\text{total area}} = \frac{40.22}{0.41} =$ 98.00 Use CN = **98**

Instructions:
1. Update project information and dates for date created and date checked/reviewed.
2. Use only one CN and area per source line.

Project Teterboro Airport - South Hanger **By:** AM **Date:** 7/29/2025

Location Teterboro, NJ **Checked By:** LM **Date Checked:** 7/29/2025

Circle one: **Present** (Developed) **Proposed Pervious Watershed A6**

Include a map, schematic, or description of coverage areas.

Runoff Curve Number (CN)

Soil Name and hydrologic group (Appendix A)	Cover description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN*2 Table 2-2	CN*2 Fig. 2-3	CN*2 Fig. 2-4	Area [x] acres / [ ] mi^2 / [ ] %	Product of CN x Area
D	**Open Space, Good Cond.**	**80**			**0.17**	**13.60**
				Totals =	**0.17**	**13.60**

CN (weighted) = $\frac{\text{total product}}{\text{total area}} = \frac{13.60}{0.17} =$ 80.00 Use CN = **80**

Instructions:
1. Update project information and dates for date created and date checked/reviewed.
2. Use only one CN and area per source line.

Project	**Teterboro Airport - South Hanger**		**By:** **AM**	**Date:**	**7/29/2025**
Location	**Teterboro, NJ**		**Checked By:** **LM**	**Date Checked:**	**7/29/2025**
Circle one:	**Present**	(Developed)	**Proposed Impervious Watershed A7**		

Include a map, schematic, or description of coverage areas.

Runoff Curve Number (CN)

Soil Name and hydrologic group (Appendix A)	Cover description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN*2			Area	Product of CN x Area
		Table 2-2	Fig. 2-3	Fig. 2-4	[x] acres [ ] mi^2 [ ] %	
D	**Impervious**	**98**			**1.67**	**163.42**
				Totals =	**1.67**	**163.42**

CN (weighted) = $\frac{\text{total product}}{\text{total area}} = \frac{163.42}{1.67} =$ 98.00 Use CN = **98**

Instructions:
1. Update project information and dates for date created and date checked/reviewed.
2. Use only one CN and area per source line.

Project Teterboro Airport - South Hanger **By:** AM **Date:** 7/29/2025

Location Teterboro, NJ **Checked By:** LM **Date Checked:** 7/29/2025

Circle one: **Present** (Developed) **Proposed Pervious Watershed B**

Include a map, schematic, or description of coverage areas.

Runoff Curve Number (CN)

Soil Name and hydrologic group (Appendix A)	Cover description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN*2 Table 2-2	CN*2 Fig. 2-3	CN*2 Fig. 2-4	Area [x] acres / [ ] mi^2 / [ ] %	Product of CN x Area
D	**Open Space, Good Cond.**	**80**			**1.97**	**157.48**
D	**Brush, Good Cond.**	**73**			**2.84**	**207.00**
D	**Woods, Good Cond.**	**77**			**3.98**	**306.48**
D	**Streets, Gravel**	**91**			**0.71**	**65.01**
				Totals =	**9.50**	**735.97**

CN (weighted) = $\frac{\text{total product}}{\text{total area}} = \frac{735.97}{9.50} =$ 77.48 Use CN = **77**

Instructions:
1. Update project information and dates for date created and date checked/reviewed.
2. Use only one CN and area per source line.

Project Teterboro Airport - South Hanger **By:** AM **Date:** 7/29/2025

Location Teterboro, NJ **Checked By:** LM **Date Checked:** 7/29/2025

Circle one: **Present** (Developed) **Proposed Impervious Watershed B**

Include a map, schematic, or description of coverage areas.

Runoff Curve Number (CN)

Soil Name and hydrologic group (Appendix A)	Cover description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN*2			Area	Product of CN x Area
		Table 2-2	Fig. 2-3	Fig. 2-4	[x] acres [ ] mi^2 [ ] %	
C	**Impervious**	**98**			**1.60**	**156.73**
				Totals =	**1.60**	**156.73**

CN (weighted) = $\frac{\text{total product}}{\text{total area}} = \frac{156.73}{1.60} =$ 98.00 Use CN = **98**

Instructions:
1. Update project information and dates for date created and date checked/reviewed.
2. Use only one CN and area per source line.

Project	**Teterboro Airport - South Hanger**		**By:** **AM**	**Date:**	**7/29/2025**
Location	**Teterboro, NJ**		**Checked By:** **LM**	**Date Checked:**	**7/29/2025**
Circle one:	**Present**	(Developed)	**Proposed Pervious Watershed C**		

Include a map, schematic, or description of coverage areas.

Runoff Curve Number (CN)

Soil Name and hydrologic group (Appendix A)	Cover description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN*2			Area	Product of CN x Area
		Table 2-2	Fig. 2-3	Fig. 2-4	[x] acres [ ] mi^2 [ ] %	
D	**Open Space, Good Cond.**	**80**			**0.05**	**3.92**
				Totals =	**0.05**	**3.92**

CN (weighted) = $\frac{\text{total product}}{\text{total area}} = \frac{3.92}{0.05} =$ 80.00 Use CN = **80**

Instructions:
1. Update project information and dates for date created and date checked/reviewed.
2. Use only one CN and area per source line.

Current Rainfall

Project:	Teterboro Airport - South Hanger	**By:**	AM	**Date:**	7/29/2025
Location:	Teterboro, NJ	**Checked By:**	LM	**Date Checked:**	7/29/2025

Circle One: Present (**Developed**) — **Watershed A2**

Circle One: (**T_c**) T_t **through subarea**

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)

			Segment ID			
1.	Surface Description (NEH table 15-1)			1		
				Smooth Surfaces		
2.	Manning's Roughness Coeff., n (NEH table 15-1)			0.011		
3.	Design Flow Length, L_{des} (Total L_{des} < 100 ft)		ft	100		
4.	Two-year 24-hr rainfall, P_2		in	3.35		
5.	Land Slope, s		ft/ft	0.013		
6.	McCuen-Spiess Limit, L_{mcs} (max. 100 ft)	$L_{mcs} = (100\ s^{0.5})/n$	ft	100		
7.	Compute T_t	$T_t = \frac{0.007\ (nL_{des})^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.024	+	+
					Sheet Flow Sub-Total	0.024 hours

Shallow Concentrated Flow

			Segment ID			
8.	Surface Description			2		
				Pavement		
9.	Flow Length, L		ft	113		
10.	Watercourse Slope, s		ft/ft	0.005		
11.	Average Velocity, V (NEH table 15-3)		ft/s	1.39		
12.	Compute T_t	$T_t = \frac{L}{3600\ V}$	hr	0.023	+	+
					Shallow Conc. Flow Sub-Total	0.023 hours

Channel Flow

			Segment ID			
13.	Flow Length, L		ft			
14.	Average Velocity, V		ft/s			
15.	Compute T_t	$T_t = \frac{L}{3600\ V}$	hr		+	+
					Channel Flow Sub-Total	0.000 hours

Watershed or subarea T_c or T_t
(Add Sub-Total T_t from prior steps)

Total Tc (hours) =	0.046 hours
Total Tc (minutes) =	3 minutes

Current Rainfall

Project:	Teterboro Airport - South Hanger	**By:**	AM	**Date:**	7/29/2025
Location:	Teterboro, NJ	**Checked By:**	LM	**Date Checked:**	7/29/2025

Circle One: Present **(Developed)** — **Watershed A3**

Circle One: **(T_c)** T_t **through subarea**

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)		Segment ID	1		
1.	Surface Description (NEH table 15-1)		Smooth Surfaces		
2.	Manning's Roughness Coeff., n (NEH table 15-1)		0.011		
3.	Design Flow Length, L_{des} (L_{des} < 100 ft)	ft	100		
4.	Two-year 24-hr rainfall, P_2	in	3.35		
5.	Land Slope, s	ft/ft	0.010		
6.	McCuen-Spiess Limit, L_{mcs} (max. 100 ft) $L_{mcs} = (100\ s^{0.5})/n$	ft	100		
7.	Compute T_t $T_t = \frac{0.007\ (nL_{des})^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.026	+	+
				Sheet Flow Sub-Total	0.026 hours

Shallow Concentrated Flow		Segment ID	2		
8.	Surface Description		Pavement		
9.	Flow Length, L	ft	182		
10.	Watercourse Slope, s	ft/ft	0.012		
11.	Average Velocity, V (NEH table 15-3)	ft/s	2.21		
12.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr	0.023	+	+
				Shallow Conc. Flow Sub-Total	0.023 hours

Channel Flow		Segment ID	3		
13.	Flow Length, L	ft	88		
14.	Average Velocity, V	ft/s	2.00		
15.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr	0.012	+	+
				Channel Flow Sub-Total	0.012 hours

Watershed or subarea T_c or T_t		
(Add Sub-Total T_t from prior steps)	Total Tc (hours) =	0.062 hours
	Total Tc (minutes) =	4 minutes

Current Rainfall

Project: Teterboro Airport - South Hanger | **By:** AM | **Date:** 7/29/2025

Location: Teterboro, NJ | **Checked By:** LM | **Date Checked:** 7/29/2025

Circle One: Present (**Developed**) — **Watershed A4**

Circle One: (**T_c**) T_t **through subarea**

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)		Segment ID	1		
1.	Surface Description (NEH table 15-1)		Smooth Surfaces		
2.	Manning's Roughness Coeff., n (NEH table 15-1)		0.011		
3.	Design Flow Length, L_{des} (Total L_{des} < 100 ft)	ft	100		
4.	Two-year 24-hr rainfall, P_2	in	3.35		
5.	Land Slope, s	ft/ft	0.010		
6.	McCuen-Spiess Limit, L_{mcs} (max. 100 ft) $L_{mcs} = (100\ s^{0.5})/n$	ft	100		
7.	Compute T_t $T_t = \frac{0.007\ (nL_{des})^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.026	+	+
				Sheet Flow Sub-Total	0.026 hours

Shallow Concentrated Flow		Segment ID	2		
8.	Surface Description		Pavement		
9.	Flow Length, L	ft	182		
10.	Watercourse Slope, s	ft/ft	0.012		
11.	Average Velocity, V (NEH table 15-3)	ft/s	2.19		
12.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr	0.023	+	+
				Shallow Conc. Flow Sub-Total	0.023 hours

Channel Flow		Segment ID	3		
13.	Flow Length, L	ft	88		
14.	Average Velocity, V	ft/s	2.00		
15.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr	0.012	+	+
				Channel Flow Sub-Total	0.012 hours

Watershed or subarea T_c or T_t	Total Tc (hours) =	0.061 hours
(Add Sub-Total T_t from prior steps)	Total Tc (minutes) =	4 minutes

Current Rainfall

Project: Teterboro Airport - South Hanger | **By:** AM | **Date:** 7/29/2025

Location: Teterboro, NJ | **Checked By:** LM | **Date Checked:** 7/29/2025

Circle One: Present (**Developed**) — **Watershed A5 - Impervious**

Circle One: (**T_c**) T_t **through subarea**

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)		Segment ID	1		
1.	Surface Description (NEH table 15-1)		Smooth Surfaces		
2.	Manning's Roughness Coeff., n (NEH table 15-1)		0.011		
3.	Design Flow Length, L_{des} (Total L_{des} < 100 ft)	ft	25		
4.	Two-year 24-hr rainfall, P_2	in	3.35		
5.	Land Slope, s	ft/ft	0.035		
6.	McCuen-Spiess Limit, L_{mcs} (max. 100 ft) $L_{mcs} = (100\ s^{0.5})/n$	ft	100		
7.	Compute T_t $T_t = \frac{0.007\ (nL_{des})^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.005	+	+
				Sheet Flow Sub-Total	0.005 hours

Shallow Concentrated Flow		Segment ID			
8.	Surface Description				
9.	Flow Length, L	ft			
10.	Watercourse Slope, s	ft/ft			
11.	Average Velocity, V (NEH table 15-3)	ft/s			
12.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr		+	+
				Shallow Conc. Flow Sub-Total	0.000 hours

Channel Flow		Segment ID			
13.	Flow Length, L	ft			
14.	Average Velocity, V	ft/s			
15.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr		+	+
				Channel Flow Sub-Total	0.000 hours

Watershed or subarea T_c or T_t	Total Tc (hours) =	0.005 hours
(Add Sub-Total T_t from prior steps)	Software Limitations Require Minimum Tc =	2 minutes

Current Rainfall

Project: Teterboro Airport - South Hanger | **By:** AM | **Date:** 7/29/2025

Location: Teterboro, NJ | **Checked By:** LM | **Date Checked:** 7/29/2025

Circle One: Present (**Developed**) — **Watershed A5 - Pervious**

Circle One: (**T_c**) T_t **through subarea**

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)		Segment ID	1	2	
1.	Surface Description (NEH table 15-1)		Dense Grasses	Smooth Surfaces	
2.	Manning's Roughness Coeff., n (NEH table 15-1)		0.24	0.011	
3.	Design Flow Length, L_{des} (Total L_{des} < 100 ft)	ft	11	5	
4.	Two-year 24-hr rainfall, P_2	in	3.35	3.35	
5.	Land Slope, s	ft/ft	0.249	0.112	
6.	McCuen-Spiess Limit, L_{mcs} (max. 100 ft) $L_{mcs} = (100\ s^{0.5})/n$	ft	100	100	
7.	Compute T_t $T_t = \frac{0.007\ (nL_{des})^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.014 +	0.001 +	
				Sheet Flow Sub-Total	0.015 hours

Shallow Concentrated Flow		Segment ID			
8.	Surface Description				
9.	Flow Length, L	ft			
10.	Watercourse Slope, s	ft/ft			
11.	Average Velocity, V (NEH table 15-3)	ft/s			
12.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr	+	+	
				Shallow Conc. Flow Sub-Total	0.000 hours

Channel Flow		Segment ID			
13.	Flow Length, L	ft			
14.	Average Velocity, V	ft/s			
15.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr	+	+	
				Channel Flow Sub-Total	0.000 hours

Watershed or subarea T_c or T_t
(Add Sub-Total T_t from prior steps)

Total Tc (hours) =	0.015 hours
Software Limitations Require Minimum Tc =	2 minutes

Current Rainfall

Project:	Teterboro Airport - South Hanger	**By:**	AM	**Date:**	7/29/2025
Location:	Teterboro, NJ	**Checked By:**	LM	**Date Checked:**	7/29/2025

Circle One: Present (**Developed**) — **Watershed A6 - Impervious**

Circle One: (**T_c**) T_t **through subarea**

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)		Segment ID	1		
1.	Surface Description (NEH table 15-1)		Smooth Surfaces		
2.	Manning's Roughness Coeff., n (NEH table 15-1)		0.011		
3.	Design Flow Length, L_{des} (L_{des} < 100 ft)	ft	31		
4.	Two-year 24-hr rainfall, P_2	in	3.35		
5.	Land Slope, s	ft/ft	0.011		
6.	McCuen-Spiess Limit, L_{mcs} (max. 100 ft) $L_{mcs} = (100\ s^{0.5})/n$	ft	100		
7.	Compute T_t $T_t = \frac{0.007\ (nL_{des})^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.010	+	+
				Sheet Flow Sub-Total	0.010 hours

Shallow Concentrated Flow		Segment ID			
8.	Surface Description				
9.	Flow Length, L	ft			
10.	Watercourse Slope, s	ft/ft			
11.	Average Velocity, V (NEH table 15-3)	ft/s			
12.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr		+	+
				Shallow Conc. Flow Sub-Total	0.000 hours

Channel Flow		Segment ID			
13.	Flow Length, L	ft			
14.	Average Velocity, V	ft/s			
15.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr		+	+
				Channel Flow Sub-Total	0.000 hours

Watershed or subarea T_c or T_t		
(Add Sub-Total T_t from prior steps)	Total Tc (hours) =	0.010 hours
	Software Limitations Require Minimum Tc =	2 minutes

Current Rainfall

Project:	Teterboro Airport - South Hanger	**By:**	AM	**Date:**	7/29/2025
Location:	Teterboro, NJ	**Checked By:**	LM	**Date Checked:**	7/29/2025

Circle One: Present (**Developed**) — **Watershed A6 - Pervious**

Circle One: (**T_c**) T_t **through subarea**

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)

			Segment ID	1		2		
1.	Surface Description (NEH table 15-1)			Dense Grasses		Smooth Surfaces		
2.	Manning's Roughness Coeff., n (NEH table 15-1)			0.24		0.011		
3.	Design Flow Length, L_{des} (Total L_{des} < 100 ft)		ft	24		5		
4.	Two-year 24-hr rainfall, P_2		in	3.35		3.35		
5.	Land Slope, s		ft/ft	0.004		0.098		
6.	McCuen-Spiess Limit, L_{mcs} (max. 100 ft)	$L_{mcs} = (100\ s^{0.5})/n$	ft	26		100		
7.	Compute T_t	$T_t = \frac{0.007\ (nL_{des})^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.145	+	0.001	+	

Sheet Flow Sub-Total: **0.146 hours**

Shallow Concentrated Flow

			Segment ID					
8.	Surface Description							
9.	Flow Length, L		ft					
10.	Watercourse Slope, s		ft/ft					
11.	Average Velocity, V (NEH table 15-3)		ft/s					
12.	Compute T_t	$T_t = \frac{L}{3600\ V}$	hr		+		+	

Shallow Conc. Flow Sub-Total: **0.000 hours**

Channel Flow

			Segment ID					
13.	Flow Length, L		ft					
14.	Average Velocity, V		ft/s					
15.	Compute T_t	$T_t = \frac{L}{3600\ V}$	hr		+		+	

Channel Flow Sub-Total: **0.000 hours**

Watershed or subarea T_c or T_t
(Add Sub-Total T_t from prior steps)

Total Tc (hours) = **0.146 hours**

Total Tc (minutes) = **9 minutes**

Current Rainfall

Project:	Teterboro Airport - South Hanger	**By:**	AM	**Date:**	7/31/2025
Location:	Teterboro, NJ	**Checked By:**	LM	**Date Checked:**	7/31/2025

Circle One: Present (Developed) — **Watershed B - Impervious (Undetained)**

Circle One: (T_c) T_t **through subarea**

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)

		Segment ID	1		2		
1.	Surface Description (NEH table 15-1)		Smooth Surfaces		Dense Grasses		
2.	Manning's Roughness Coeff., n (NEH table 15-1)		0.011		0.24		
3.	Flow Length, L (total L < 100 ft)	ft	35		65		
4.	Two-year 24-hr rainfall, P_2	in	3.35		3.35		
5.	Land Slope, s	ft/ft	0.014		0.013		
6.	Compute T_t $T_t = \frac{0.007\,(nL)^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.010	+	0.197	+	
					Sheet Flow Sub-Total		0.207 hours

Shallow Concentrated Flow

		Segment ID	3		4		
7.	Surface Description		Pavement		Grassed Waterways		
8.	Flow Length, L	ft	29		513		
9.	Watercourse Slope, s	ft/ft	0.014		0.002		
10.	Average Velocity, V (NEH table 15-3)	ft/s	2.39		0.66		
11.	Compute T_t $T_t = \frac{L}{3600\,V}$	hr	0.003	+	0.217	+	
					Shallow Conc. Flow Sub-Total		0.220 hours

Channel Flow

		Segment ID					
12.	Flow Length, L	ft					
13.	Cross Sectional flow area, a	ft^2					
14.	Wetted Perimeter, P_w	ft					
15.	Hydraulic Radius, r $r = a / P_w$	ft					
16.	Channel Slope, s	ft/ft					
17.	Manning's Roughness Coeff., n						
18.	Velocity, V $V = (1.49\, r^{2/3} s^{1/2}) / n$	ft/s					
19.	Compute T_t $T_t = \frac{L}{3600\,V}$	hr		+		+	
					Channel Flow Sub-Total		0.000 hours

Channel Flow

		Segment ID					
20.	Flow Length, L	ft					
21.	Cross Sectional flow area, a	ft^2					
22.	Wetted Perimeter, P_w	ft					
23.	Hydraulic Radius, r $r = a / P_w$	ft					
24.	Channel Slope, s	ft/ft					
25.	Manning's Roughness Coeff., n						
26.	Velocity, V $V = (1.49\, r^{2/3} s^{1/2}) / n$	ft/s					
27.	Compute T_t $T_t = \frac{L}{3600\,V}$	hr		+		+	
					Channel Flow Sub-Total		0.000 hours

Channel Flow

		Segment ID			
12.	Flow Length, L	ft			

Current Rainfall

Project:	**Teterboro Airport - South Hanger**	**By:**	**AM**	**Date:**	**7/31/2025**
Location:	**Teterboro, NJ**	**Checked By:**	**LM**	**Date Checked:**	**7/31/2025**

Circle One: **Present** (Developed) — **Watershed B - Impervious (Undetained)**

Circle One: (T_c) T_t **through subarea**

Include a map, schematic, or description of flow segments.

13.	Average Velocity, V		ft/s			+		+
14.	Compute T_t	$T_t = \frac{L}{3600\ V}$	hr			+		+
					Channel Flow Sub-Total		**0.000 hours**	

Channel Flow

			Segment ID	**5**				
15.	Flow Length, L		ft	**1,353**				
16.	Average Velocity, V		ft/s	**0.40**				
17.	Compute T_t	$T_t = \frac{L}{3600\ V}$	hr	**0.940**		+		+
					Channel Flow Sub-Total		**0.940 hours**	

Watershed or subarea T_c or T_t	Total Tc (hours) =	**1.367 hours**
(Add Sub-Total T_t from prior steps)	Total Tc (minutes) =	**82 minutes**

Current Rainfall

Project: Teterboro Airport - South Hanger | **By:** AM | **Date:** 7/31/2025

Location: Teterboro, NJ | **Checked By:** LM | **Date Checked:** 7/31/2025

Circle One: Present (Developed) — Watershed B - Pervious (Undetained)

Circle One: (T_c) T_t through subarea

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)

			Segment ID	1		
1.	Surface Description (NEH table 15-1)			Woods Light Underbrush		
2.	Manning's Roughness Coeff., n (NEH table 15-1)			0.40		
3.	Flow Length, L (total L < 100 ft)		ft	100		
4.	Two-year 24-hr rainfall, P_2		in	3.35		
5.	Land Slope, s		ft/ft	0.004		
6.	Compute T_t	$T_t = \frac{0.007\,(nL)^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.666	+	+

Sheet Flow Sub-Total: **0.666 hours**

Shallow Concentrated Flow

			Segment ID	2	3	
7.	Surface Description			Woodlands	Grassed Waterways	
8.	Flow Length, L		ft	232	470	
9.	Watercourse Slope, s		ft/ft	0.005	0.000	
10.	Average Velocity, V (NEH table 15-3)		ft/s	0.35	0.26	
11.	Compute T_t	$T_t = \frac{L}{3600\,V}$	hr	0.182	+ 0.506	+

Shallow Conc. Flow Sub-Total: **0.688 hours**

Channel Flow

			Segment ID			
12.	Flow Length, L		ft			
13.	Cross Sectional flow area, a		ft^2			
14.	Wetted Perimeter, P_w		ft			
15.	Hydraulic Radius, r	$r = a / P_w$	ft			
16.	Channel Slope, s		ft/ft			
17.	Manning's Roughness Coeff., n					
18.	Velocity, V	$V = (1.49\, r^{2/3} s^{1/2}) / n$	ft/s			
19.	Compute T_t	$T_t = \frac{L}{3600\,V}$	hr		+	+

Channel Flow Sub-Total: **0.000 hours**

Channel Flow

			Segment ID			
20.	Flow Length, L		ft			
21.	Cross Sectional flow area, a		ft^2			
22.	Wetted Perimeter, P_w		ft			
23.	Hydraulic Radius, r	$r = a / P_w$	ft			
24.	Channel Slope, s		ft/ft			
25.	Manning's Roughness Coeff., n					
26.	Velocity, V	$V = (1.49\, r^{2/3} s^{1/2}) / n$	ft/s			
27.	Compute T_t	$T_t = \frac{L}{3600\,V}$	hr		+	+

Channel Flow Sub-Total: **0.000 hours**

Channel Flow

			Segment ID	4		
12.	Flow Length, L		ft	1,353		

Current Rainfall

Project:	**Teterboro Airport - South Hanger**	**By:**	**AM**	**Date:**	**7/31/2025**
Location:	**Teterboro, NJ**	**Checked By:**	**LM**	**Date Checked:**	**7/31/2025**

Circle One: **Present** (Developed) — **Watershed B - Pervious (Undetained)**

Circle One: (T_c) T_t **through subarea**

Include a map, schematic, or description of flow segments.

13.	Average Velocity, V		ft/s	0.40				
14.	Compute T_t	$T_t = \frac{L}{3600\ V}$	hr	0.940	+		+	
						Channel Flow Sub-Total		0.940 hours

Channel Flow

			Segment ID					
15.	Flow Length, L		ft					
16.	Average Velocity, V		ft/s					
17.	Compute T_t	$T_t = \frac{L}{3600\ V}$	hr		+		+	
						Channel Flow Sub-Total		0.000 hours

Watershed or subarea T_c or T_t	Total Tc (hours) =	**2.294 hours**
(Add Sub-Total T_t from prior steps)	Total Tc (minutes) =	**138 minutes**

Current Rainfall

Project: Teterboro Airport - South Hanger | **By:** AM | **Date:** 7/29/2025

Location: Teterboro, NJ | **Checked By:** LM | **Date Checked:** 7/29/2025

Circle One: Present **(Developed)** | **Watershed C - Pervious**

Circle One: **(T_c)** T_t **through subarea**

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)		Segment ID	1		
1.	Surface Description (NEH table 15-1)		Dense Grasses		
2.	Manning's Roughness Coeff., n (NEH table 15-1)		0.24		
3.	Design Flow Length, L_{des} (L_{des} < 100 ft)	ft	95		
4.	Two-year 24-hr rainfall, P_2	in	3.35		
5.	Land Slope, s	ft/ft	0.053		
6.	McCuen-Spiess Limit, L_{mcs} (max. 100 ft) $L_{mcs} = (100\ s^{0.5})/n$	ft	96		
7.	Compute T_t $T_t = \frac{0.007\ (nL_{des})^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.151	+	+
				Sheet Flow Sub-Total	0.151 hours

Shallow Concentrated Flow		Segment ID	2		
8.	Surface Description		Grassed Waterways		
9.	Flow Length, L	ft	104		
10.	Watercourse Slope, s	ft/ft	0.003		
11.	Average Velocity, V (NEH table 15-3)	ft/s	0.82		
12.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr	0.035	+	+
				Shallow Conc. Flow Sub-Total	0.035 hours

Channel Flow		Segment ID			
13.	Flow Length, L	ft			
14.	Average Velocity, V	ft/s			
15.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr		+	+
				Channel Flow Sub-Total	0.000 hours

Watershed or subarea T_c or T_t		
(Add Sub-Total T_t from prior steps)	Total Tc (hours) =	0.186 hours
	Total Tc (minutes) =	11 minutes

Projected Rainfall

Project:	Teterboro Airport - South Hanger	**By:**	AM	**Date:**	7/29/2025
Location:	Teterboro, NJ	**Checked By:**	LM	**Date Checked:**	7/29/2025

Circle One: Present (**Developed**) — **Watershed A2**

Circle One: (**T_c**) T_t **through subarea**

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)

	Segment ID		1			
1.	Surface Description (NEH table 15-1)		Smooth Surfaces			
2.	Manning's Roughness Coeff., n (NEH table 15-1)		0.011			
3.	Design Flow Length, L_{des} (Total L_{des} < 100 ft)	ft	100			
4.	Two-year 24-hr rainfall, P_2	in	3.98			
5.	Land Slope, s	ft/ft	0.013			
6.	McCuen-Spiess Limit, L_{mcs} (max. 100 ft) $L_{mcs} = (100\ s^{0.5})/n$	ft	100			
7.	Compute T_t $T_t = \frac{0.007\ (nL_{des})^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.022	+		+
					Sheet Flow Sub-Total	0.022 hours

Shallow Concentrated Flow

	Segment ID		2			
8.	Surface Description		Pavement			
9.	Flow Length, L	ft	113			
10.	Watercourse Slope, s	ft/ft	0.005			
11.	Average Velocity, V (NEH table 15-3)	ft/s	1.39			
12.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr	0.023	+		+
					Shallow Conc. Flow Sub-Total	0.023 hours

Channel Flow

	Segment ID					
13.	Flow Length, L	ft				
14.	Average Velocity, V	ft/s				
15.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr		+		+
					Channel Flow Sub-Total	0.000 hours

Watershed or subarea T_c or T_t
(Add Sub-Total T_t from prior steps)

Total Tc (hours) =	**0.044 hours**
Total Tc (minutes) =	**3 minutes**

Projected Rainfall

Project:	Teterboro Airport - South Hanger	**By:**	AM	**Date:**	7/29/2025
Location:	Teterboro, NJ	**Checked By:**	LM	**Date Checked:**	7/29/2025

Circle One: Present (**Developed**) — **Watershed A3**

Circle One: (**T_c**) T_t **through subarea**

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)		Segment ID	1		
1.	Surface Description (NEH table 15-1)		Smooth Surfaces		
2.	Manning's Roughness Coeff., n (NEH table 15-1)		0.011		
3.	Design Flow Length, L_{des} (L_{des} < 100 ft)	ft	100		
4.	Two-year 24-hr rainfall, P_2	in	3.98		
5.	Land Slope, s	ft/ft	0.010		
6.	McCuen-Spiess Limit, L_{mcs} (max. 100 ft) $L_{mcs} = (100\ s^{0.5})/n$	ft	100		
7.	Compute T_t $T_t = \frac{0.007\ (nL_{des})^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.024	+	+
				Sheet Flow Sub-Total	0.024 hours

Shallow Concentrated Flow		Segment ID	2		
8.	Surface Description		Pavement		
9.	Flow Length, L	ft	182		
10.	Watercourse Slope, s	ft/ft	0.012		
11.	Average Velocity, V (NEH table 15-3)	ft/s	2.21		
12.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr	0.023	+	+
				Shallow Conc. Flow Sub-Total	0.023 hours

Channel Flow		Segment ID	3		
13.	Flow Length, L	ft	88		
14.	Average Velocity, V	ft/s	2.00		
15.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr	0.012	+	+
				Channel Flow Sub-Total	0.012 hours

Watershed or subarea T_c or T_t		
(Add Sub-Total T_t from prior steps)	Total Tc (hours) =	0.059 hours
	Total Tc (minutes) =	4 minutes

Projected Rainfall

Project:	Teterboro Airport - South Hanger	**By:**	AM	**Date:**	7/29/2025
Location:	Teterboro, NJ	**Checked By:**	LM	**Date Checked:**	7/29/2025

Circle One: Present (**Developed**) — **Watershed A4**

Circle One: (**T_c**) T_t **through subarea**

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)

	Segment ID		1			
1.	Surface Description (NEH table 15-1)		Smooth Surfaces			
2.	Manning's Roughness Coeff., n (NEH table 15-1)		0.011			
3.	Design Flow Length, L_{des} (Total L_{des} < 100 ft)	ft	100			
4.	Two-year 24-hr rainfall, P_2	in	3.98			
5.	Land Slope, s	ft/ft	0.010			
6.	McCuen-Spiess Limit, L_{mcs} (max. 100 ft) $L_{mcs} = (100\ s^{0.5})/n$	ft	100			
7.	Compute T_t $T_t = \frac{0.007\ (nL_{des})^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.024	+		+
					Sheet Flow Sub-Total	0.024 hours

Shallow Concentrated Flow

	Segment ID		2			
8.	Surface Description		Pavement			
9.	Flow Length, L	ft	182			
10.	Watercourse Slope, s	ft/ft	0.012			
11.	Average Velocity, V (NEH table 15-3)	ft/s	2.19			
12.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr	0.023	+		+
					Shallow Conc. Flow Sub-Total	0.023 hours

Channel Flow

	Segment ID		3			
13.	Flow Length, L	ft	88			
14.	Average Velocity, V	ft/s	2.00			
15.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr	0.012	+		+
					Channel Flow Sub-Total	0.012 hours

Watershed or subarea T_c or T_t
(Add Sub-Total T_t from prior steps)

Total Tc (hours) =	0.059 hours
Total Tc (minutes) =	4 minutes

Projected Rainfall

Project:	Teterboro Airport - South Hanger	**By:**	AM	**Date:**	7/29/2025
Location:	Teterboro, NJ	**Checked By:**	LM	**Date Checked:**	7/29/2025

Circle One: Present **(Developed)** — **Watershed A5 - Impervious**

Circle One: **(T_c)** T_t **through subarea**

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)		Segment ID	1		
1.	Surface Description (NEH table 15-1)		Smooth Surfaces		
2.	Manning's Roughness Coeff., n (NEH table 15-1)		0.011		
3.	Design Flow Length, L_{des} (L_{des} < 100 ft)	ft	25		
4.	Two-year 24-hr rainfall, P_2	in	3.98		
5.	Land Slope, s	ft/ft	0.035		
6.	McCuen-Spiess Limit, L_{mcs} (max. 100 ft) $L_{mcs} = (100\ s^{0.5})/n$	ft	100		
7.	Compute T_t $T_t = \frac{0.007\ (nL_{des})^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.005	+	+
				Sheet Flow Sub-Total	0.005 hours

Shallow Concentrated Flow		Segment ID			
8.	Surface Description				
9.	Flow Length, L	ft			
10.	Watercourse Slope, s	ft/ft			
11.	Average Velocity, V (NEH table 15-3)	ft/s			
12.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr		+	+
				Shallow Conc. Flow Sub-Total	0.000 hours

Channel Flow		Segment ID			
13.	Flow Length, L	ft			
14.	Average Velocity, V	ft/s			
15.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr		+	+
				Channel Flow Sub-Total	0.000 hours

Watershed or subarea T_c or T_t **(Add Sub-Total T_t from prior steps)**		
	Total Tc (hours) =	0.005 hours
	Software Limitations Require Minimum Tc =	2 minutes

Projected Rainfall

Project: Teterboro Airport - South Hanger | **By:** AM | **Date:** 7/29/2025

Location: Teterboro, NJ | **Checked By:** LM | **Date Checked:** 7/29/2025

Circle One: Present (**Developed**) — **Watershed A5 - Pervious**

Circle One: (**T_c**) T_t **through subarea**

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)

			Segment ID	1	2	
1.	Surface Description (NEH table 15-1)			Dense Grasses	Smooth Surfaces	
2.	Manning's Roughness Coeff., n (NEH table 15-1)			0.24	0.011	
3.	Design Flow Length, L_{des} (Total L_{des} < 100 ft)		ft	11	5	
4.	Two-year 24-hr rainfall, P_2		in	3.98	3.98	
5.	Land Slope, s		ft/ft	0.249	0.112	
6.	McCuen-Spiess Limit, L_{mcs} (max. 100 ft)	$L_{mcs} = (100\ s^{0.5})/n$	ft	100	100	
7.	Compute T_t	$T_t = \frac{0.007\ (nL_{des})^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.013 +	0.001 +	

Sheet Flow Sub-Total: **0.014 hours**

Shallow Concentrated Flow

			Segment ID			
8.	Surface Description					
9.	Flow Length, L		ft			
10.	Watercourse Slope, s		ft/ft			
11.	Average Velocity, V (NEH table 15-3)		ft/s			
12.	Compute T_t	$T_t = \frac{L}{3600\ V}$	hr	+	+	

Shallow Conc. Flow Sub-Total: **0.000 hours**

Channel Flow

			Segment ID			
13.	Flow Length, L		ft			
14.	Average Velocity, V		ft/s			
15.	Compute T_t	$T_t = \frac{L}{3600\ V}$	hr	+	+	

Channel Flow Sub-Total: **0.000 hours**

Watershed or subarea T_c or T_t
(Add Sub-Total T_t from prior steps)

Total Tc (hours) = **0.014 hours**

Software Limitations Require Minimum Tc = **2 minutes**

Projected Rainfall

Project: Teterboro Airport - South Hanger | **By:** AM | **Date:** 7/29/2025

Location: Teterboro, NJ | **Checked By:** LM | **Date Checked:** 7/29/2025

Circle One: Present (**Developed**) — **Watershed A6 - Impervious**

Circle One: (**T_c**) T_t **through subarea**

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)		Segment ID	1		
1.	Surface Description (NEH table 15-1)		Smooth Surfaces		
2.	Manning's Roughness Coeff., n (NEH table 15-1)		0.011		
3.	Design Flow Length, L_{des} (Total L_{des} < 100 ft)	ft	31		
4.	Two-year 24-hr rainfall, P_2	in	3.98		
5.	Land Slope, s	ft/ft	0.011		
6.	McCuen-Spiess Limit, L_{mcs} (max. 100 ft) $L_{mcs} = (100\ s^{0.5})/n$	ft	100		
7.	Compute T_t $T_t = \frac{0.007\ (nL_{des})^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.009	+	+
			Sheet Flow Sub-Total		0.009 hours

Shallow Concentrated Flow		Segment ID			
8.	Surface Description				
9.	Flow Length, L	ft			
10.	Watercourse Slope, s	ft/ft			
11.	Average Velocity, V (NEH table 15-3)	ft/s			
12.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr		+	+
			Shallow Conc. Flow Sub-Total		0.000 hours

Channel Flow		Segment ID			
13.	Flow Length, L	ft			
14.	Average Velocity, V	ft/s			
15.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr		+	+
			Channel Flow Sub-Total		0.000 hours

Watershed or subarea T_c or T_t
(Add Sub-Total T_t from prior steps)

Total Tc (hours) =	0.009 hours
Software Limitations Require Minimum Tc =	2 minutes

Projected Rainfall

Project:	Teterboro Airport - South Hanger	**By:**	AM	**Date:**	7/29/2025
Location:	Teterboro, NJ	**Checked By:**	LM	**Date Checked:**	7/29/2025

Circle One: Present **(Developed)** — **Watershed A6 - Pervious**

Circle One: **(T_c)** T_t **through subarea**

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)

			Segment ID	1	2	
1.	Surface Description (NEH table 15-1)			Dense Grasses	Smooth Surfaces	
2.	Manning's Roughness Coeff., n (NEH table 15-1)			0.24	0.011	
3.	Design Flow Length, L_{des} (Total L_{des} < 100 ft)		ft	24	5	
4.	Two-year 24-hr rainfall, P_2		in	3.98	3.98	
5.	Land Slope, s		ft/ft	0.004	0.098	
6.	McCuen-Spiess Limit, L_{mcs} (max. 100 ft)	$L_{mcs} = (100\ s^{0.5})/n$	ft	26	100	
7.	Compute T_t	$T_t = \frac{0.007\ (nL_{des})^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.133	+ 0.001	+

Sheet Flow Sub-Total **0.134 hours**

Shallow Concentrated Flow

			Segment ID			
8.	Surface Description					
9.	Flow Length, L		ft			
10.	Watercourse Slope, s		ft/ft			
11.	Average Velocity, V (NEH table 15-3)		ft/s			
12.	Compute T_t	$T_t = \frac{L}{3600\ V}$	hr		+	+

Shallow Conc. Flow Sub-Total **0.000 hours**

Channel Flow

			Segment ID			
13.	Flow Length, L		ft			
14.	Average Velocity, V		ft/s			
15.	Compute T_t	$T_t = \frac{L}{3600\ V}$	hr		+	+

Channel Flow Sub-Total **0.000 hours**

Watershed or subarea T_c or T_t
(Add Sub-Total T_t from prior steps)

Total Tc (hours) =	**0.134 hours**
Total Tc (minutes) =	**8 minutes**

Projected Rainfall

Project:	Teterboro Airport - South Hanger	**By:**	AM	**Date:**	7/31/2025
Location:	Teterboro, NJ	**Checked By:**	LM	**Date Checked:**	7/31/2025

Circle One: Present (Developed) — Watershed B - Impervious (Undetained)

Circle One: (T_c) T_t through subarea

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)

		Segment ID	1	2	
1.	Surface Description (NEH table 15-1)		Smooth Surfaces	Dense Grasses	
2.	Manning's Roughness Coeff., n (NEH table 15-1)		0.011	0.24	
3.	Flow Length, L (total L < 100 ft)	ft	35	65	
4.	Two-year 24-hr rainfall, P_2	in	3.98	3.98	
5.	Land Slope, s	ft/ft	0.014	0.013	
6.	Compute T_t $T_t = \frac{0.007\,(nL)^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.009 +	0.181 +	
				Sheet Flow Sub-Total	0.190 hours

Shallow Concentrated Flow

		Segment ID	3	4	
7.	Surface Description		Pavement	Grassed Waterways	
8.	Flow Length, L	ft	29	513	
9.	Watercourse Slope, s	ft/ft	0.014	0.002	
10.	Average Velocity, V (NEH table 15-3)	ft/s	2.39	0.66	
11.	Compute T_t $T_t = \frac{L}{3600\,V}$	hr	0.003 +	0.217 +	
				Shallow Conc. Flow Sub-Total	0.220 hours

Channel Flow

		Segment ID			
12.	Flow Length, L	ft			
13.	Cross Sectional flow area, a	ft^2			
14.	Wetted Perimeter, P_w	ft			
15.	Hydraulic Radius, r $r = a / P_w$	ft			
16.	Channel Slope, s	ft/ft			
17.	Manning's Roughness Coeff., n				
18.	Velocity, V $V = (1.49\, r^{2/3} s^{1/2}) / n$	ft/s			
19.	Compute T_t $T_t = \frac{L}{3600\,V}$	hr	+	+	
				Channel Flow Sub-Total	0.000 hours

Channel Flow

		Segment ID			
20.	Flow Length, L	ft			
21.	Cross Sectional flow area, a	ft^2			
22.	Wetted Perimeter, P_w	ft			
23.	Hydraulic Radius, r $r = a / P_w$	ft			
24.	Channel Slope, s	ft/ft			
25.	Manning's Roughness Coeff., n				
26.	Velocity, V $V = (1.49\, r^{2/3} s^{1/2}) / n$	ft/s			
27.	Compute T_t $T_t = \frac{L}{3600\,V}$	hr	+	+	
				Channel Flow Sub-Total	0.000 hours

Channel Flow

		Segment ID			
12.	Flow Length, L	ft			

Projected Rainfall

Project:	**Teterboro Airport - South Hanger**	**By:**	**AM**	**Date:**	**7/31/2025**
Location:	**Teterboro, NJ**	**Checked By:**	**LM**	**Date Checked:**	**7/31/2025**

Circle One: **Present** (Developed) — **Watershed B - Impervious (Undetained)**

Circle One: (T_c) T_t **through subarea** _____

Include a map, schematic, or description of flow segments.

13.	Average Velocity, V		ft/s					
14.	Compute T_t	$T_t = \frac{L}{3600\ V}$	hr		+		+	
					Channel Flow Sub-Total			**0.000 hours**

Channel Flow

			Segment ID	**5**				
15.	Flow Length, L		ft	**1,353**				
16.	Average Velocity, V		ft/s	**0.40**				
17.	Compute T_t	$T_t = \frac{L}{3600\ V}$	hr	**0.940**	+		+	
					Channel Flow Sub-Total			**0.940 hours**

Watershed or subarea T_c or T_t	Total Tc (hours) =	**1.350 hours**
(Add Sub-Total T_t from prior steps)	Total Tc (minutes) =	**81 minutes**

Projected Rainfall

Project: Teterboro Airport - South Hanger | **By:** AM | **Date:** 7/31/2025

Location: Teterboro, NJ | **Checked By:** LM | **Date Checked:** 7/31/2025

Circle One: Present (Developed) — Watershed B - Pervious (Undetained)

Circle One: (T_c) T_t through subarea

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)

			Segment ID	1		
1.	Surface Description (NEH table 15-1)			Woods Light Underbrush		
2.	Manning's Roughness Coeff., n (NEH table 15-1)			0.40		
3.	Flow Length, L (total L < 100 ft)		ft	100		
4.	Two-year 24-hr rainfall, P_2		in	3.98		
5.	Land Slope, s		ft/ft	0.004		
6.	Compute T_t	$T_t = \frac{0.007\,(nL)^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.611	+	+

Sheet Flow Sub-Total: **0.611 hours**

Shallow Concentrated Flow

			Segment ID	2	3	
7.	Surface Description			Woodlands	Grassed Waterways	
8.	Flow Length, L		ft	232	470	
9.	Watercourse Slope, s		ft/ft	0.005	0.000	
10.	Average Velocity, V (NEH table 15-3)		ft/s	0.35	0.26	
11.	Compute T_t	$T_t = \frac{L}{3600\,V}$	hr	0.182	+ 0.506	+

Shallow Conc. Flow Sub-Total: **0.688 hours**

Channel Flow

			Segment ID			
12.	Flow Length, L		ft			
13.	Cross Sectional flow area, a		ft^2			
14.	Wetted Perimeter, P_w		ft			
15.	Hydraulic Radius, r	$r = a / P_w$	ft			
16.	Channel Slope, s		ft/ft			
17.	Manning's Roughness Coeff., n					
18.	Velocity, V	$V = (1.49\, r^{2/3} s^{1/2}) / n$	ft/s			
19.	Compute T_t	$T_t = \frac{L}{3600\,V}$	hr		+	+

Channel Flow Sub-Total: **0.000 hours**

Channel Flow

			Segment ID			
20.	Flow Length, L		ft			
21.	Cross Sectional flow area, a		ft^2			
22.	Wetted Perimeter, P_w		ft			
23.	Hydraulic Radius, r	$r = a / P_w$	ft			
24.	Channel Slope, s		ft/ft			
25.	Manning's Roughness Coeff., n					
26.	Velocity, V	$V = (1.49\, r^{2/3} s^{1/2}) / n$	ft/s			
27.	Compute T_t	$T_t = \frac{L}{3600\,V}$	hr		+	+

Channel Flow Sub-Total: **0.000 hours**

Channel Flow

			Segment ID	4		
12.	Flow Length, L		ft	1,353		

Projected Rainfall

Project:	**Teterboro Airport - South Hanger**	**By:**	**AM**	**Date:**	**7/31/2025**
Location:	**Teterboro, NJ**	**Checked By:**	**LM**	**Date Checked:**	**7/31/2025**

Circle One: **Present** (Developed) — **Watershed B - Pervious (Undetained)**

Circle One: (T_c) T_t **through subarea**

Include a map, schematic, or description of flow segments.

13.	Average Velocity, V		ft/s	0.40				
14.	Compute T_t	$T_t = \frac{L}{3600\ V}$	hr	0.940	+		+	
						Channel Flow Sub-Total		0.940 hours

Channel Flow

			Segment ID					
15.	Flow Length, L		ft					
16.	Average Velocity, V		ft/s					
17.	Compute T_t	$T_t = \frac{L}{3600\ V}$	hr		+		+	
						Channel Flow Sub-Total		0.000 hours

Watershed or subarea T_c or T_t	Total Tc (hours) =	**2.239 hours**
(Add Sub-Total T_t from prior steps)	Total Tc (minutes) =	**134 minutes**

Projected Rainfall

Project:	Teterboro Airport - South Hanger	**By:**	AM	**Date:**	7/29/2025
Location:	Teterboro, NJ	**Checked By:**	LM	**Date Checked:**	7/29/2025

Circle One: Present **(Developed)** — Watershed C - Pervious

Circle One: **(T_c)** T_t **through subarea**

Include a map, schematic, or description of flow segments.

Sheet Flow (Applicable to T_c Only)

			Segment ID			
1.	Surface Description (NEH table 15-1)		1			
			Dense Grasses			
2.	Manning's Roughness Coeff., n (NEH table 15-1)		0.24			
3.	Design Flow Length, L_{des} (L_{des} < 100 ft)	ft	95			
4.	Two-year 24-hr rainfall, P_2	in	3.98			
5.	Land Slope, s	ft/ft	0.053			
6.	McCuen-Spiess Limit, L_{mcs} (max. 100 ft) $L_{mcs} = (100\ s^{0.5})/n$	ft	96			
7.	Compute T_t $T_t = \frac{0.007\ (nL_{des})^{0.8}}{P_2^{0.5} s^{0.4}}$	hr	0.138	+		+
					Sheet Flow Sub-Total	0.138 hours

Shallow Concentrated Flow

			Segment ID			
8.	Surface Description		2			
			Grassed Waterways			
9.	Flow Length, L	ft	104			
10.	Watercourse Slope, s	ft/ft	0.003			
11.	Average Velocity, V (NEH table 15-3)	ft/s	0.82			
12.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr	0.035	+		+
					Shallow Conc. Flow Sub-Total	0.035 hours

Channel Flow

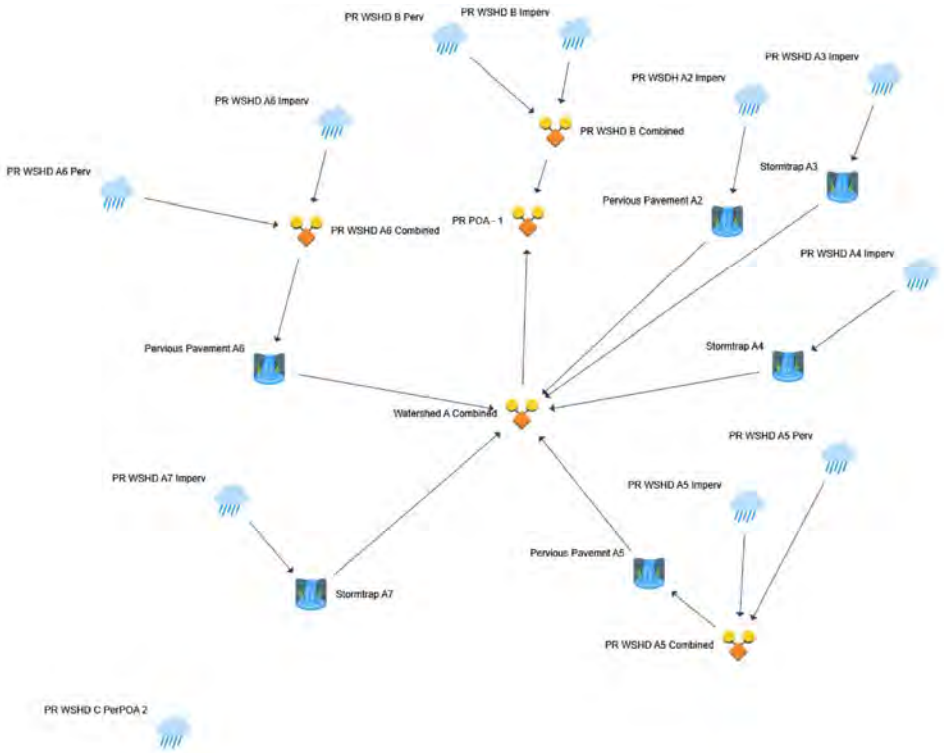
			Segment ID			
13.	Flow Length, L	ft				
14.	Average Velocity, V	ft/s				
15.	Compute T_t $T_t = \frac{L}{3600\ V}$	hr		+		+
					Channel Flow Sub-Total	0.000 hours

Watershed or subarea T_c or T_t		
(Add Sub-Total T_t from prior steps)	Total Tc (hours) =	0.173 hours
	Total Tc (minutes) =	10 minutes

Basin Model

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Hydrograph by Return Period

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07-31-2025

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Outflow (cfs)							
			1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
1	NRCS Runoff	PR WSDH A2 Imperv		3.762			5.839			9.979
2	Pond Route	Pervious Pavement A2		0.208			0.361			4.237
4	NRCS Runoff	PR WSHD A3 Imperv		7.994			12.41			21.22
5	Pond Route	Stormtrap A3		0.117			0.156			0.217
7	NRCS Runoff	PR WSHD A4 Imperv		7.208			11.19			19.14
8	Pond Route	Stormtrap A4		0.109			0.146			0.204
10	NRCS Runoff	PR WSHD A5 Imperv		2.519			3.910			6.682
11	NRCS Runoff	PR WSHD A5 Perv		0.095			0.187			0.377
12	Junction	PR WSHD A5 Combined		2.614			4.097			7.059
13	Pond Route	Pervious Pavemnt A5		0.172			0.596			4.200
15	NRCS Runoff	PR WSHD A6 Imperv		1.341			2.082			3.558
16	NRCS Runoff	PR WSHD A6 Perv		0.269			0.537			1.095
17	Junction	PR WSHD A6 Combined		1.579			2.566			4.560
18	Pond Route	Pervious Pavement A6		0.140			0.192			1.284
20	NRCS Runoff	PR WSHD A7 Imperv		5.463			8.479			14.49
21	Pond Route	Stormtrap A7		1.471			5.423			11.71
23	Junction	Watershed A Combined		2.129			6.247			20.49
25	NRCS Runoff	PR WSHD B Perv		3.227			7.003			15.31
26	NRCS Runoff	PR WSHD B Imperv		1.696			2.639			4.517
27	Junction	PR WSHD B Combined		4.459			8.963			18.71
29	Junction	PR POA - 1		5.596			10.78			26.86
32	NRCS Runoff	PR WSHD C PerPOA 2		0.076			0.151			0.309

Hydrograph 2-yr Summary

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Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	NRCS Runoff	PR WSDH A2 Imperv	3.762	12.10	12,199	----		
2	Pond Route	Pervious Pavement A2	0.208	13.53	11,879	1	1.47	7,106
4	NRCS Runoff	PR WSHD A3 Imperv	7.994	12.10	26,476	----		
5	Pond Route	Stormtrap A3	0.117	20.45	14,016	4	2.29	21,228
7	NRCS Runoff	PR WSHD A4 Imperv	7.208	12.10	23,874	----		
8	Pond Route	Stormtrap A4	0.109	20.00	12,971	7	2.22	19,040
10	NRCS Runoff	PR WSHD A5 Imperv	2.519	12.10	8,168	----		
11	NRCS Runoff	PR WSHD A5 Perv	0.095	12.10	258	----		
12	Junction	PR WSHD A5 Combined	2.614	12.10	8,426	10, 11		
13	Pond Route	Pervious Pavemnt A5	0.172	13.33	8,405	12	4.60	4,464
15	NRCS Runoff	PR WSHD A6 Imperv	1.341	12.10	4,349	----		
16	NRCS Runoff	PR WSHD A6 Perv	0.269	12.15	937	----		
17	Junction	PR WSHD A6 Combined	1.579	12.10	5,286	15, 16		
18	Pond Route	Pervious Pavement A6	0.140	13.12	5,269	17	3.44	2,546
20	NRCS Runoff	PR WSHD A7 Imperv	5.463	12.10	17,714	----		
21	Pond Route	Stormtrap A7	1.471	12.25	17,505	20	4.63	8,374
23	Junction	Watershed A Combined	2.129	12.25	70,046	2, 5, 8, 13, 18, 21		
25	NRCS Runoff	PR WSHD B Perv	3.227	13.60	45,593	----		
26	NRCS Runoff	PR WSHD B Imperv	1.696	12.87	18,149	----		
27	Junction	PR WSHD B Combined	4.459	13.40	63,742	25, 26		
29	Junction	PR POA - 1	5.596	13.37	133,788	23, 27		
32	NRCS Runoff	PR WSHD C PerPOA 2	0.076	12.17	280	----		

Hydrograph Report

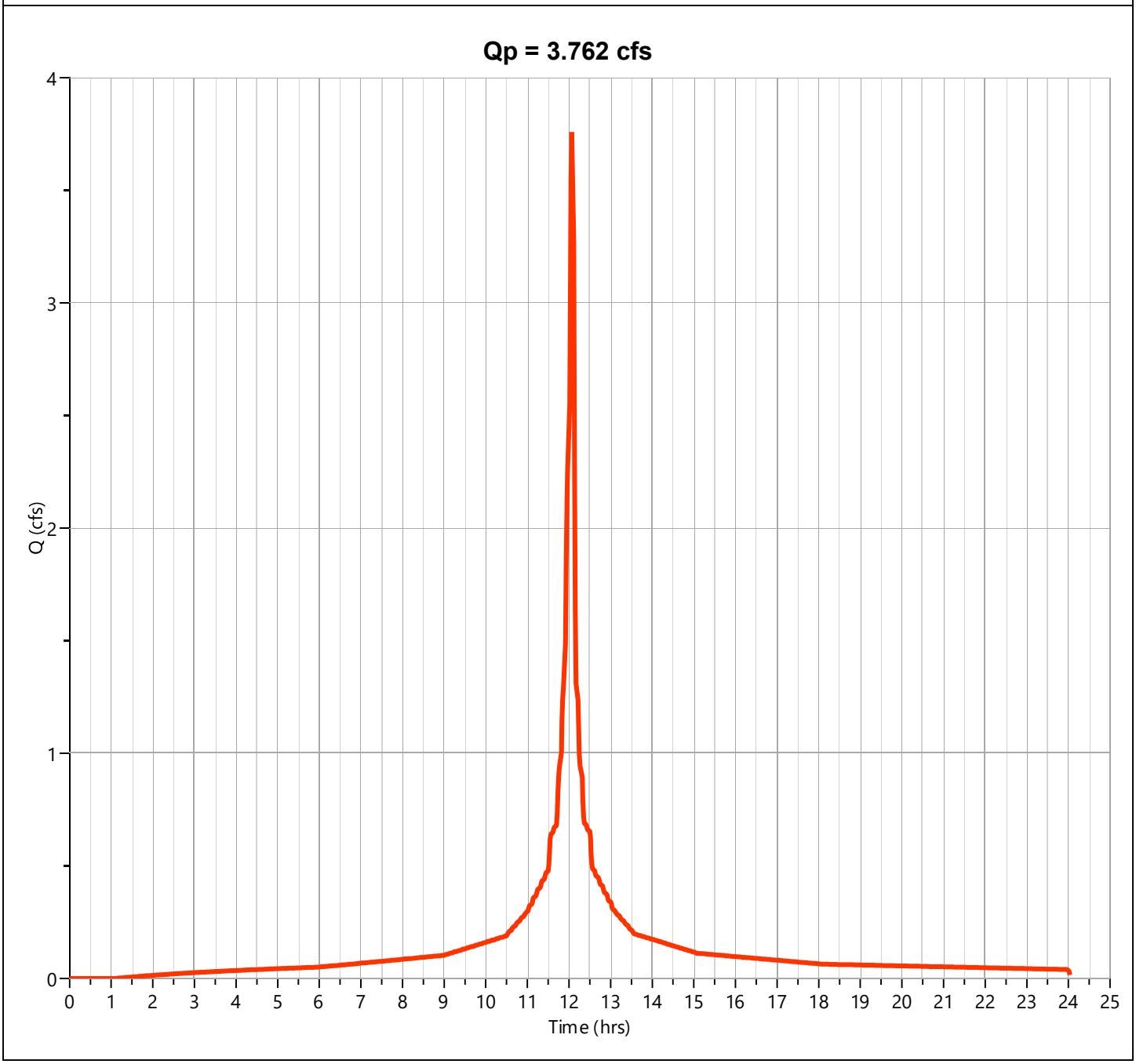
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07-31-2025

PR WSDH A2 Imperv

Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 3.762 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 12,199 cuft
Drainage Area	= 1.15 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 3.0 min
Total Rainfall	= 3.35 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

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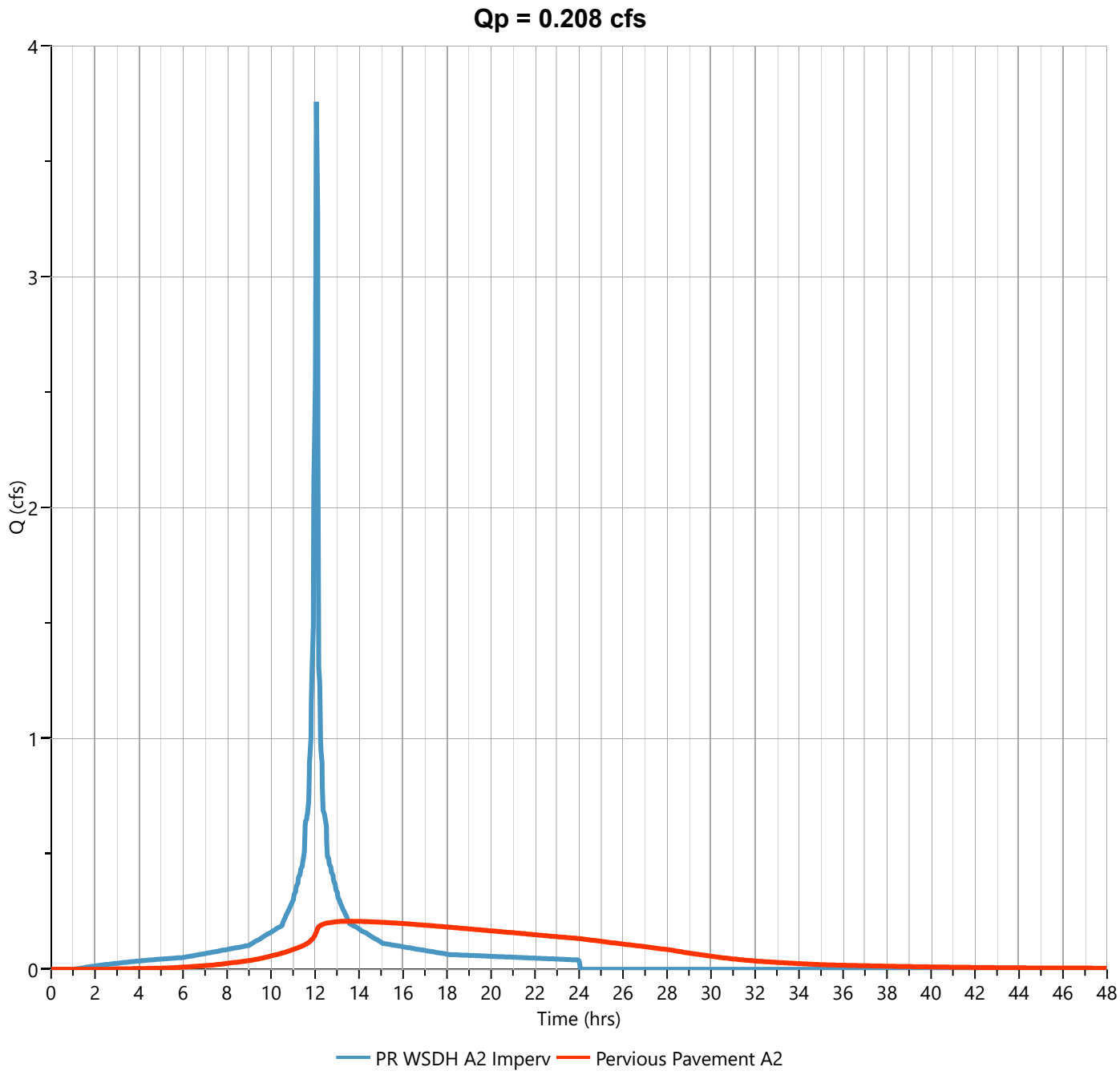
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Pervious Pavement A2 — Hyd. No. 2

Hydrograph Type	= Pond Route	Peak Flow	= 0.208 cfs
Storm Frequency	= 2-yr	Time to Peak	= 13.53 hrs
Time Interval	= 1 min	Hydrograph Volume	= 11,879 cuft
Inflow Hydrograph	= 1 - PR WSDH A2 Imperv	Max. Elevation	= 1.47 ft
Pond Name	= Pervious Pavement A2	Max. Storage	= 7,106 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 7.47 hrs



Pond Report

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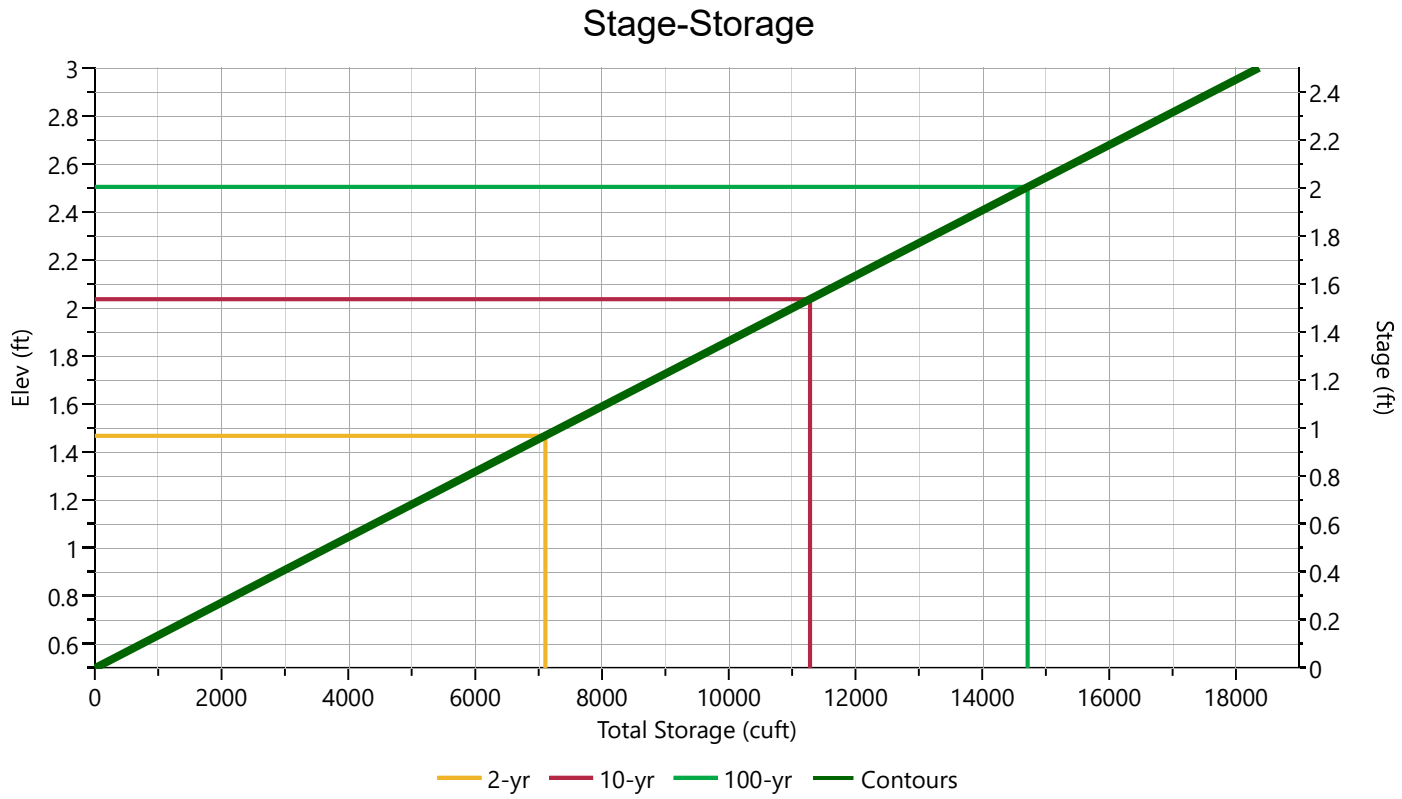
Pervious Pavement A2

Stage-Storage

User Defined Contours	
Description	**Input**
Bottom Elevation, ft	0.50
Voids (%)	40.00
Volume Calc	None

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)	Total Storage (cuft)
0.00	0.50	18,362	0.000	0.000
0.50	1.00	18,362	3,672	3,672
1.00	1.50	18,362	3,672	7,345
1.50	2.00	18,362	3,672	11,017
2.00	2.50	18,362	3,672	14,690
2.50	3.00	18,362	3,672	18,362



Pond Report

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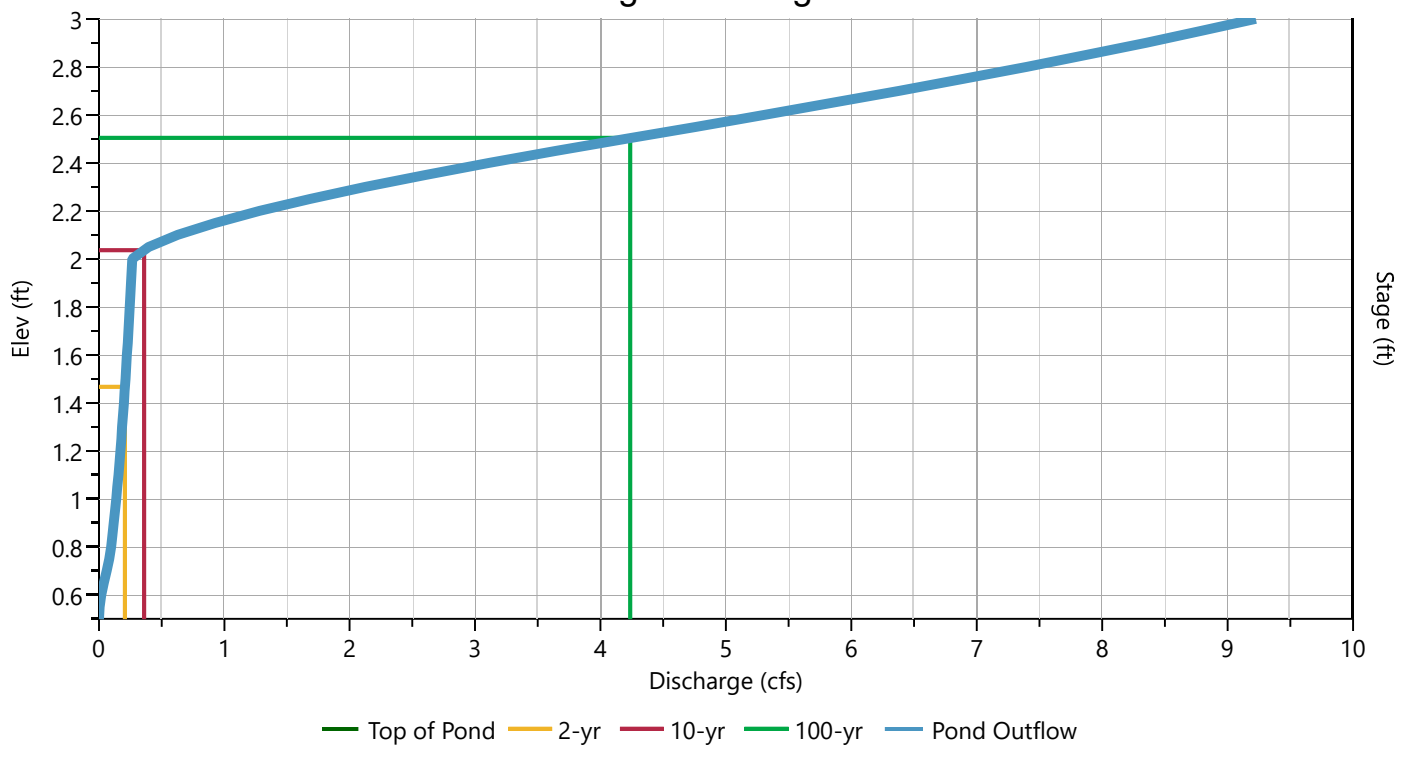
07-31-2025

Pervious Pavement A2

Stage-Discharge

Culvert / Orifices	Cir Culvert	Orifice			Perforated Riser	
		1 (m)	2	3		
Rise, in	18	3			Hole Diameter, in	
Span, in	18	3			No. holes	
No. Barrels	1	1			Invert Elevation, ft	
Invert Elevation, ft	0.50	0.50			Height, ft	
Orifice Coefficient, Co	0.60	0.60			Orifice Coefficient, Co	
Length, ft	10					
Barrel Slope, %	1					
N-Value, n	0.012					
Weirs	**Riser**	**Weir**			**Ancillary**	
		1 (m)	2	3		
Shape / Type		Rectangular			Exfiltration, in/hr	
Crest Elevation, ft		2				
Crest Length, ft		3.5				
Angle, deg						
Weir Coefficient, Cw		3.3				

m = Flows through Culvert, i = Independent



Pond Report

File: Proposed HydrologyCurrent Precipitation.hys

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Pervious Pavement A2

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.00	0.50	0.000	0.000	0.000				0.000						0.000
0.50	1.00	3,672	0.138 ic	0.138				0.000						0.138
1.00	1.50	7,345	0.212 ic	0.212				0.000						0.212
1.50	2.00	11,017	0.267 ic	0.267				0.000						0.267
2.00	2.50	14,690	4.200 oc	0.159				4.040 s						4.200
2.50	3.00	18,362	9.226 oc	0.149				9.077 s						9.226

Suffix key: ic = inlet control, oc = outlet control, s = submerged weir

Pond Report

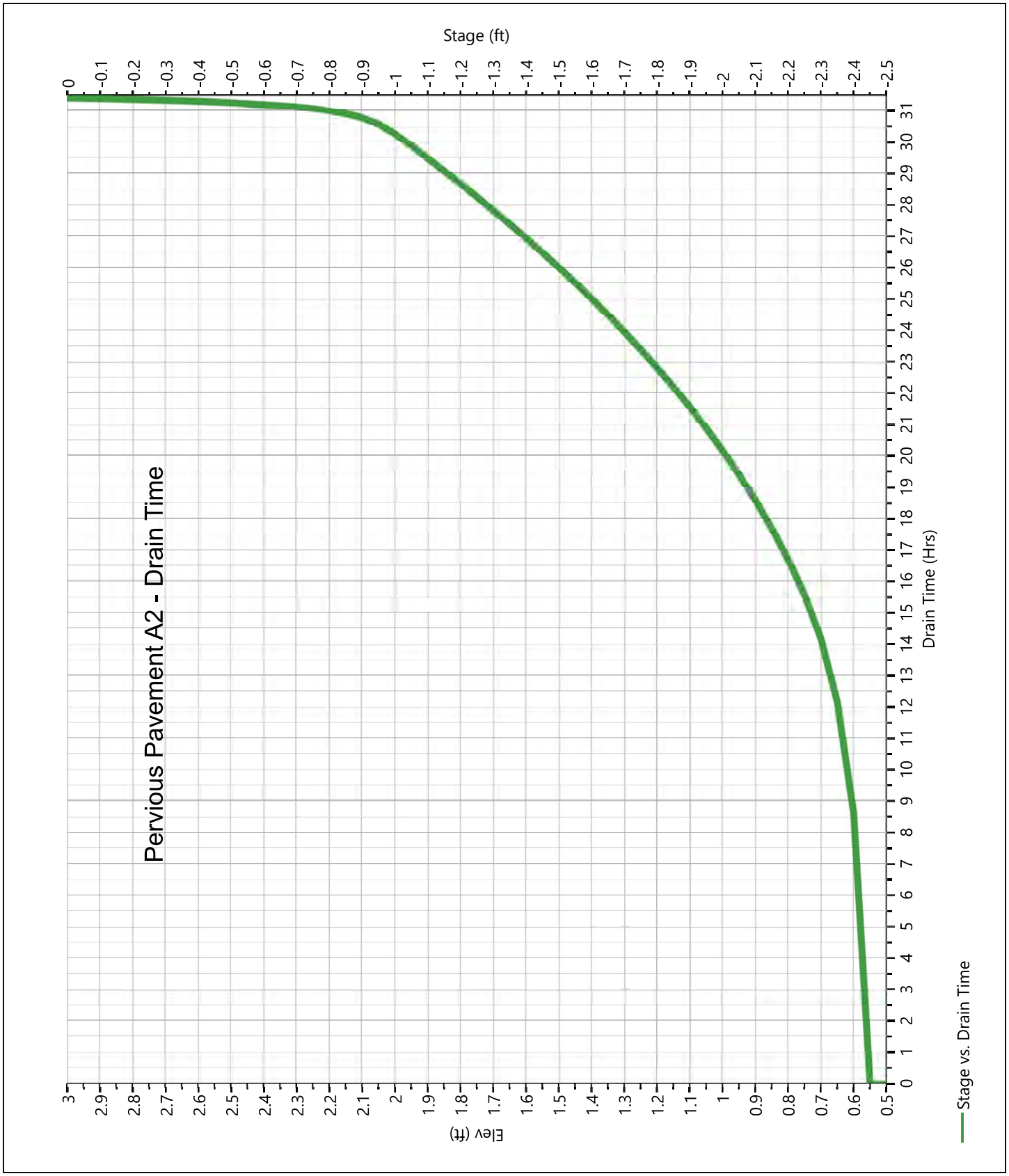
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Pervious Pavement A2

Pond Drawdown



Hydrograph Report

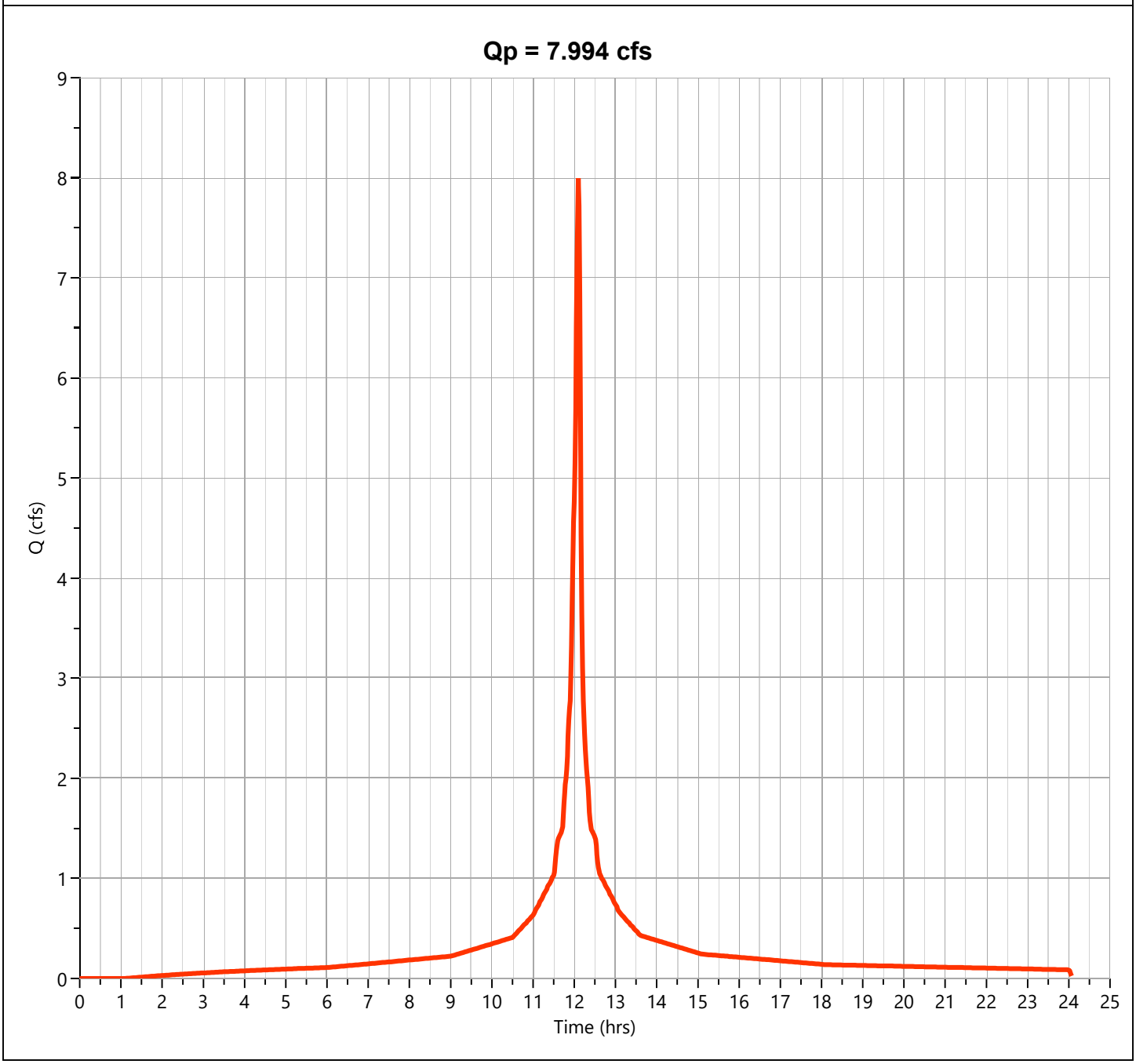
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07-31-2025

PR WSHD A3 Imperv

Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 7.994 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 26,476 cuft
Drainage Area	= 2.34 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 4.0 min
Total Rainfall	= 3.35 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.38

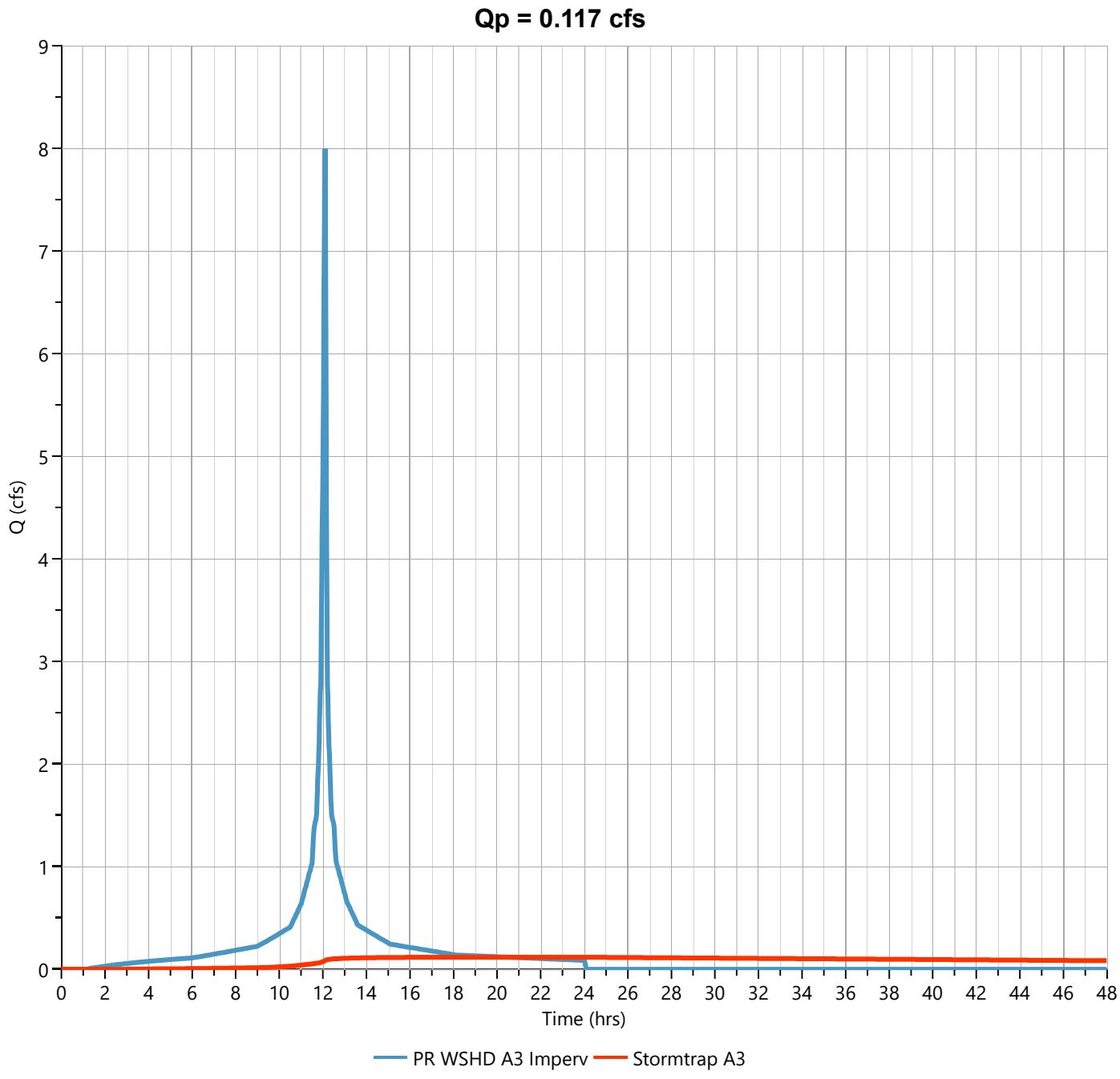
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07-31-2025

Stormtrap A3 — Hyd. No. 5

Hydrograph Type	= Pond Route	Peak Flow	= 0.117 cfs
Storm Frequency	= 2-yr	Time to Peak	= 20.45 hrs
Time Interval	= 1 min	Hydrograph Volume	= 14,016 cuft
Inflow Hydrograph	= 4 - PR WSHD A3 Imperv	Max. Elevation	= 2.29 ft
Pond Name	= Stormtrap A3	Max. Storage	= 21,228 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 15.80 hrs



Pond Report

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07-31-2025

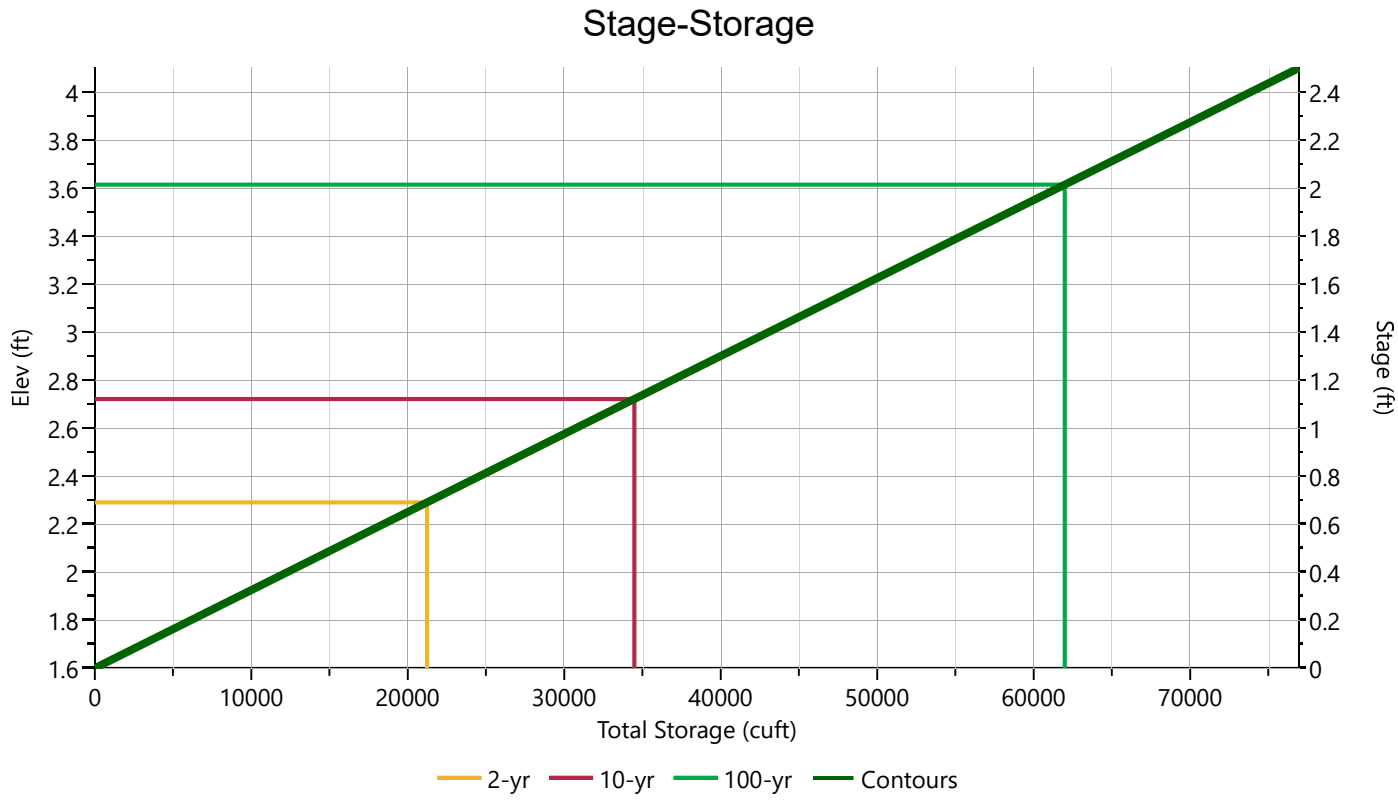
Stormtrap A3

Stage-Storage

User Defined Contours	
Description	**Input**
Bottom Elevation, ft	1.60
Voids (%)	100.00
Volume Calc	Conic

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)	Total Storage (cuft)
0.00	1.60	30,793	0.000	0.000
1.00	2.60	30,793	30,790	30,790
2.00	3.60	30,793	30,790	61,580
2.50	4.10	30,793	15,395	76,975



Pond Report

File: Proposed HydrologyCurrent Precipitation.hys

Hydrology Studio v 3.0.0.38

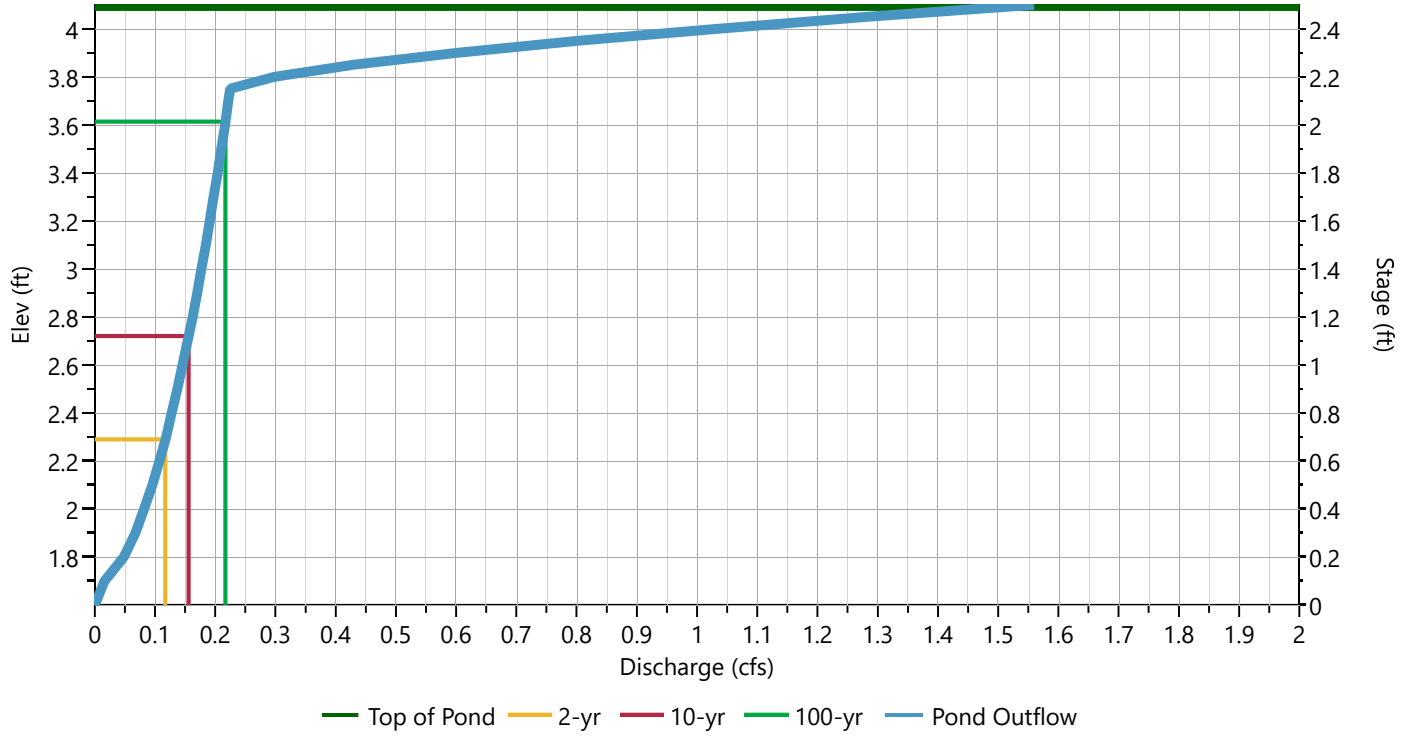
07-31-2025

Stormtrap A3

Stage-Discharge

Culvert / Orifices	Cir Culvert	Orifice			Perforated Riser	
		1 (m)	2	3		
Rise, in	18	2.5			Hole Diameter, in	
Span, in	18	2.5			No. holes	
No. Barrels	1	1			Invert Elevation, ft	
Invert Elevation, ft	1.60	1.60			Height, ft	
Orifice Coefficient, Co	0.60	0.60			Orifice Coefficient, Co	
Length, ft	10					
Barrel Slope, %	.3					
N-Value, n	0.012					
Weirs	**Riser**	**Weir**			**Ancillary**	
		1 (m)	2	3		
Shape / Type		Rectangular			Exfiltration, in/hr	
Crest Elevation, ft		3.75				
Crest Length, ft		2				
Angle, deg						
Weir Coefficient, Cw		3.3				

m = Flows through Culvert, i = Independent



Pond Report

File: Proposed HydrologyCurrent Precipitation.hys

Hydrology Studio v 3.0.0.38

07-31-2025

Stormtrap A3

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.00	1.60	0.000	0.000	0.000				0.000						0.000
1.00	2.60	30,790	0.146 oc	0.146				0.000						0.146
2.00	3.60	61,580	0.216 oc	0.216				0.000						0.216
2.50	4.10	76,975	1.559 oc	0.193				1.367						1.559

Suffix key: ic = inlet control, oc = outlet control, s = submerged weir

Pond Report

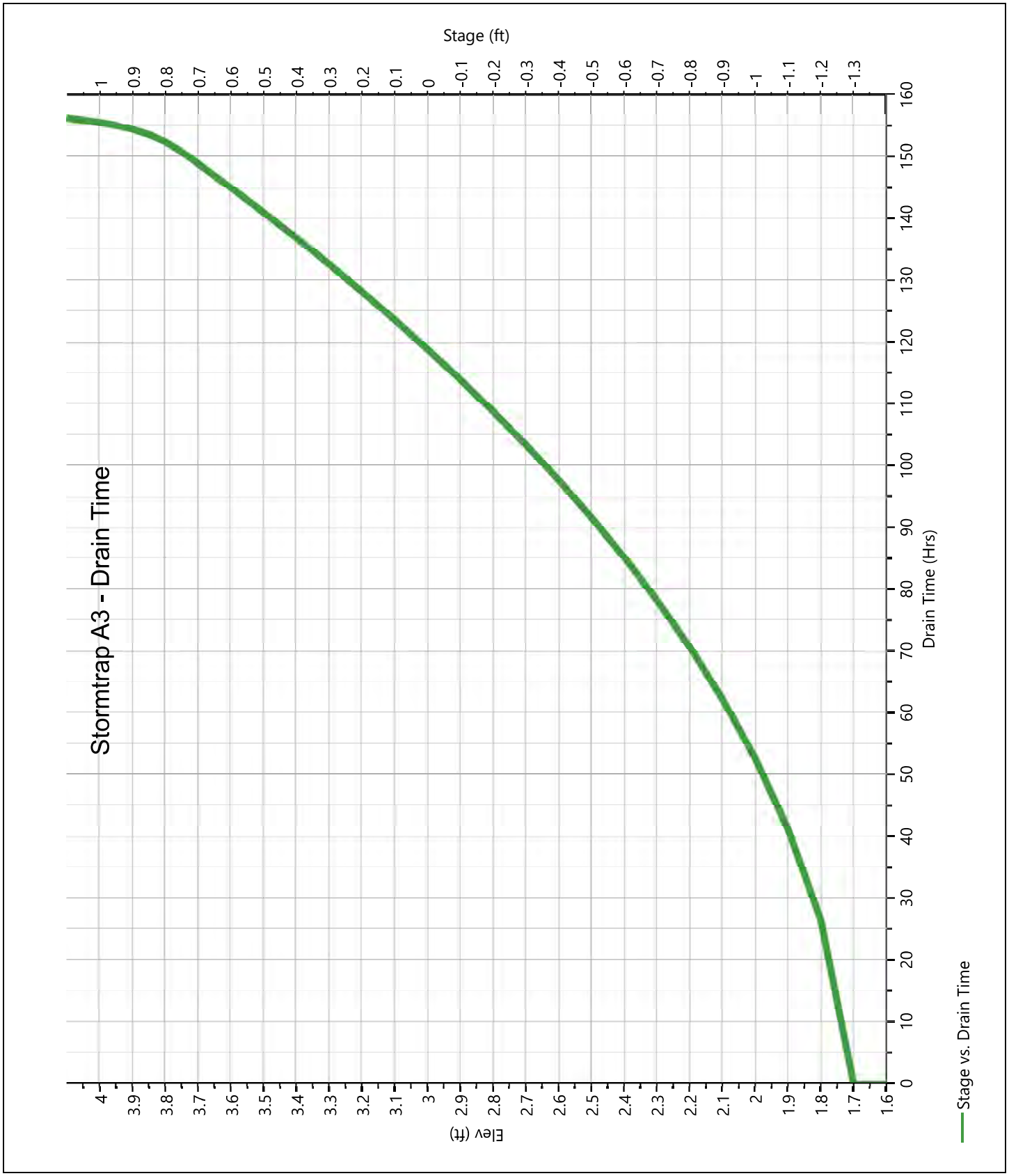
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Hydrology Studio v 3.0.0.38

07-31-2025

Stormtrap A3

Pond Drawdown



Hydrograph Report

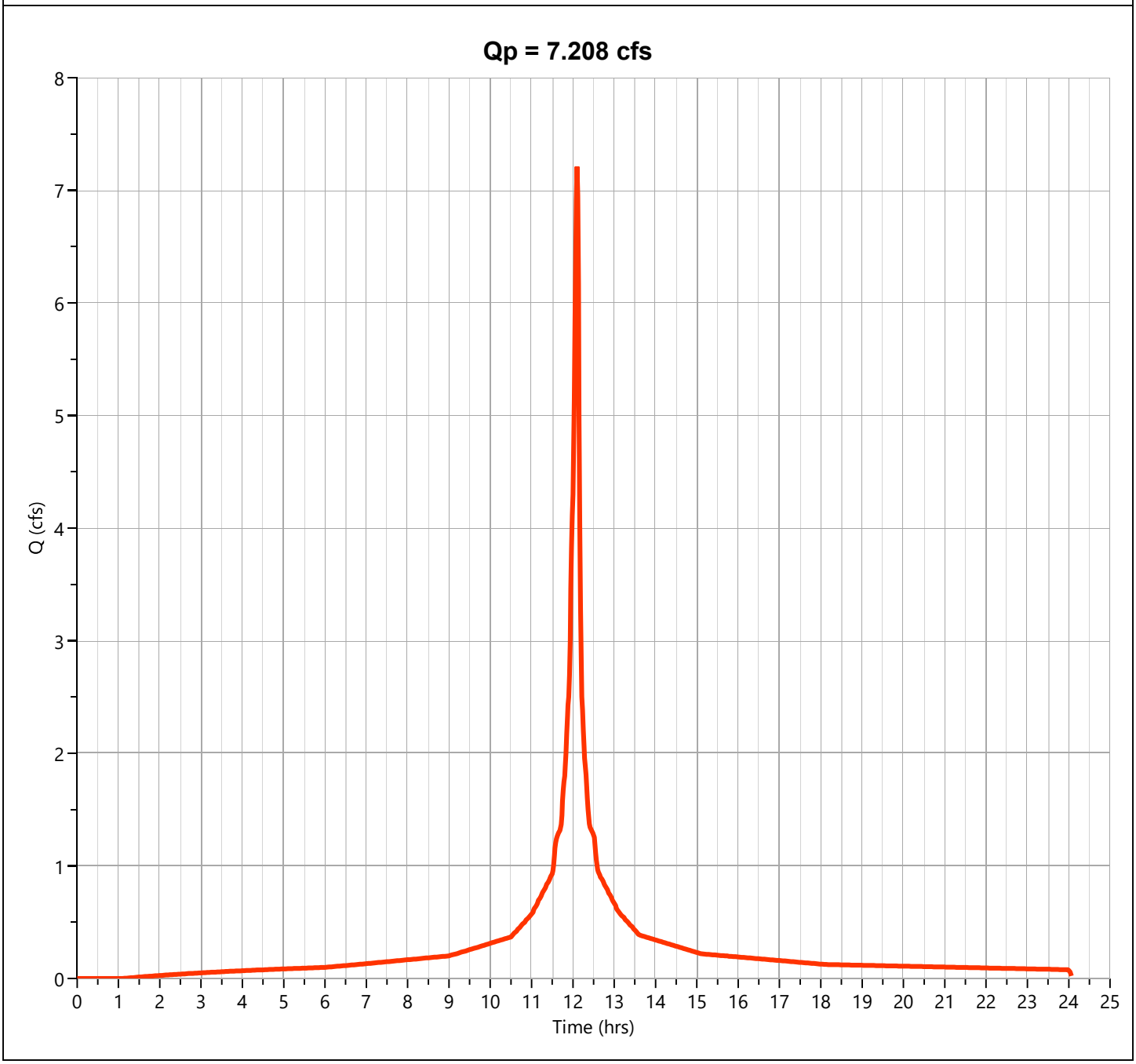
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A4 Imperv

Hyd. No. 7

Hydrograph Type	= NRCS Runoff	Peak Flow	= 7.208 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 23,874 cuft
Drainage Area	= 2.11 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 4.0 min
Total Rainfall	= 3.35 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.38

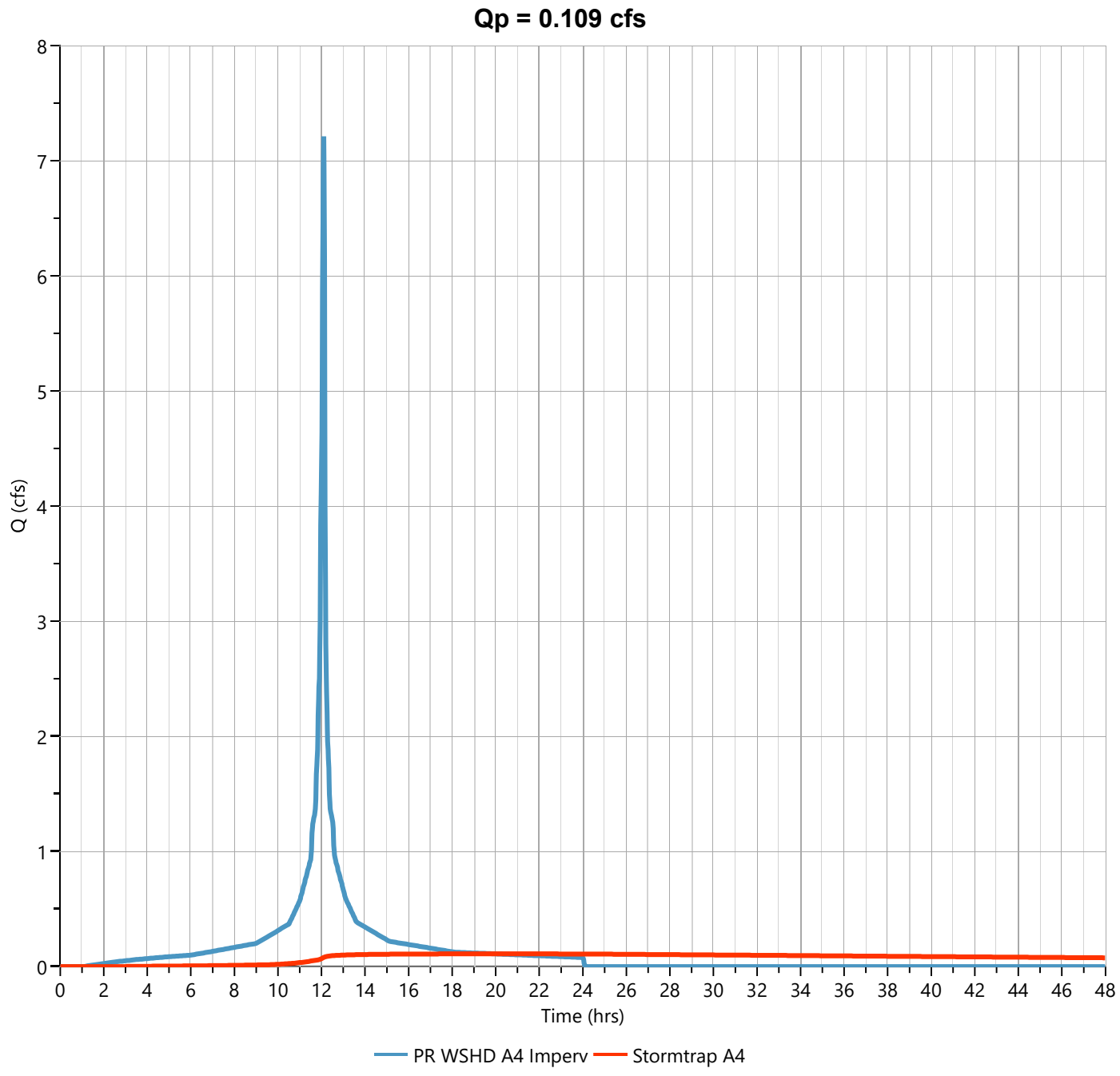
File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

Stormtrap A4 — Hyd. No. 8

Hydrograph Type	= Pond Route	Peak Flow	= 0.109 cfs
Storm Frequency	= 2-yr	Time to Peak	= 20.00 hrs
Time Interval	= 1 min	Hydrograph Volume	= 12,971 cuft
Inflow Hydrograph	= 7 - PR WSHD A4 Imperv	Max. Elevation	= 2.22 ft
Pond Name	= Stormtrap A4	Max. Storage	= 19,040 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 15.75 hrs



Pond Report

File: Proposed HydrologyCurrent Precipitation.hys

Hydrology Studio v 3.0.0.38

07-31-2025

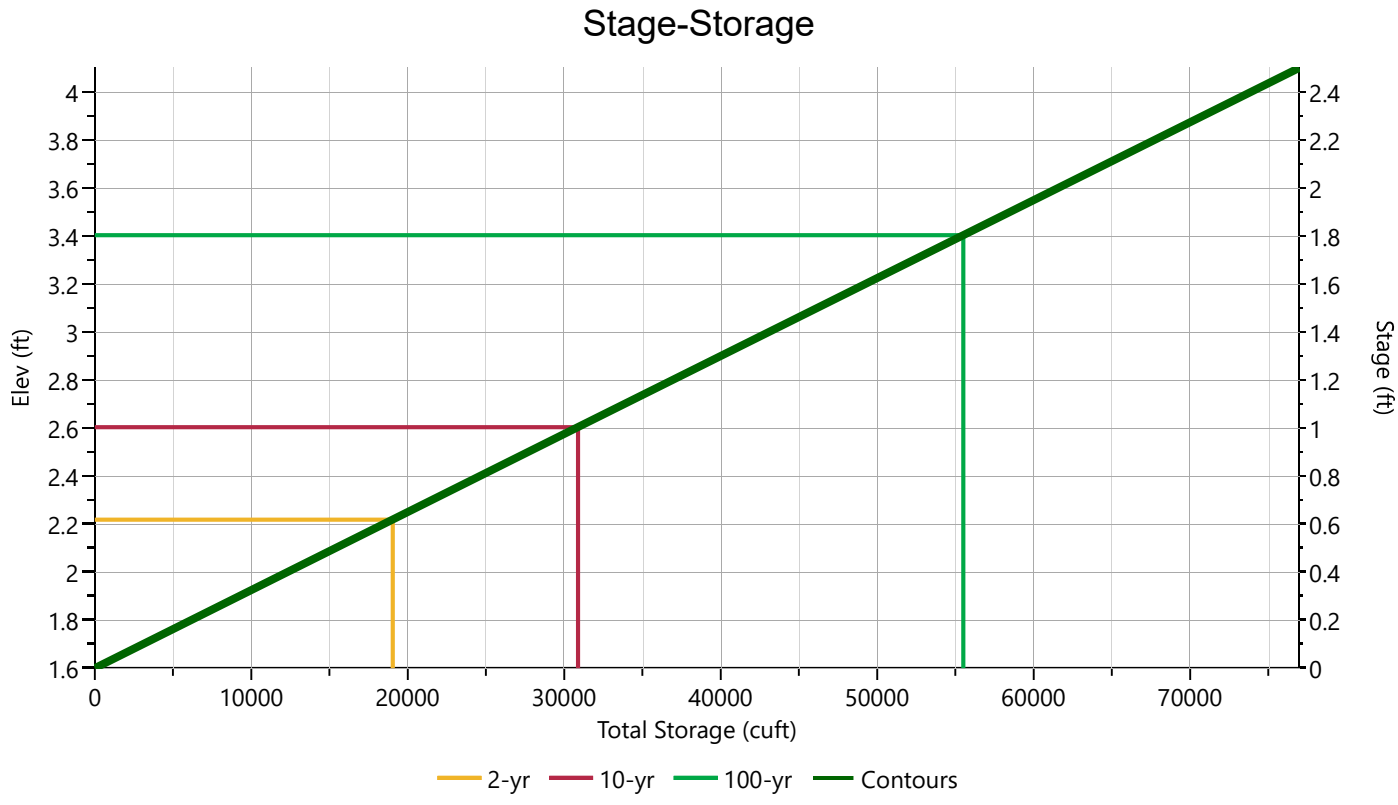
Stormtrap A4

Stage-Storage

User Defined Contours	
Description	**Input**
Bottom Elevation, ft	1.60
Voids (%)	100.00
Volume Calc	Ave End Area

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)	Total Storage (cuft)
0.00	1.60	30,793	0.000	0.000
1.00	2.60	30,793	30,793	30,793
2.00	3.60	30,793	30,793	61,586
2.50	4.10	30,793	15,397	76,983



Pond Report

File: Proposed HydrologyCurrent Precipitation.hys

Hydrology Studio v 3.0.0.38

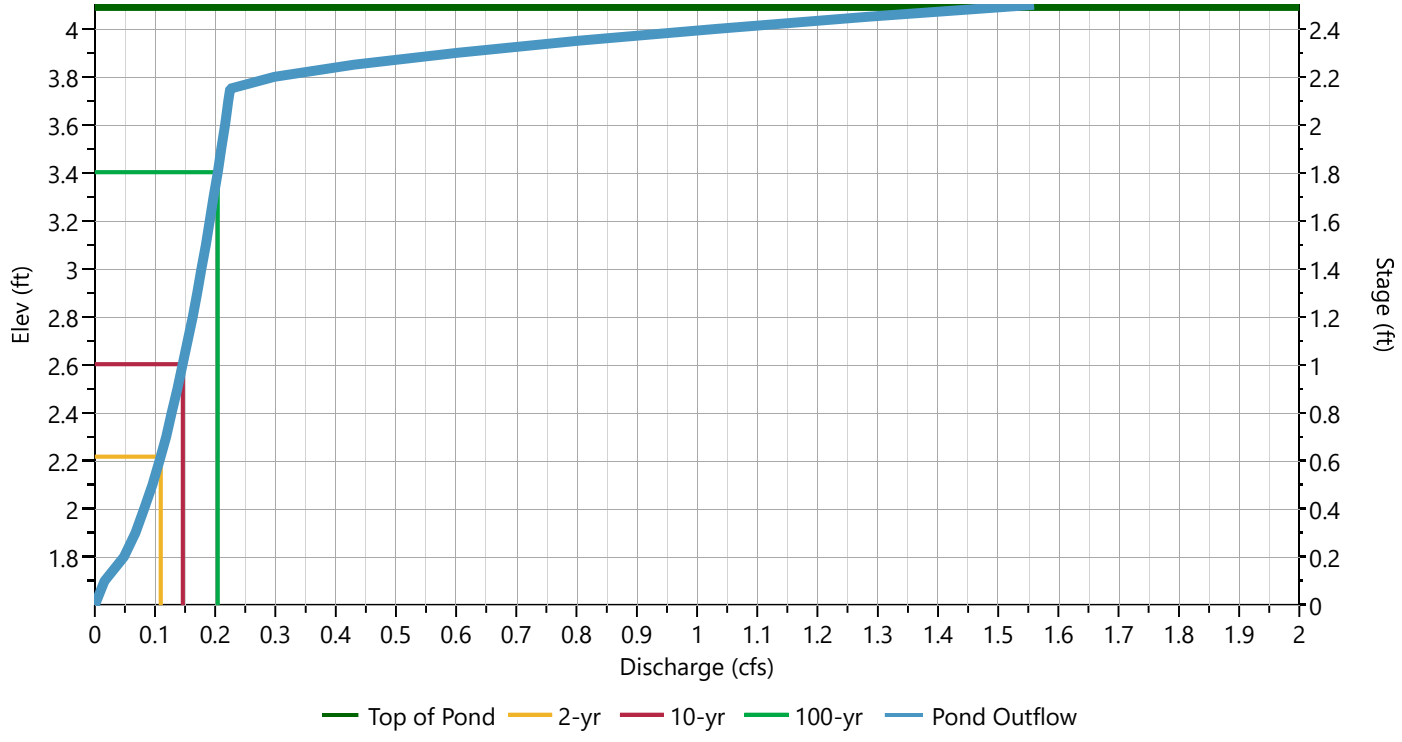
07-31-2025

Stormtrap A4

Stage-Discharge

Culvert / Orifices	Cir Culvert	Orifice			Perforated Riser	
		1 (m)	2	3		
Rise, in	18	2.5			Hole Diameter, in	
Span, in	18	2.5			No. holes	
No. Barrels	1	1			Invert Elevation, ft	
Invert Elevation, ft	1.60	1.60			Height, ft	
Orifice Coefficient, Co	0.60	0.60			Orifice Coefficient, Co	
Length, ft	10					
Barrel Slope, %	.3					
N-Value, n	0.012					
Weirs	**Riser**	**Weir**			**Ancillary**	
		1 (m)	2	3		
Shape / Type		Rectangular			Exfiltration, in/hr	
Crest Elevation, ft		3.75				
Crest Length, ft		2				
Angle, deg						
Weir Coefficient, Cw		3.3				

m = Flows through Culvert, i = Independent



Pond Report

File: Proposed HydrologyCurrent Precipitation.hys

Hydrology Studio v 3.0.0.38

07-31-2025

Stormtrap A4

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.00	1.60	0.000	0.000	0.000				0.000						0.000
1.00	2.60	30,793	0.146 oc	0.146				0.000						0.146
2.00	3.60	61,586	0.216 oc	0.216				0.000						0.216
2.50	4.10	76,983	1.559 oc	0.193				1.367						1.559

Suffix key: ic = inlet control, oc = outlet control, s = submerged weir

Pond Report

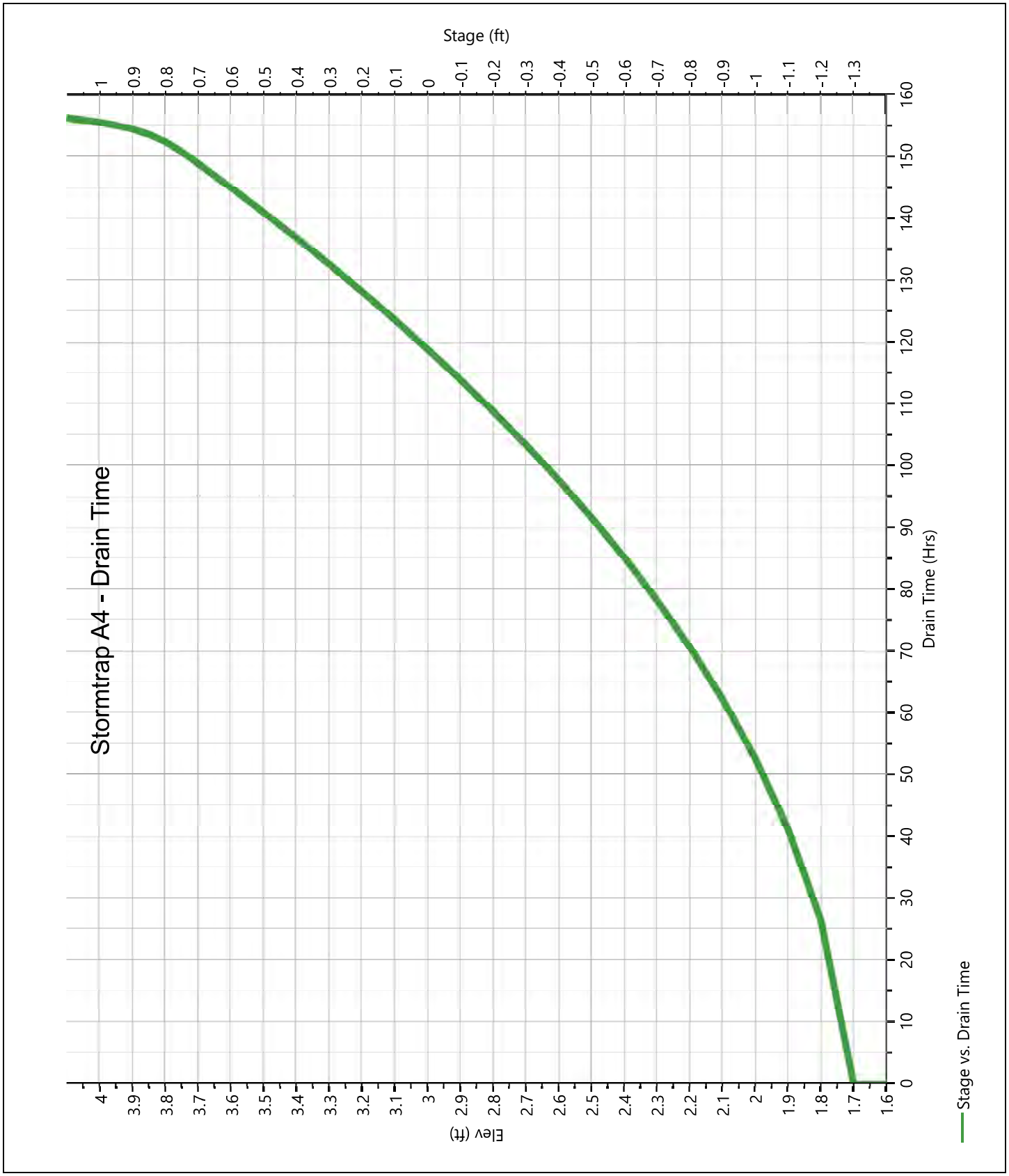
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Hydrology Studio v 3.0.0.38

07-31-2025

Stormtrap A4

Pond Drawdown



Hydrograph Report

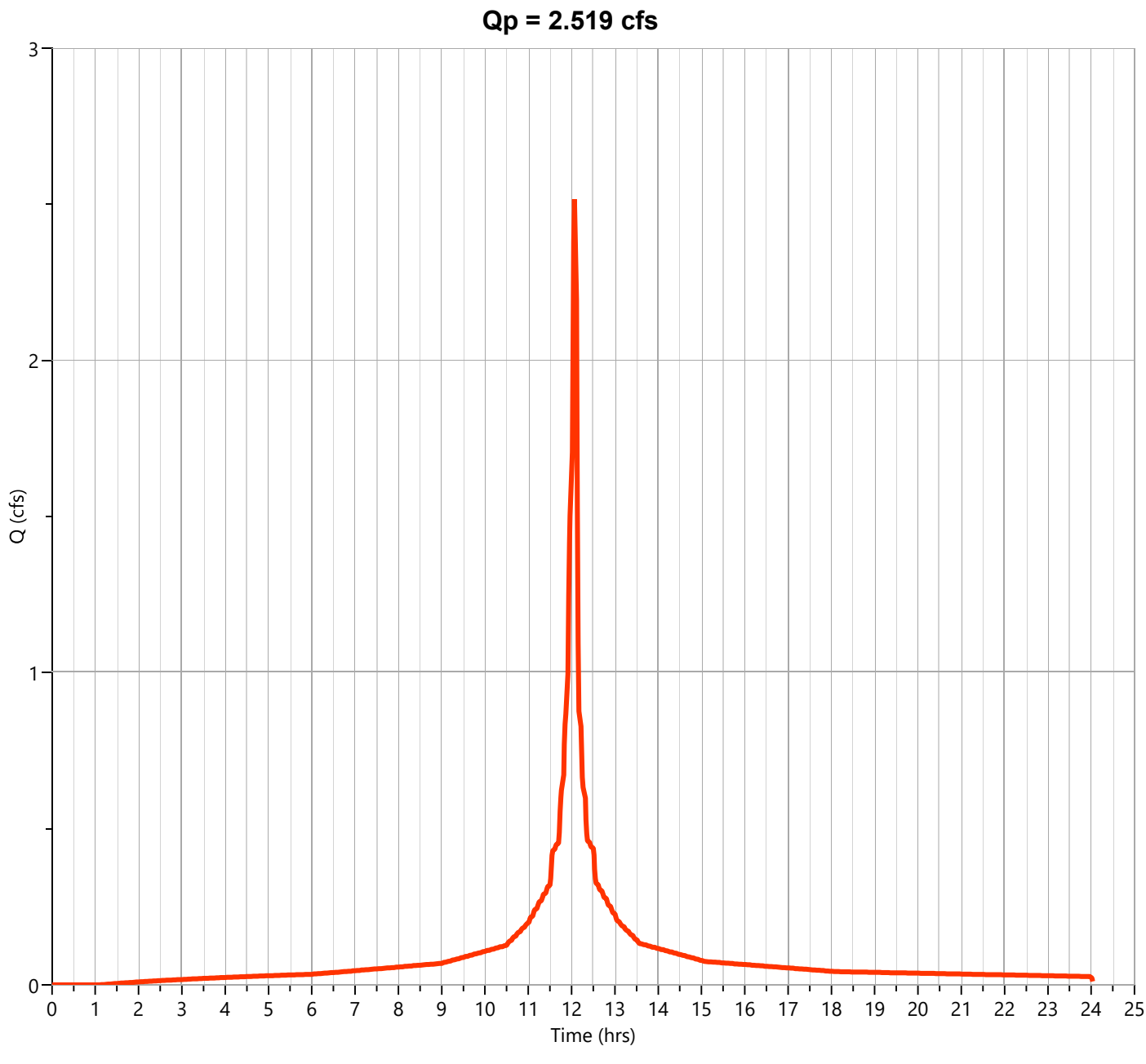
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A5 Imperv

Hyd. No. 10

Hydrograph Type	= NRCS Runoff	Peak Flow	= 2.519 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 8,168 cuft
Drainage Area	= 0.77 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 3.35 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

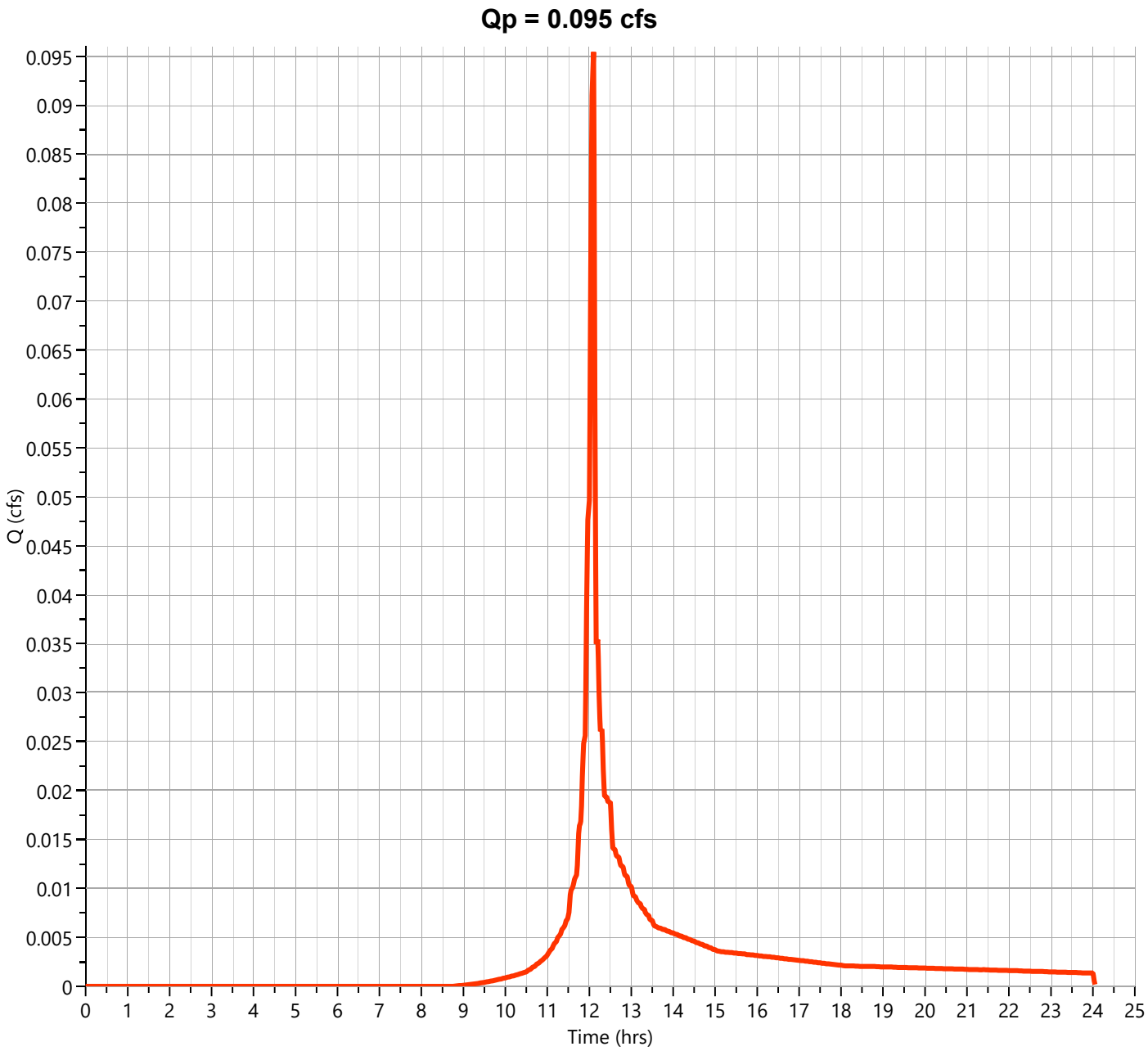
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A5 Perv

Hyd. No. 11

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.095 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 258 cuft
Drainage Area	= 0.05 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 3.35 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

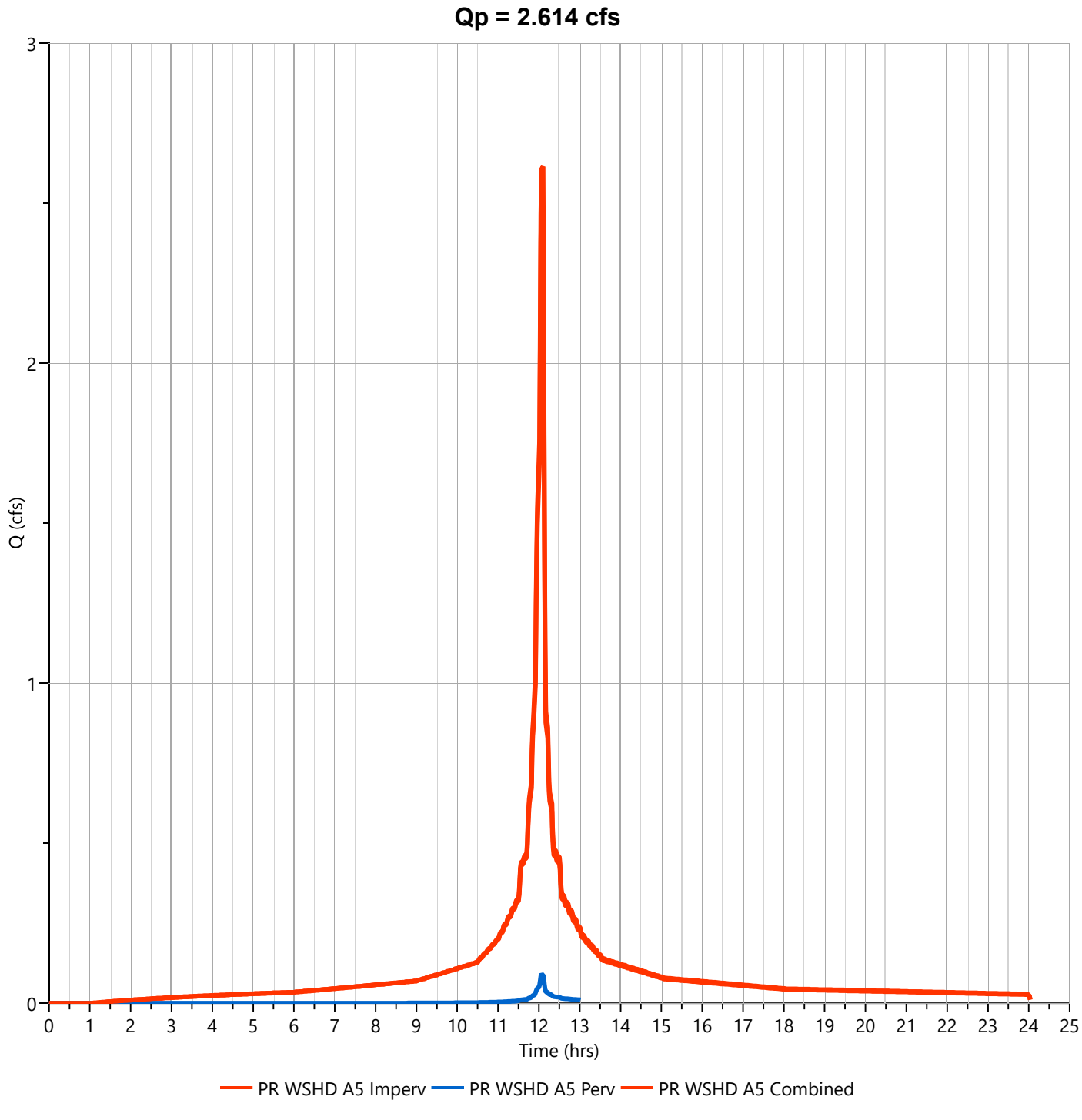
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A5 Combined

Hyd. No. 12

Hydrograph Type	= Junction	Peak Flow	= 2.614 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Hydrograph Volume	= 8,426 cuft
Inflow Hydrographs	= 10, 11	Total Contrib. Area	= 0.82 ac



Hydrograph Report

Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

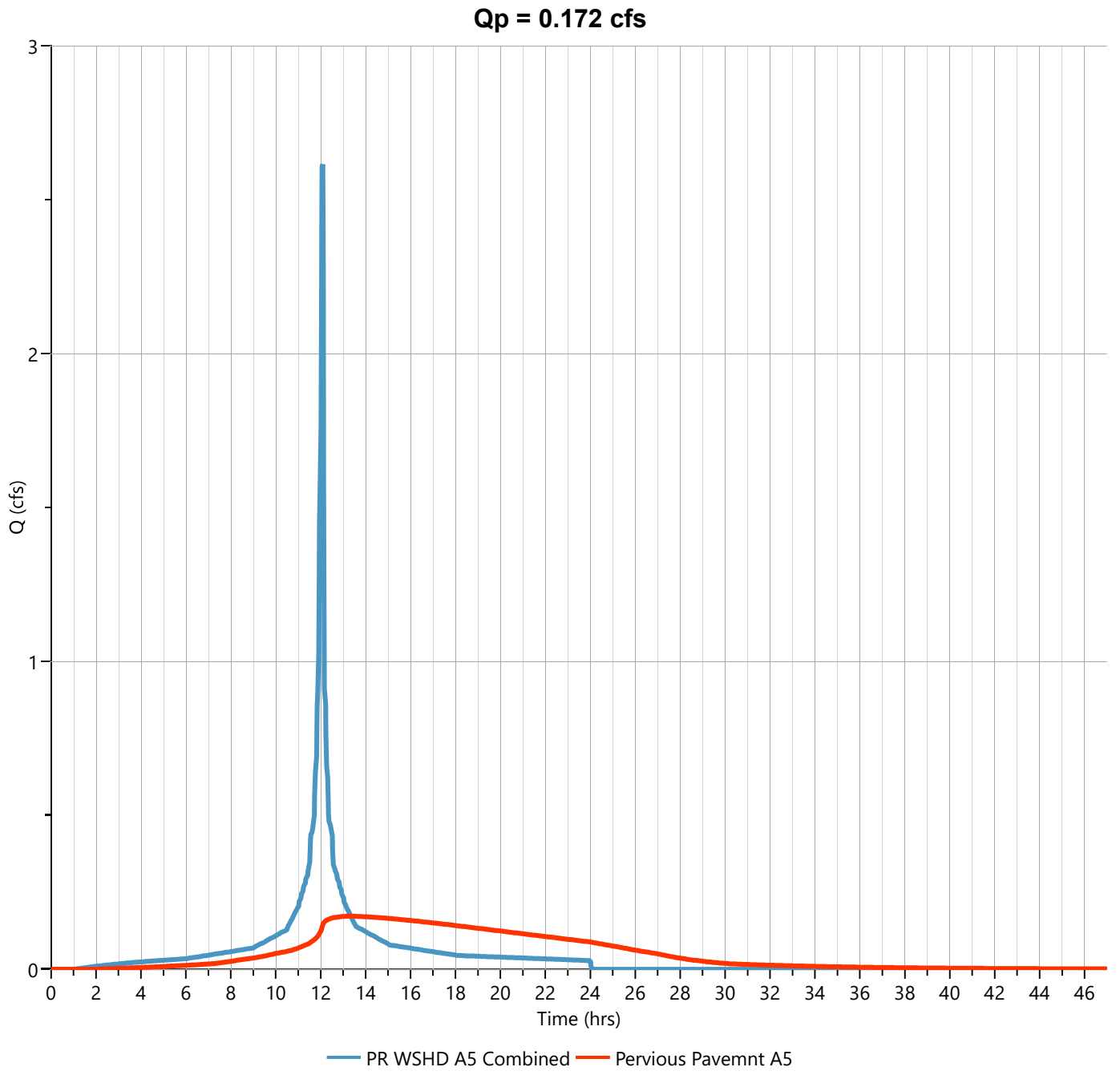
Pervious Pavemnt A5

Hyd. No. 13

Hydrograph Type	= Pond Route	Peak Flow	= 0.172 cfs
Storm Frequency	= 2-yr	Time to Peak	= 13.33 hrs
Time Interval	= 1 min	Hydrograph Volume	= 8,405 cuft
Inflow Hydrograph	= 12 - PR WSHD A5 Combined	Max. Elevation	= 4.60 ft
Pond Name	= Pervious Pavement A5	Max. Storage	= 4,464 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 5.68 hrs



Pond Report

File: Proposed HydrologyCurrent Precipitation.hys

Hydrology Studio v 3.0.0.38

07-31-2025

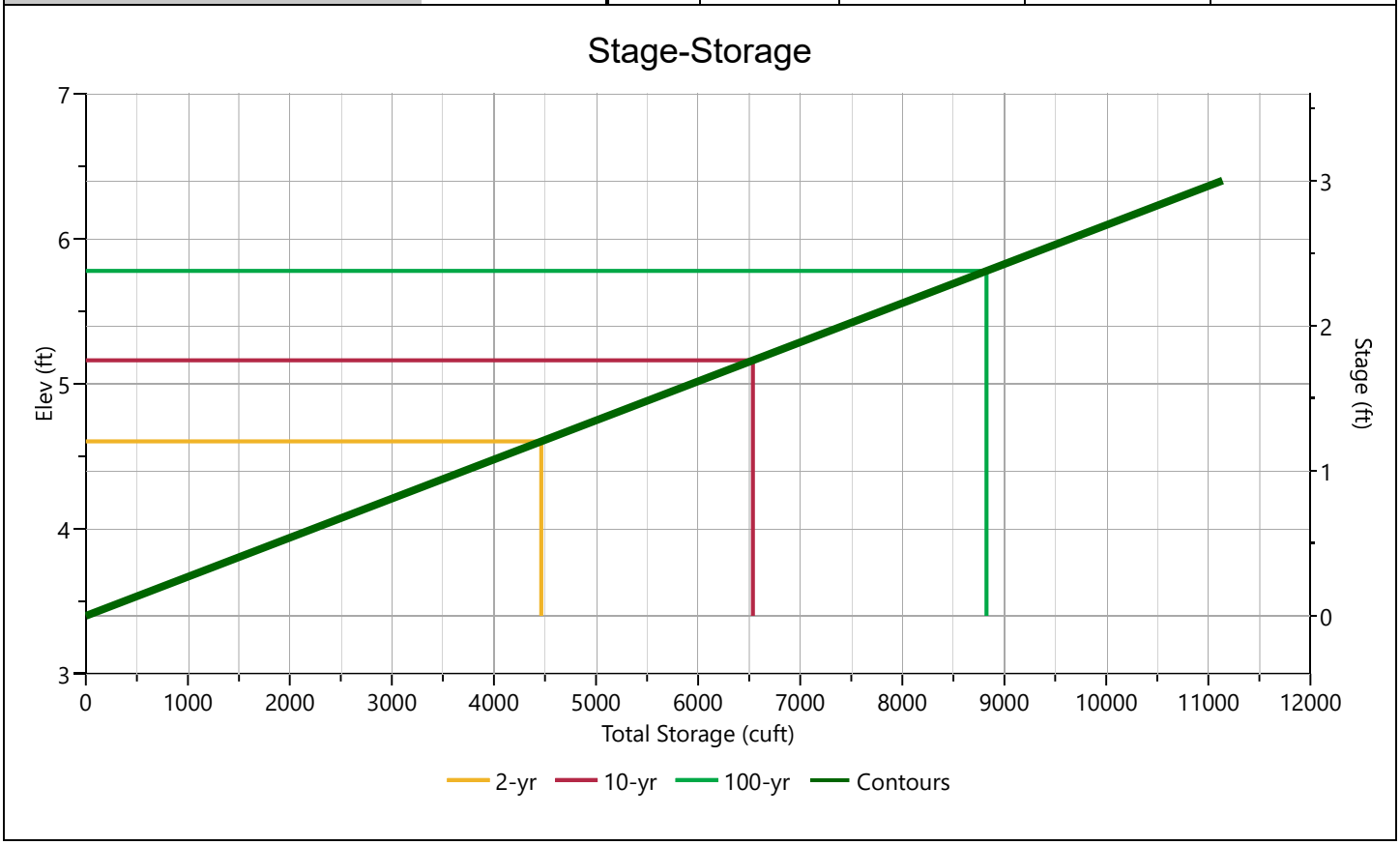
Pervious Pavement A5

Stage-Storage

User Defined Contours	
Description	**Input**
Bottom Elevation, ft	3.40
Voids (%)	40.00
Volume Calc	Ave End Area

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)	Total Storage (cuft)
0.00	3.40	9,282	0.000	0.000
1.00	4.40	9,282	3,713	3,713
2.00	5.40	9,282	3,713	7,426
3.00	6.40	9,282	3,713	11,138



Pond Report

File: Proposed HydrologyCurrent Precipitation.hys

Hydrology Studio v 3.0.0.38

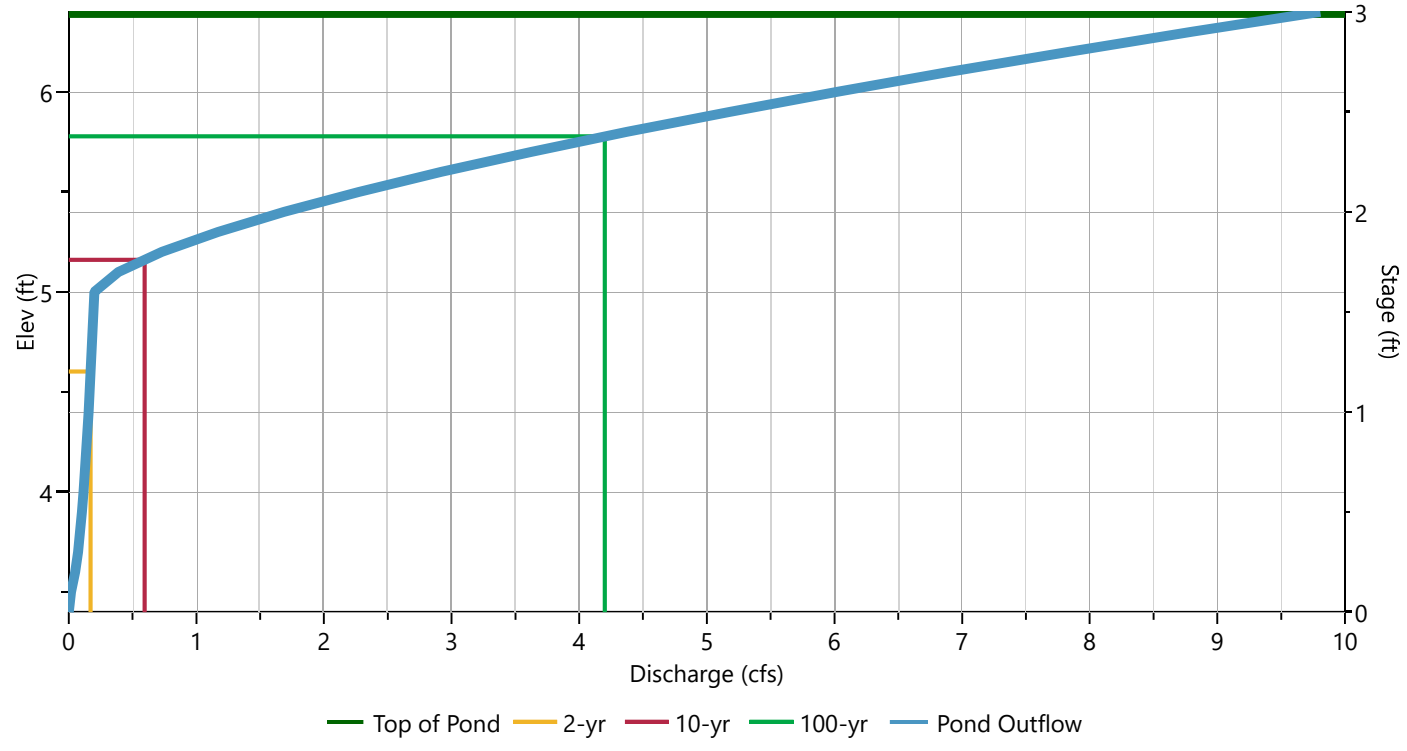
07-31-2025

Pervious Pavement A5

Stage-Discharge

Culvert / Orifices	Cir Culvert	Orifice			Perforated Riser	
		1 (m)	2	3		
Rise, in	18	2.5			Hole Diameter, in	
Span, in	18	2.5			No. holes	
No. Barrels	1	1			Invert Elevation, ft	
Invert Elevation, ft	2.00	3.40			Height, ft	
Orifice Coefficient, Co	0.60	0.60			Orifice Coefficient, Co	
Length, ft	10					
Barrel Slope, %	.3					
N-Value, n	0.012					
Weirs	**Riser**	**Weir**			**Ancillary**	
		1 (m)	2	3		
Shape / Type		Rectangular			Exfiltration, in/hr	
Crest Elevation, ft		5				
Crest Length, ft		1.75				
Angle, deg						
Weir Coefficient, Cw		3.3				

m = Flows through Culvert, i = Independent



Pond Report

File: Proposed HydrologyCurrent Precipitation.hys

Hydrology Studio v 3.0.0.38

07-31-2025

Pervious Pavement A5

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.00	3.40	0.000	0.000	0.000				0.000						0.000
1.00	4.40	3,713	0.155 oc	0.155				0.000						0.155
2.00	5.40	7,426	1.687 oc	0.226				1.461						1.687
3.00	6.40	11,138	9.806 oc	0.240				9.566						9.806

Suffix key: ic = inlet control, oc = outlet control, s = submerged weir

Pond Report

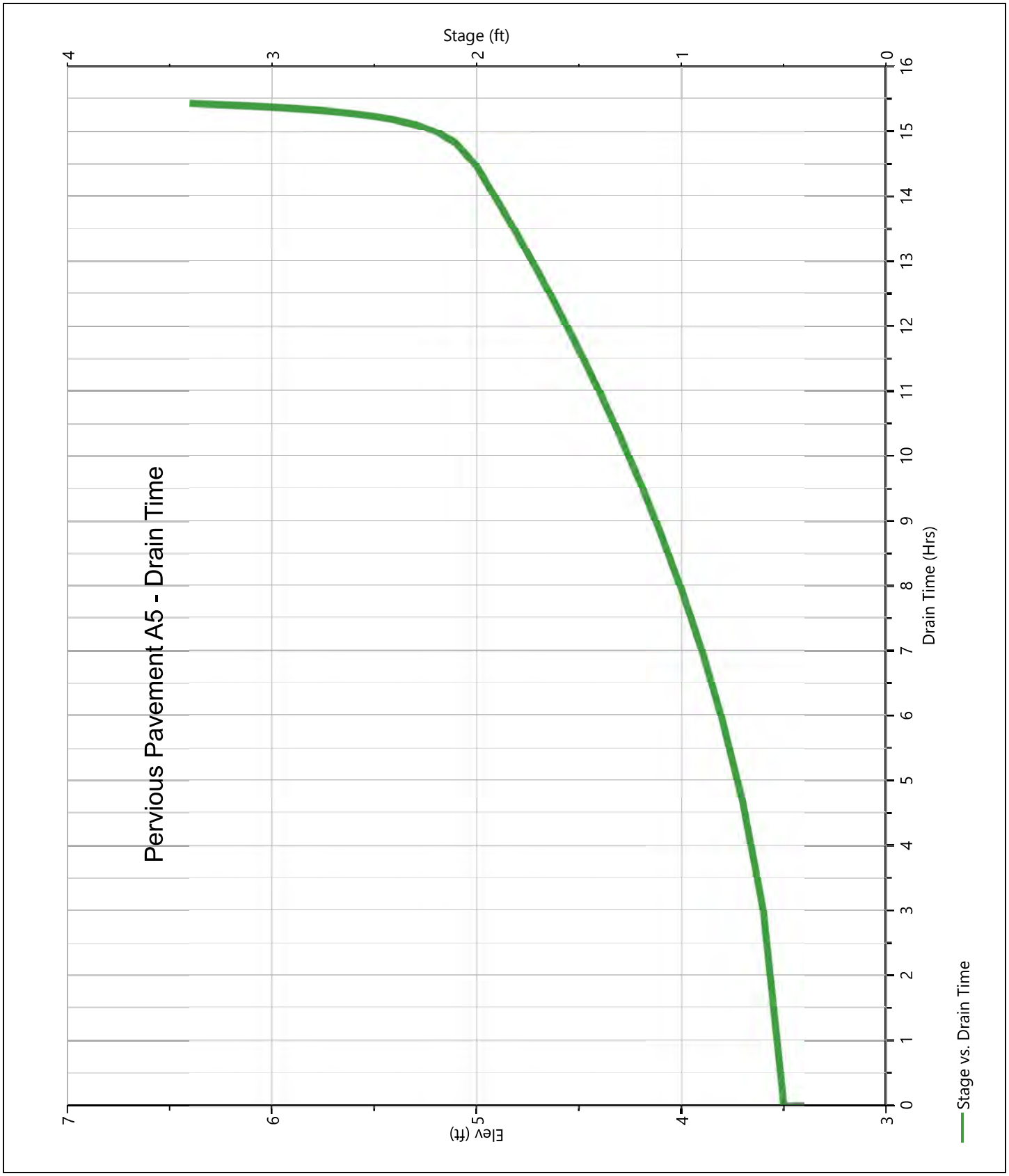
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Hydrology Studio v 3.0.0.38

07-31-2025

Pervious Pavement A5

Pond Drawdown



Hydrograph Report

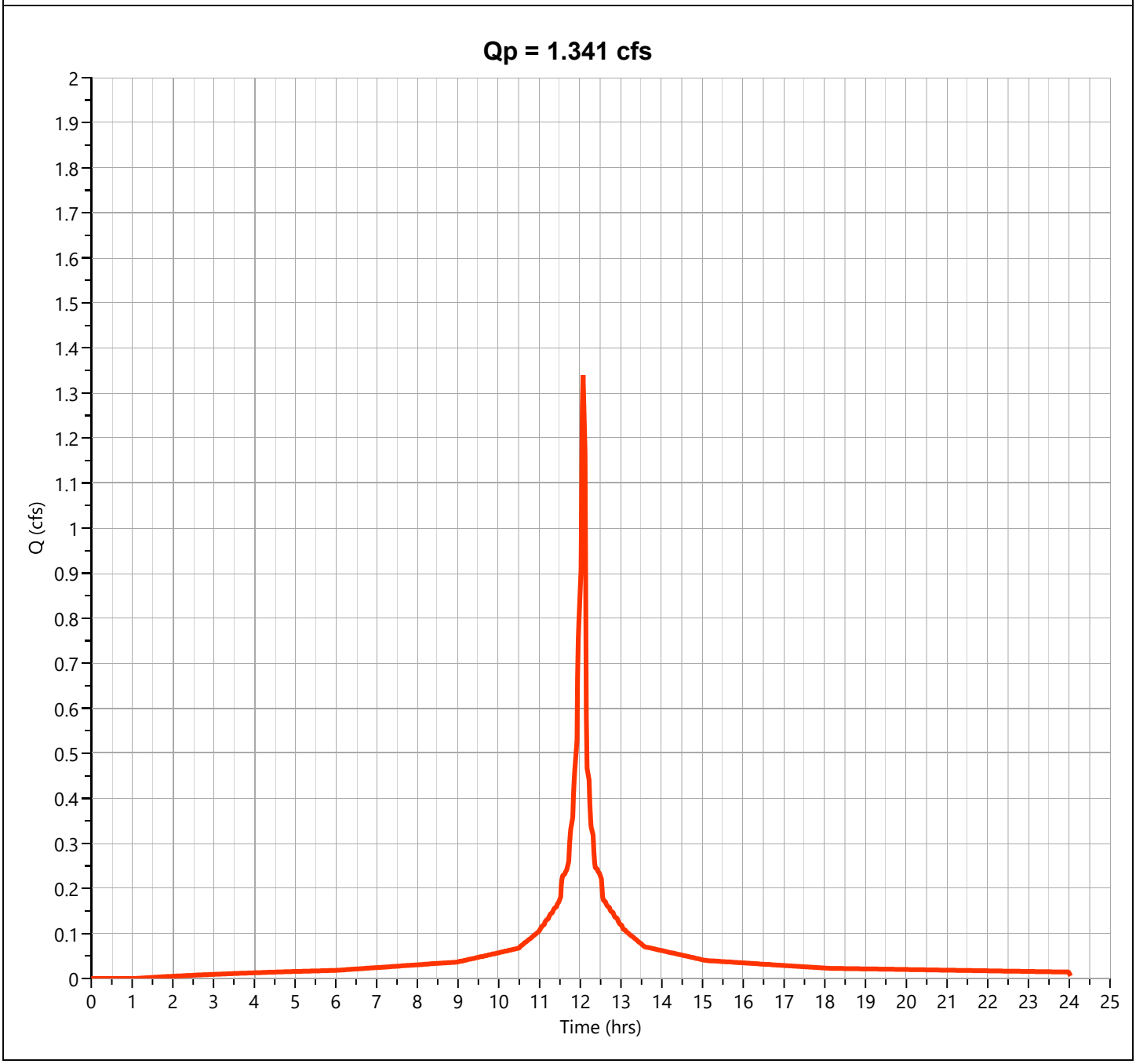
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A6 Imperv

Hyd. No. 15

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.341 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 4,349 cuft
Drainage Area	= 0.41 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 3.35 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

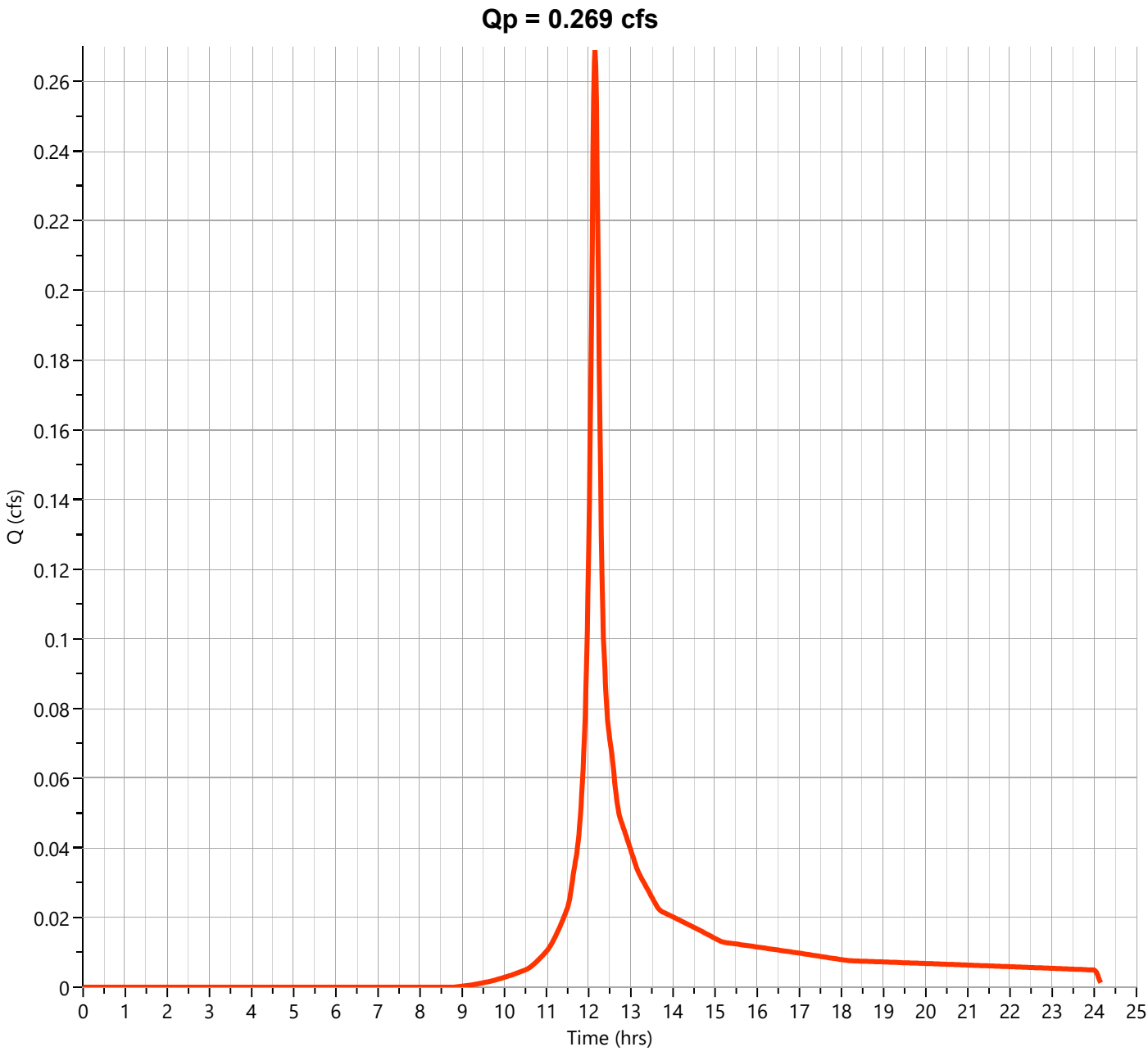
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A6 Perv

Hyd. No. 16

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.269 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.15 hrs
Time Interval	= 1 min	Runoff Volume	= 937 cuft
Drainage Area	= 0.17 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 9.0 min
Total Rainfall	= 3.35 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

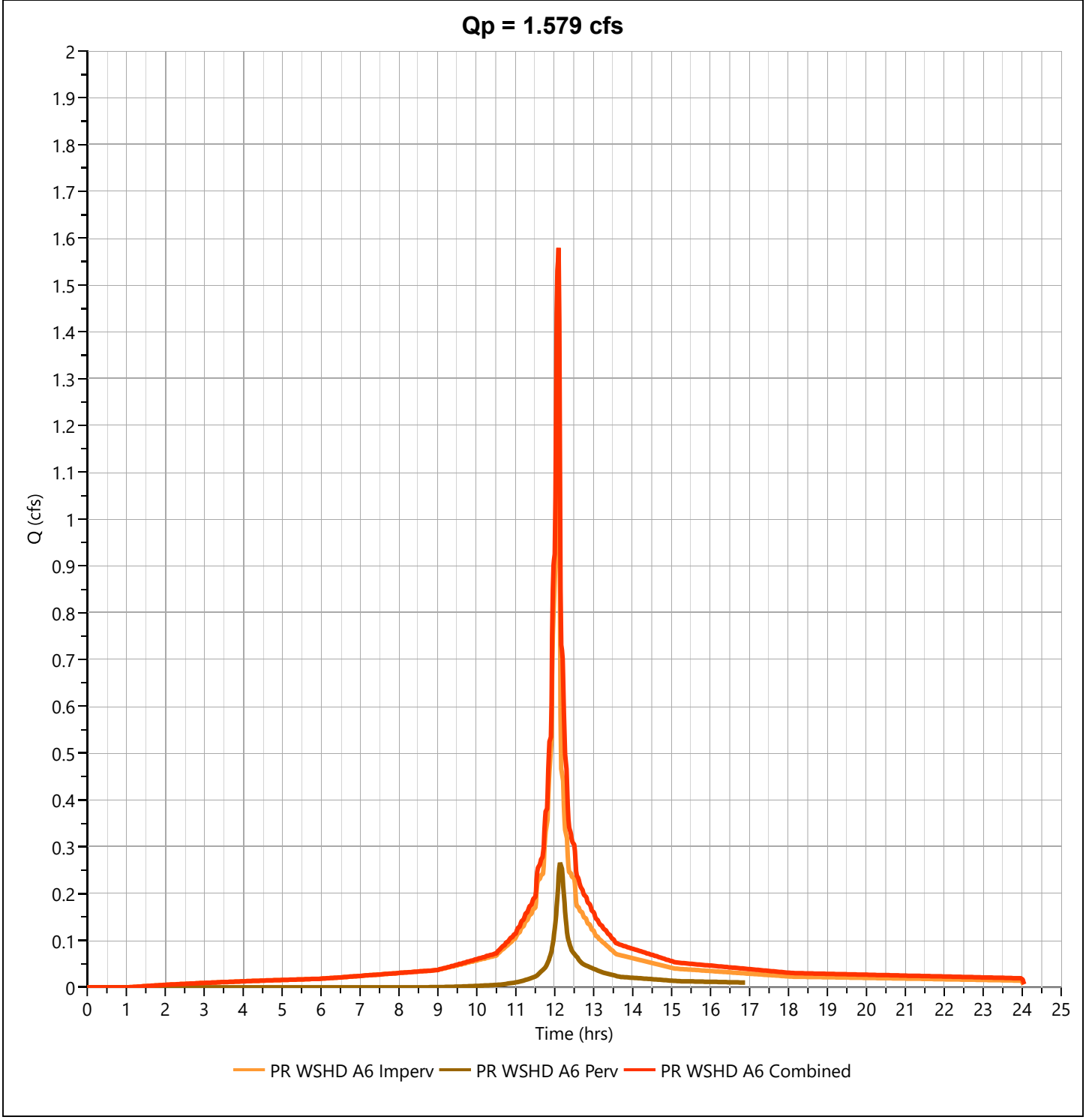
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A6 Combined

Hyd. No. 17

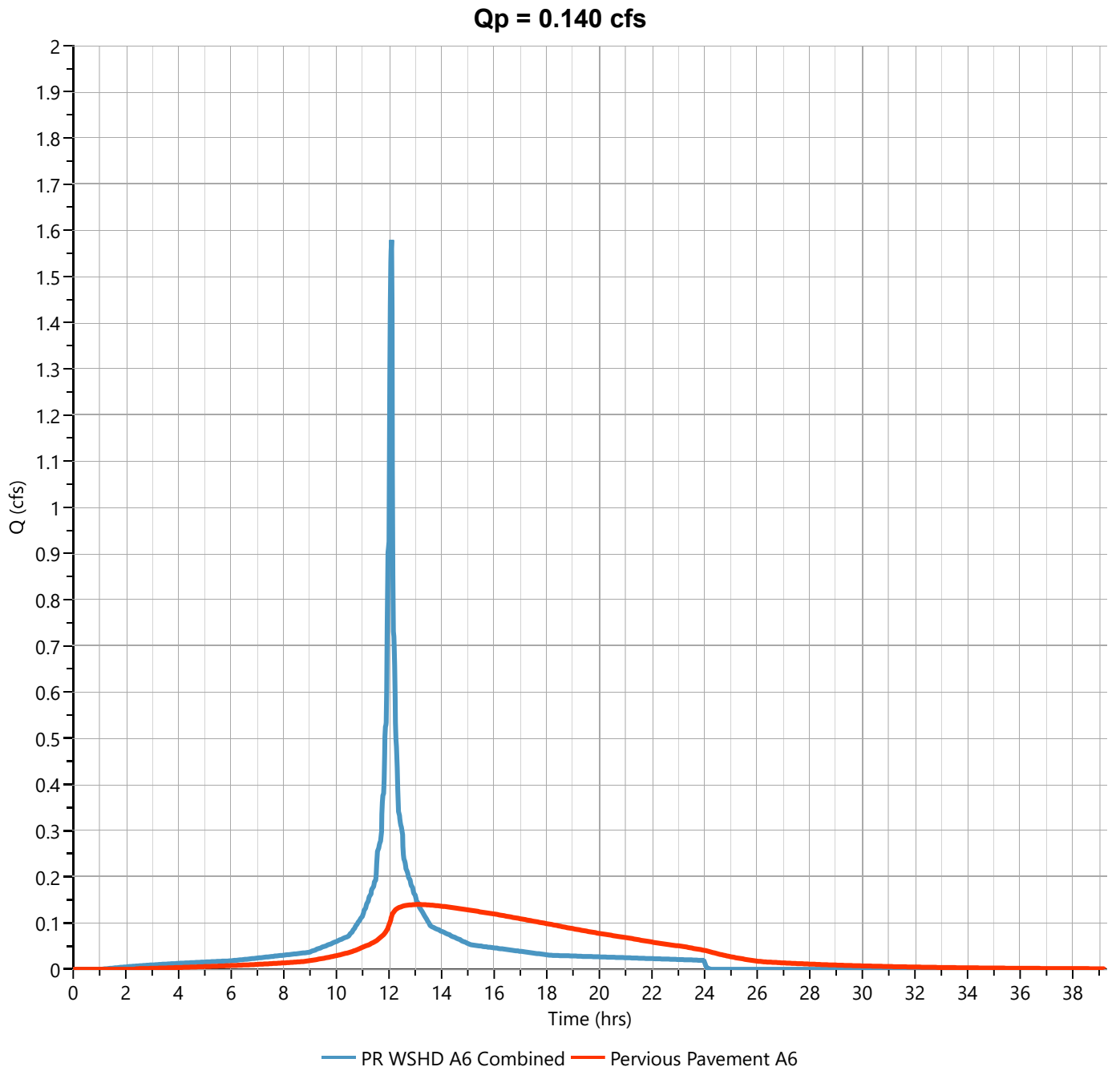
Hydrograph Type	= Junction	Peak Flow	= 1.579 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Hydrograph Volume	= 5,286 cuft
Inflow Hydrographs	= 15, 16	Total Contrib. Area	= 0.58 ac



Pervious Pavement A6 — Hyd. No. 18

Hydrograph Type	= Pond Route	Peak Flow	= 0.140 cfs
Storm Frequency	= 2-yr	Time to Peak	= 13.12 hrs
Time Interval	= 1 min	Hydrograph Volume	= 5,269 cuft
Inflow Hydrograph	= 17 - PR WSHD A6 Combined	Max. Elevation	= 3.44 ft
Pond Name	= Pervious Pavement A6	Max. Storage	= 2,546 cuft

Pond Routing by Storage Indication Method — *Center of mass detention time = 4.11 hrs*



Pond Report

File: Proposed HydrologyCurrent Precipitation.hys

Hydrology Studio v 3.0.0.38

07-31-2025

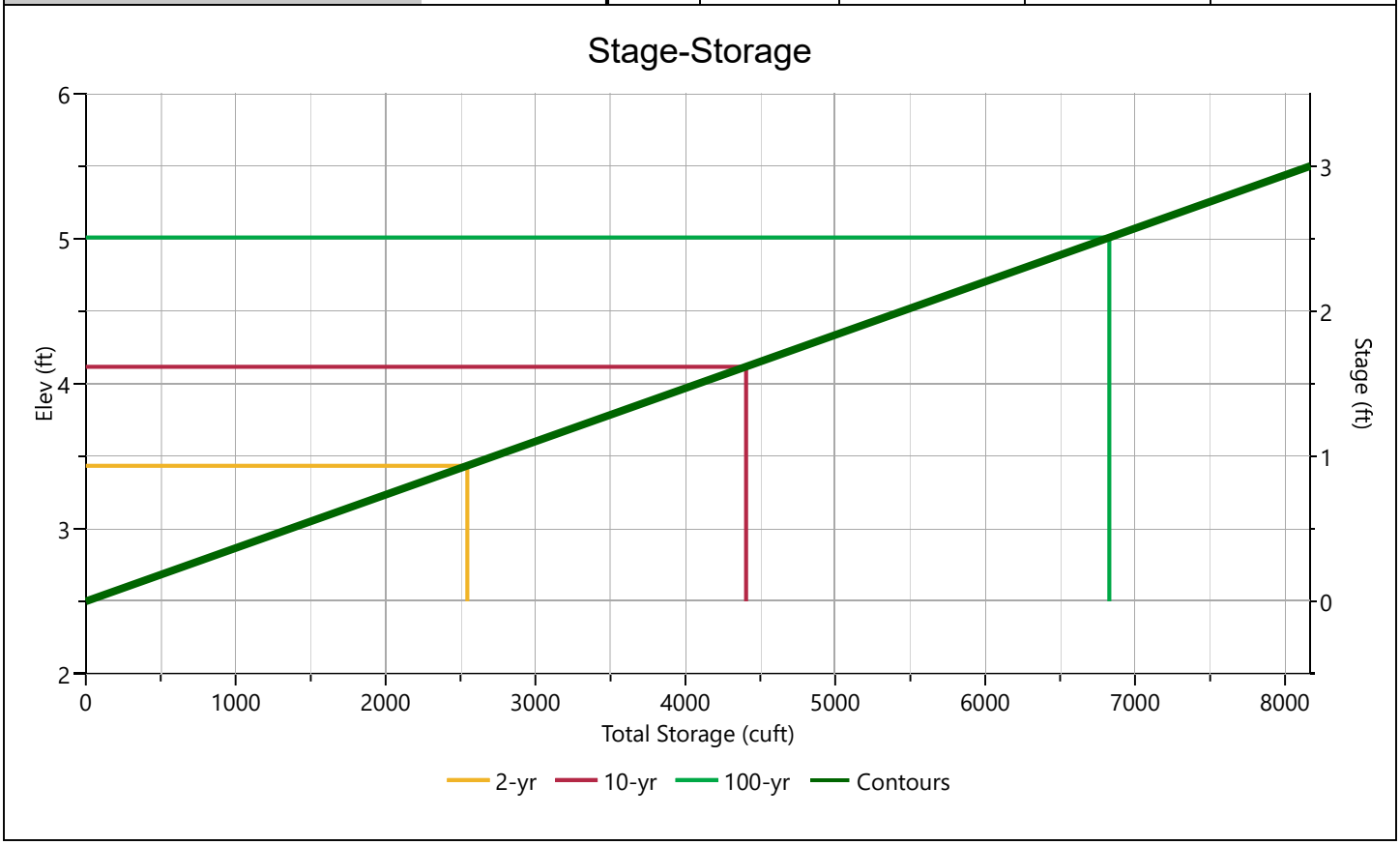
Pervious Pavement A6

Stage-Storage

User Defined Contours	
Description	**Input**
Bottom Elevation, ft	2.50
Voids (%)	40.00
Volume Calc	Ave End Area

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)	Total Storage (cuft)
0.00	2.50	6,808	0.000	0.000
1.00	3.50	6,808	2,723	2,723
2.00	4.50	6,808	2,723	5,446
3.00	5.50	6,808	2,723	8,170



Pond Report

File: Proposed HydrologyCurrent Precipitation.hys

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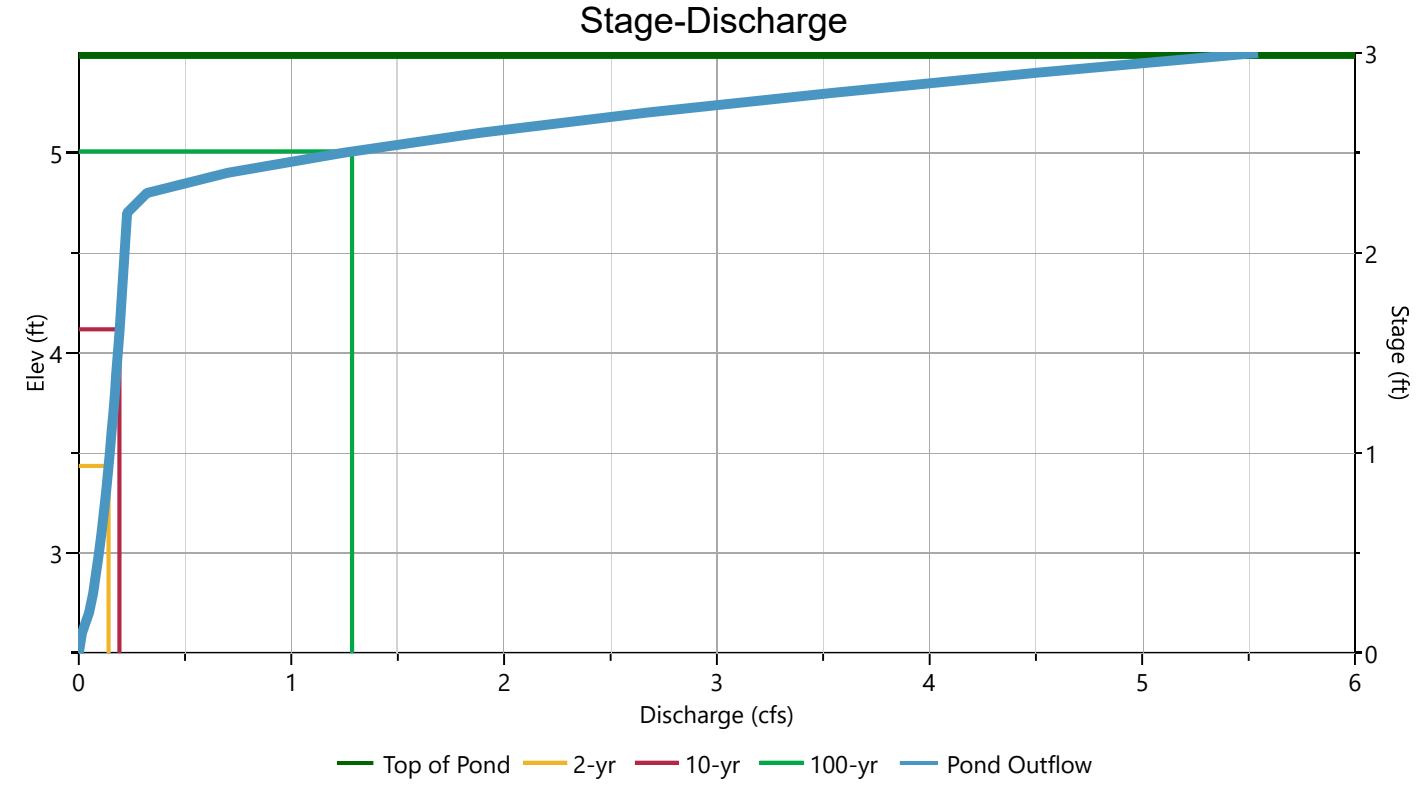
07-31-2025

Pervious Pavement A6

Stage-Discharge

Culvert / Orifices	Cir Culvert	Orifice			Perforated Riser	
		1 (m)	2	3		
Rise, in	18	2.5			Hole Diameter, in	
Span, in	18	2.5			No. holes	
No. Barrels	1	1			Invert Elevation, ft	
Invert Elevation, ft	2.50	2.50			Height, ft	
Orifice Coefficient, Co	0.60	0.60			Orifice Coefficient, Co	
Length, ft	10					
Barrel Slope, %	.3					
N-Value, n	0.012					
Weirs	**Riser**	**Weir**			**Ancillary**	
		1 (m)	2	3		
Shape / Type		Rectangular			Exfiltration, in/hr	
Crest Elevation, ft		4.75				
Crest Length, ft		2.5				
Angle, deg						
Weir Coefficient, Cw		3.3				

m = Flows through Culvert, i = Independent



Pond Report

File: Proposed HydrologyCurrent Precipitation.hys

Hydrology Studio v 3.0.0.38

07-31-2025

Pervious Pavement A6

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.00	2.50	0.000	0.000	0.000				0.000						0.000
1.00	3.50	2,723	0.146 oc	0.146				0.000						0.146
2.00	4.50	5,446	0.216 oc	0.216				0.000						0.216
3.00	5.50	8,170	5.544 oc	0.185				5.359						5.544

Suffix key: ic = inlet control, oc = outlet control, s = submerged weir

Pond Report

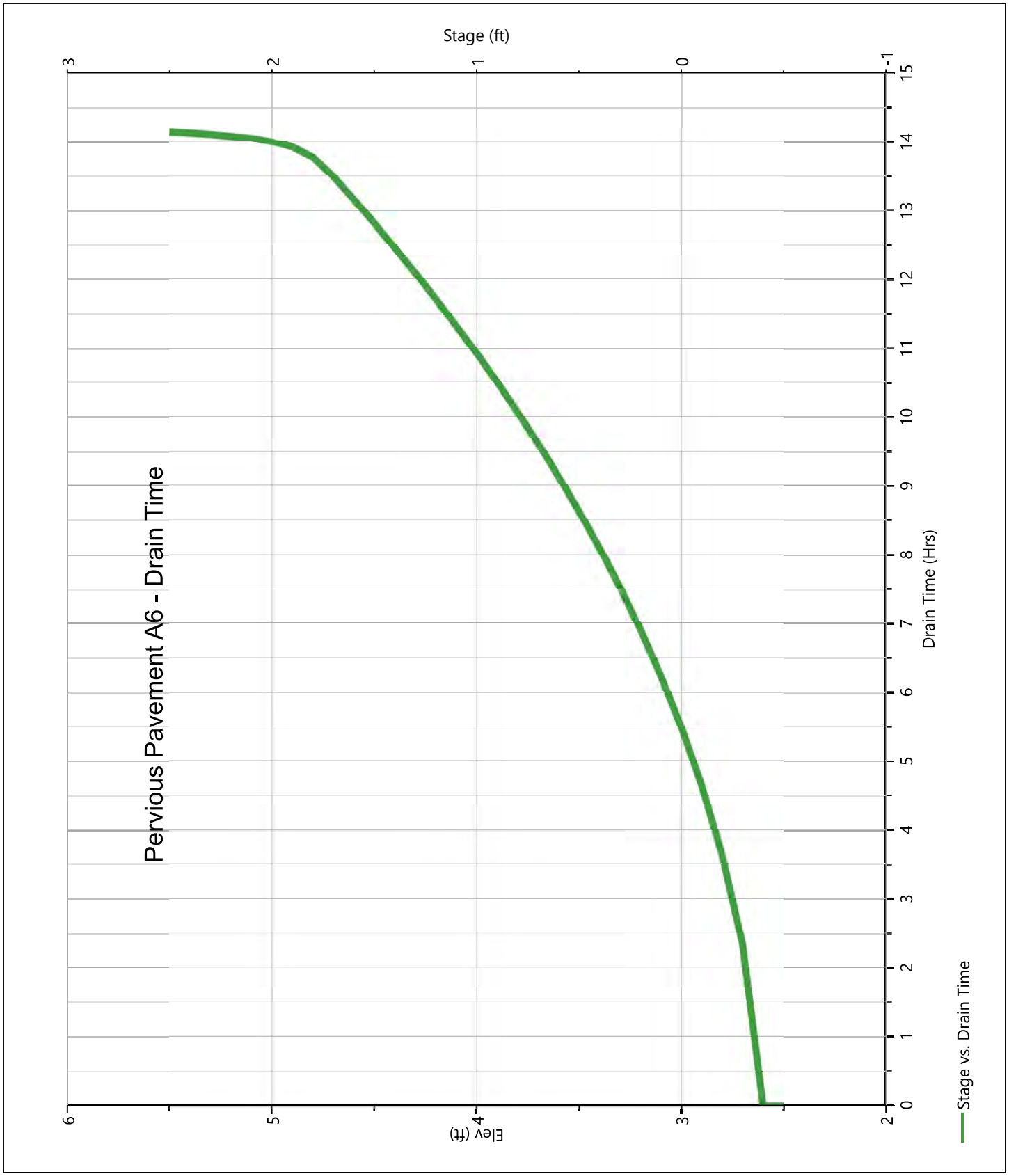
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Hydrology Studio v 3.0.0.38

07-31-2025

Pervious Pavement A6

Pond Drawdown



Hydrograph Report

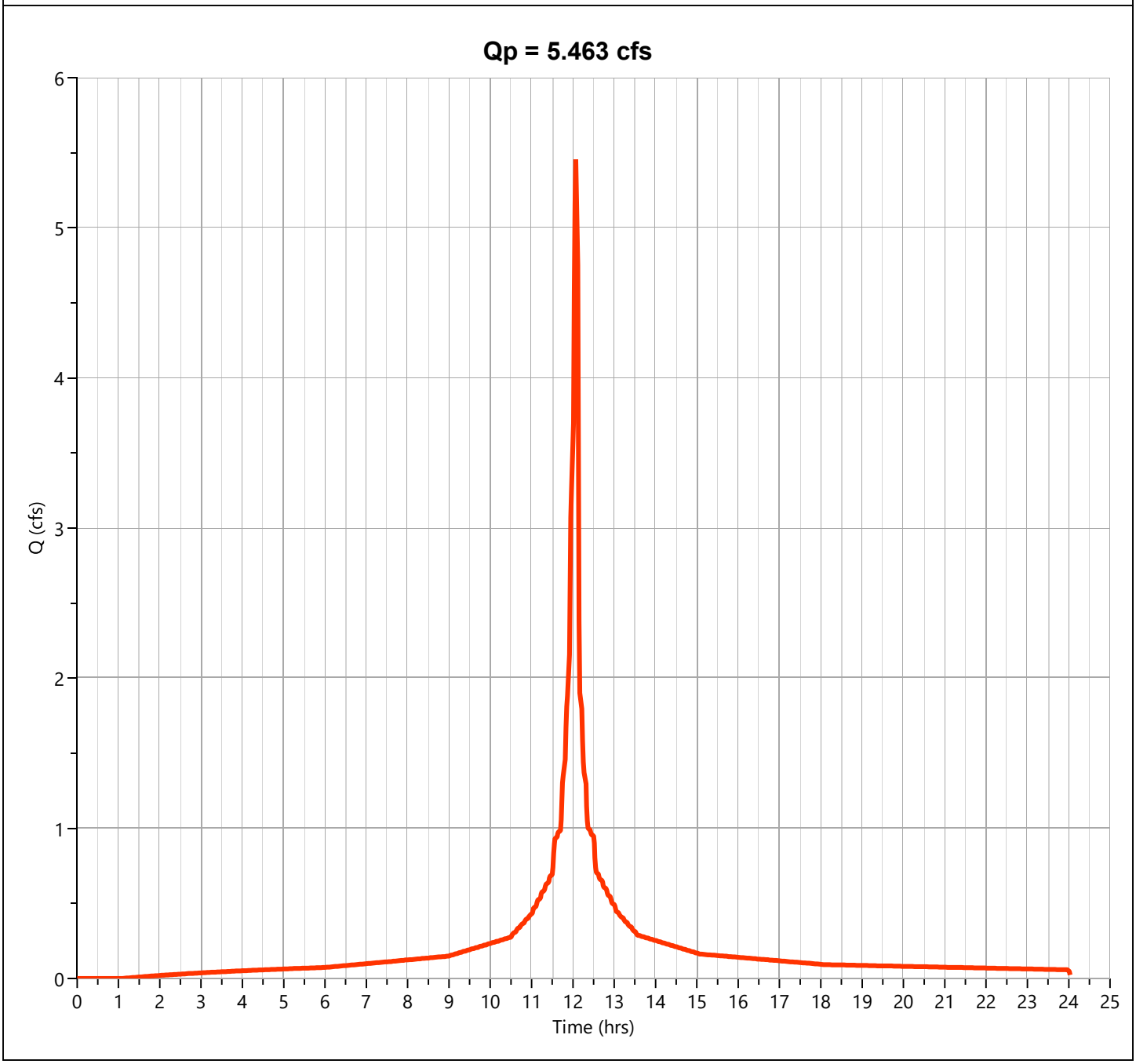
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A7 Imperv

Hyd. No. 20

Hydrograph Type	= NRCS Runoff	Peak Flow	= 5.463 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 17,714 cuft
Drainage Area	= 1.67 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 3.35 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

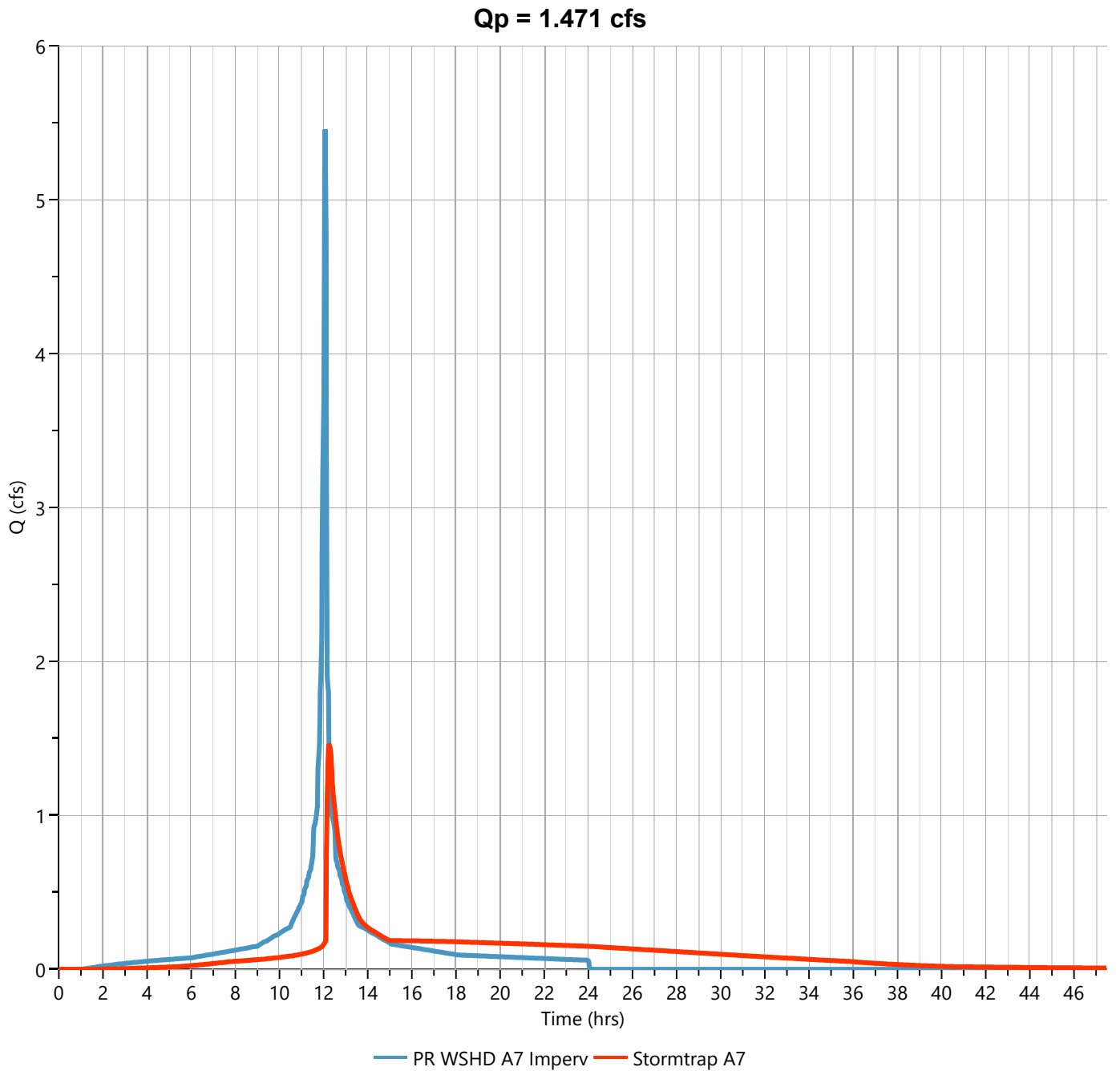
Stormtrap A7

Hyd. No. 21

Hydrograph Type	= Pond Route	Peak Flow	= 1.471 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.25 hrs
Time Interval	= 1 min	Hydrograph Volume	= 17,505 cuft
Inflow Hydrograph	= 20 - PR WSHD A7 Imperv	Max. Elevation	= 4.63 ft
Pond Name	= Stormtrap A7	Max. Storage	= 8,374 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 6.83 hrs



Pond Report

File: Proposed HydrologyCurrent Precipitation.hys

Hydrology Studio v 3.0.0.38

07-31-2025

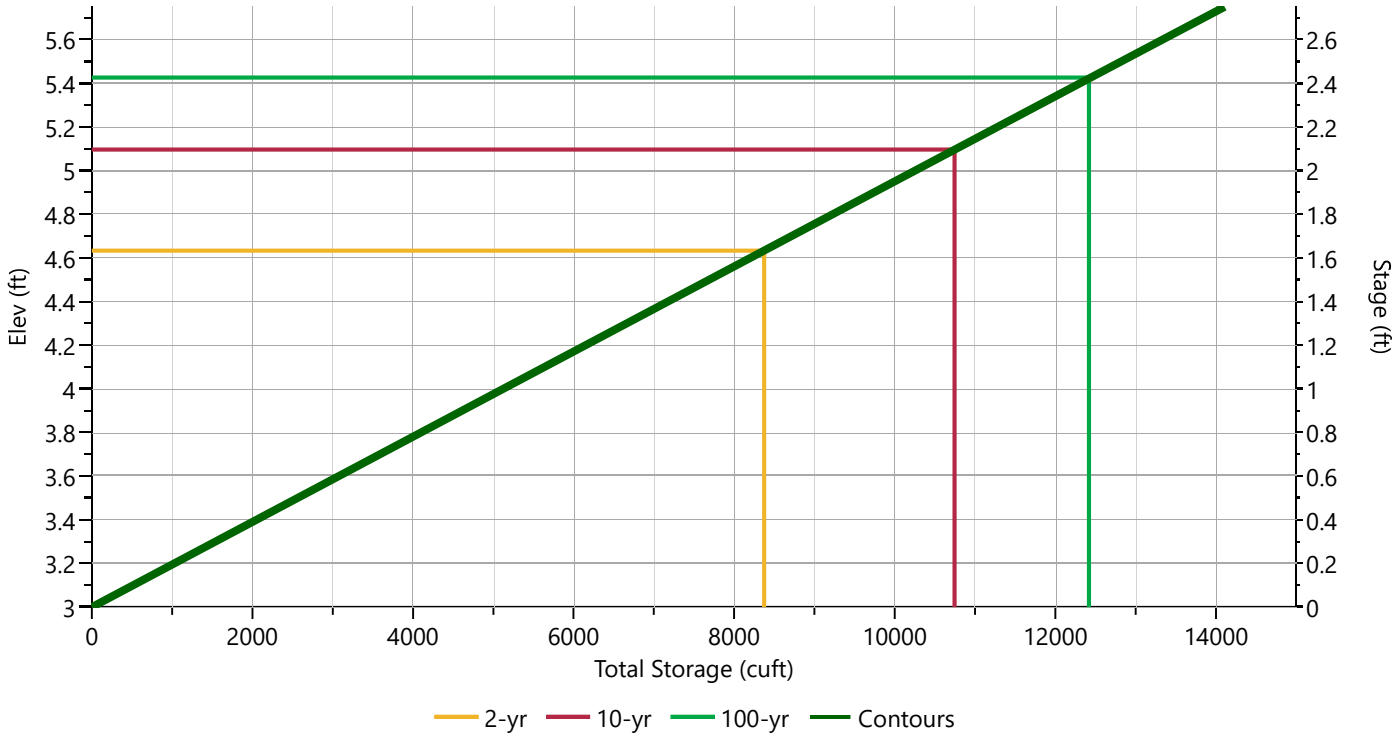
Stormtrap A7

Stage-Storage

User Defined Contours	
Description	**Input**
Bottom Elevation, ft	3.00
Voids (%)	100.00
Volume Calc	Conic

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)	Total Storage (cuft)
0.00	3.00	5,130	0.000	0.000
1.00	4.00	5,130	5,129	5,129
2.00	5.00	5,130	5,129	10,259
2.50	5.50	5,130	2,565	12,824
2.75	5.75	5,130	1,282	14,106



Pond Report

File: Proposed HydrologyCurrent Precipitation.hys

Hydrology Studio v 3.0.0.38

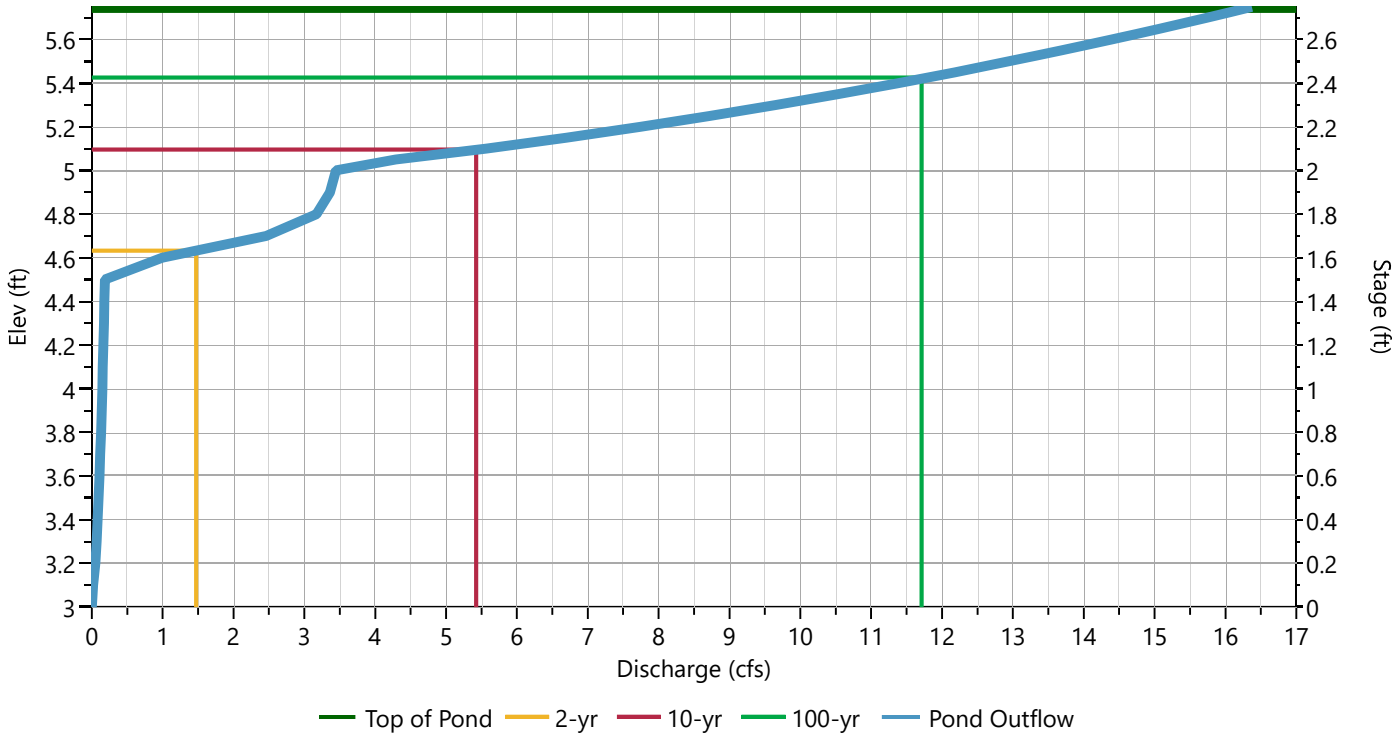
07-31-2025

Stormtrap A7

Stage-Discharge

Culvert / Orifices	Cir Culvert	Orifice			Perforated Riser	
		1 (m)	2	3		
Rise, in	24	2.5			Hole Diameter, in	
Span, in	24	2.5			No. holes	
No. Barrels	1	1			Invert Elevation, ft	
Invert Elevation, ft	3.00	3.00			Height, ft	
Orifice Coefficient, Co	0.60	0.60			Orifice Coefficient, Co	
Length, ft	10					
Barrel Slope, %	.3					
N-Value, n	0.012					
Weirs	**Riser**	**Weir**			**Ancillary**	
		1 (m)	2	3		
Shape / Type		Rectangular			Exfiltration, in/hr	
Crest Elevation, ft		4.5				
Crest Length, ft		8				
Angle, deg						
Weir Coefficient, Cw		3.3				

m = Flows through Culvert, i = Independent



Pond Report

File: Proposed HydrologyCurrent Precipitation.hys

Hydrology Studio v 3.0.0.38

07-31-2025

Stormtrap A7

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.00	3.00	0.000	0.000	0.000				0.000						0.000
1.00	4.00	5,129	0.148 oc	0.148				0.000						0.148
2.00	5.00	10,259	3.447 oc	0.026				3.421 s						3.447
2.50	5.50	12,824	12.95 oc	0.054				12.89 s						12.95
2.75	5.75	14,106	16.37 oc	0.052				16.32 s						16.37

Suffix key: ic = inlet control, oc = outlet control, s = submerged weir

Pond Report

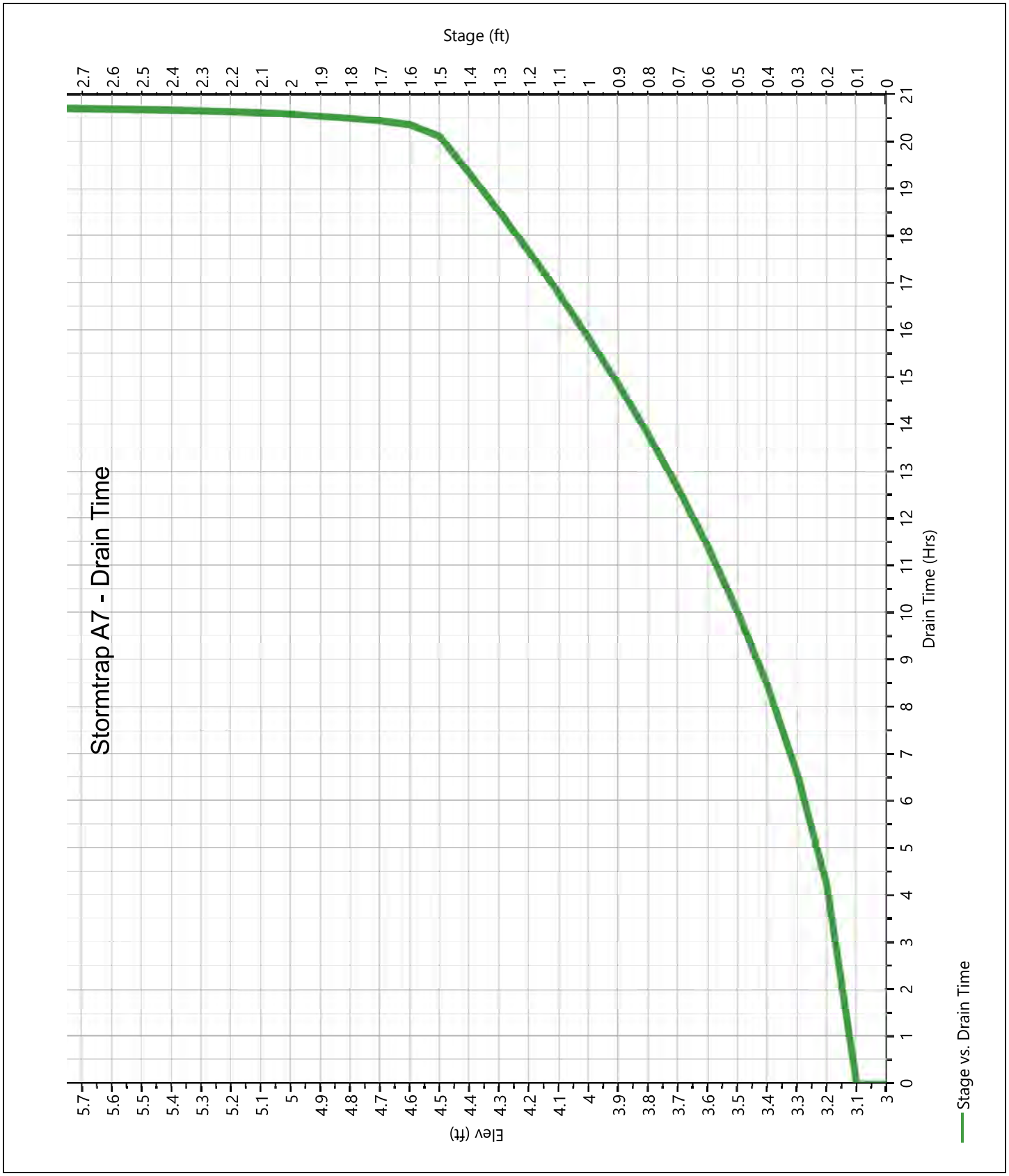
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Hydrology Studio v 3.0.0.38

07-31-2025

Stormtrap A7

Pond Drawdown



Hydrograph Report

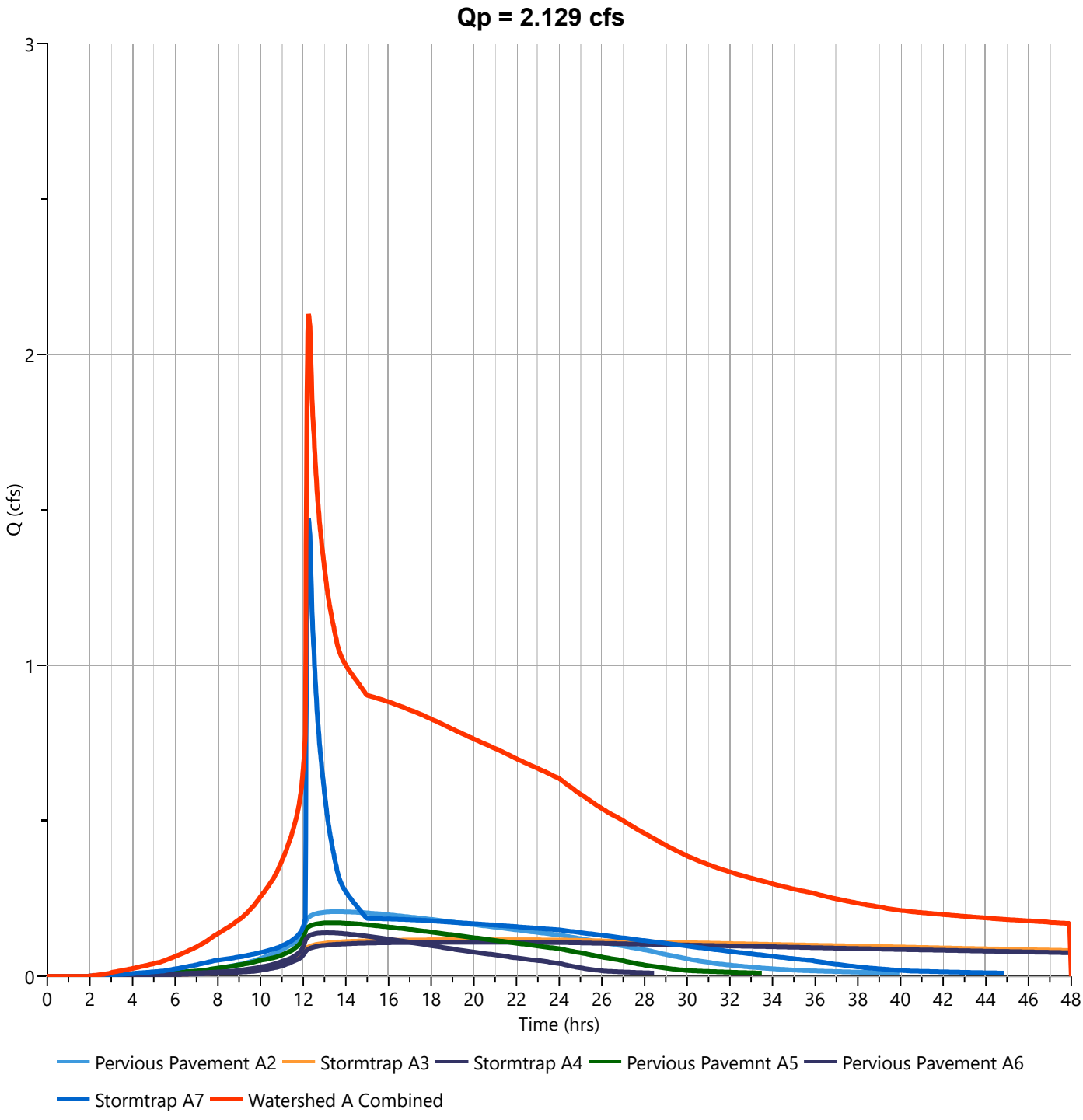
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

Watershed A Combined

Hyd. No. 23

Hydrograph Type	= Junction	Peak Flow	= 2.129 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.25 hrs
Time Interval	= 1 min	Hydrograph Volume	= 70,046 cuft
Inflow Hydrographs		Total Contrib. Area	= 0.0 ac



Hydrograph Report

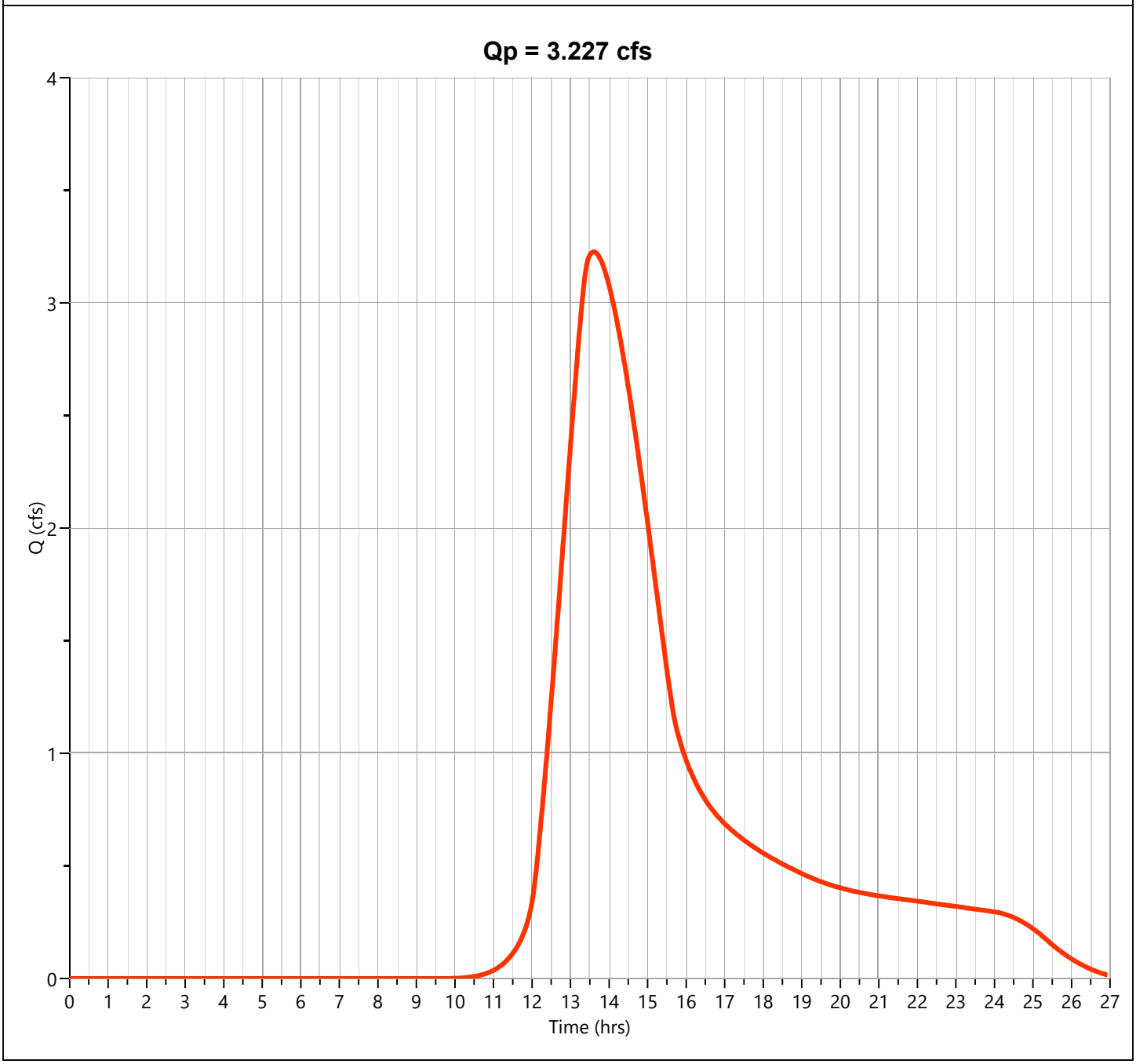
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD B Perv

Hyd. No. 25

Hydrograph Type	= NRCS Runoff	Peak Flow	= 3.227 cfs
Storm Frequency	= 2-yr	Time to Peak	= 13.60 hrs
Time Interval	= 1 min	Runoff Volume	= 45,593 cuft
Drainage Area	= 9.5 ac	Curve Number	= 77.00
Tc Method	= User	Time of Conc. (Tc)	= 138.0 min
Total Rainfall	= 3.35 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

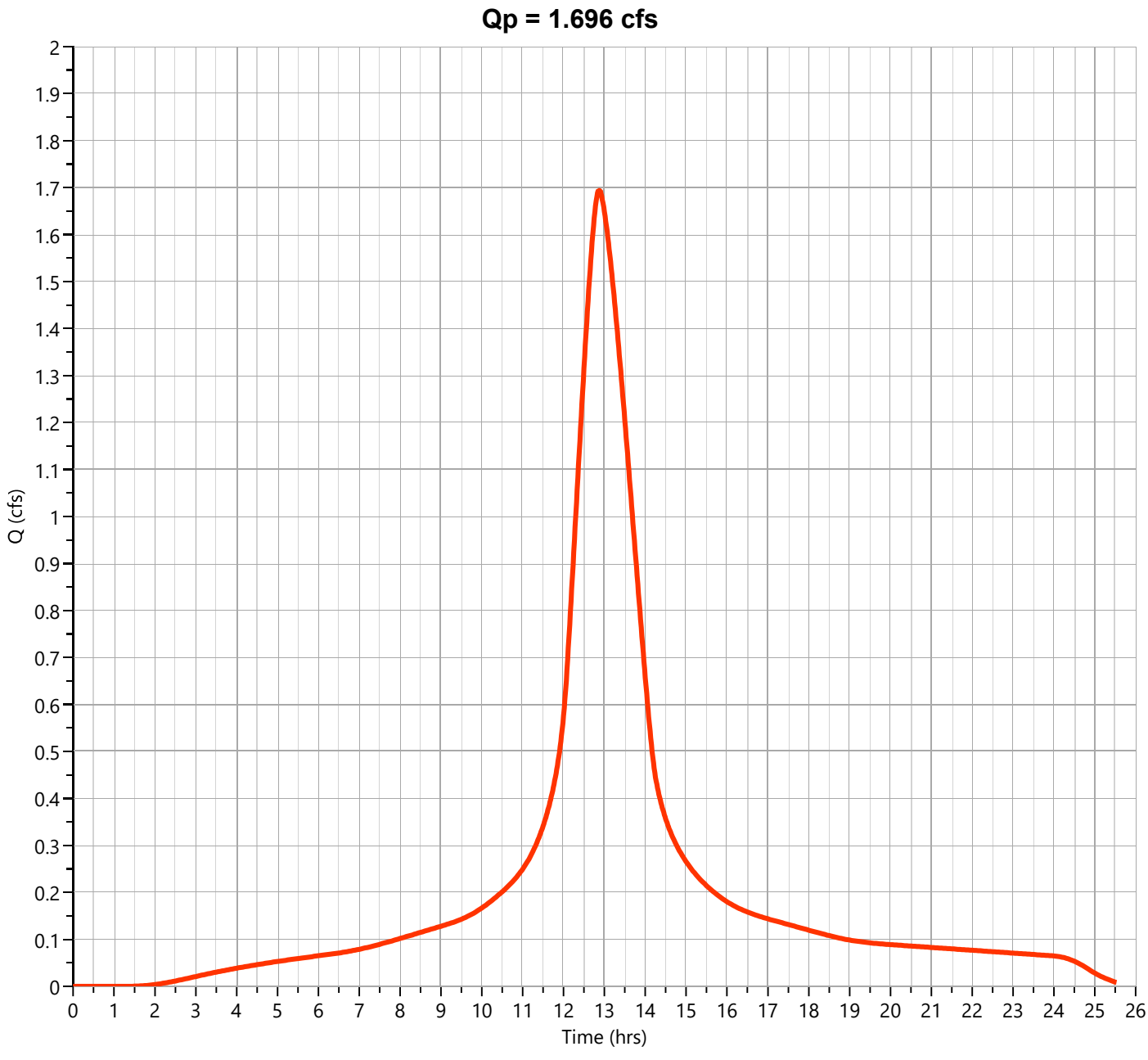
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File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD B Imperv

Hyd. No. 26

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.696 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.87 hrs
Time Interval	= 1 min	Runoff Volume	= 18,149 cuft
Drainage Area	= 1.6 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 82.0 min
Total Rainfall	= 3.35 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

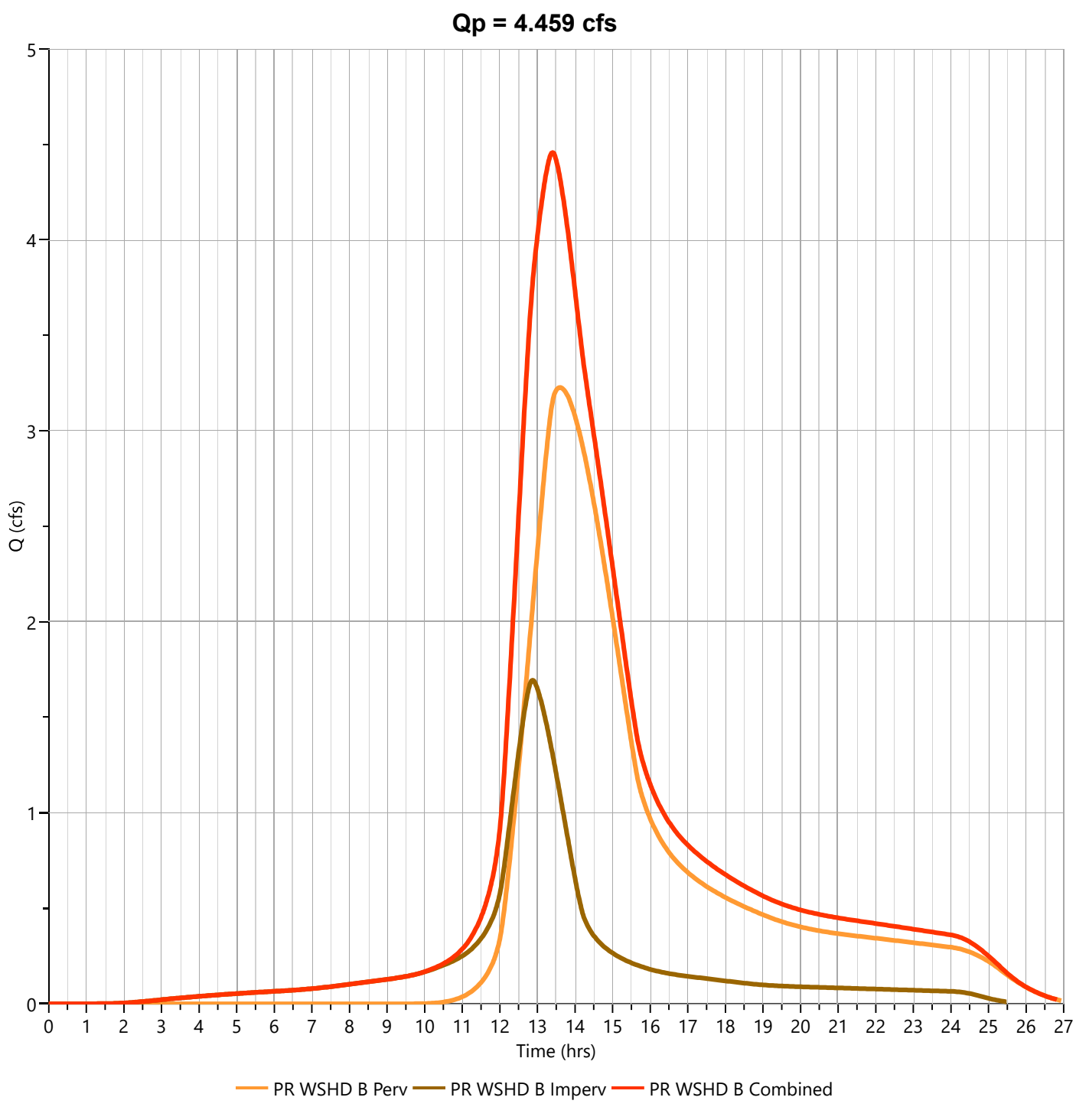
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD B Combined

Hyd. No. 27

Hydrograph Type	= Junction	Peak Flow	= 4.459 cfs
Storm Frequency	= 2-yr	Time to Peak	= 13.40 hrs
Time Interval	= 1 min	Hydrograph Volume	= 63,742 cuft
Inflow Hydrographs	= 25, 26	Total Contrib. Area	= 11.1 ac



Hydrograph Report

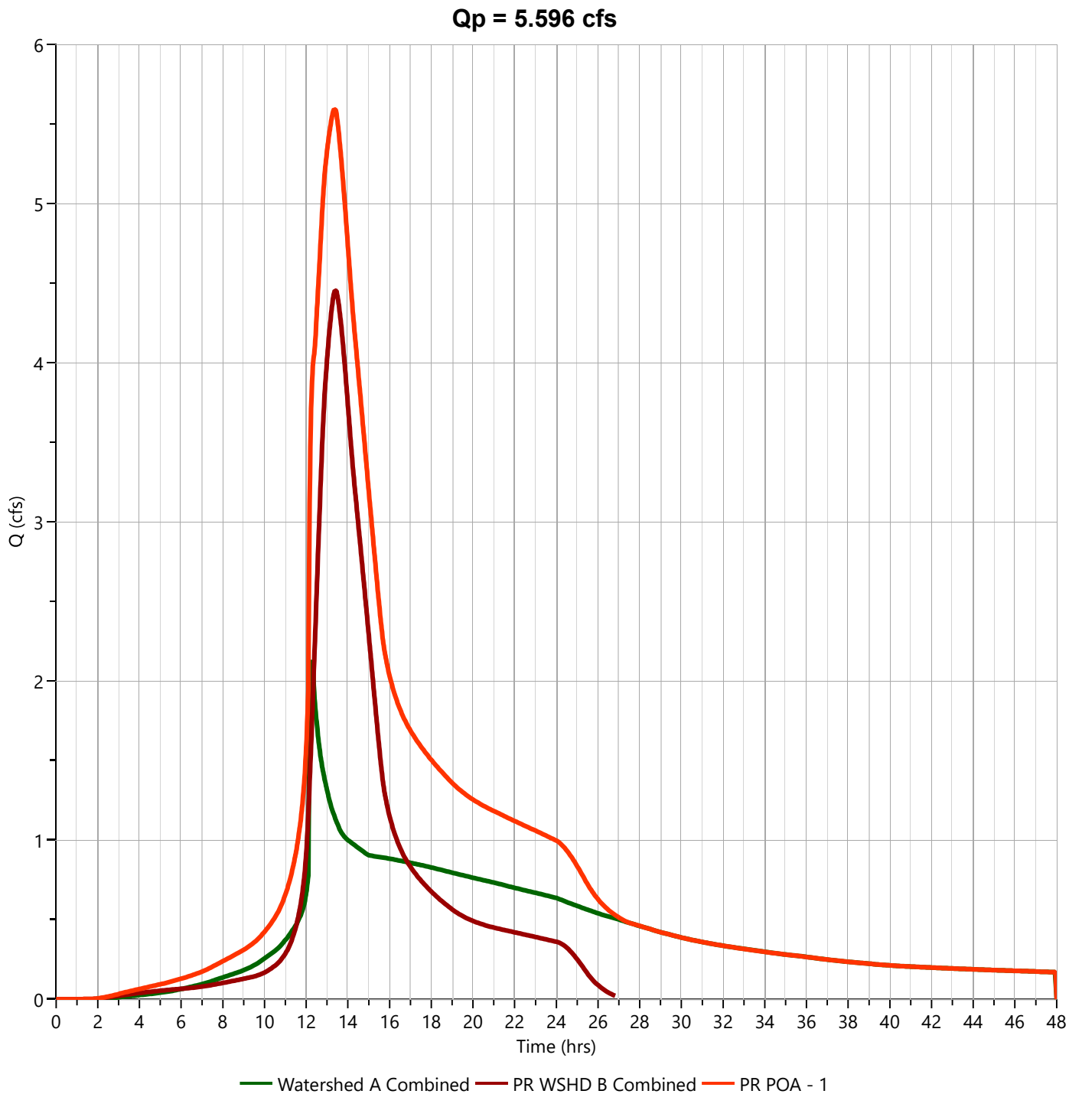
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR POA - 1

Hyd. No. 29

Hydrograph Type	= Junction	Peak Flow	= 5.596 cfs
Storm Frequency	= 2-yr	Time to Peak	= 13.37 hrs
Time Interval	= 1 min	Hydrograph Volume	= 133,788 cuft
Inflow Hydrographs		Total Contrib. Area	= 0.0 ac



Hydrograph Report

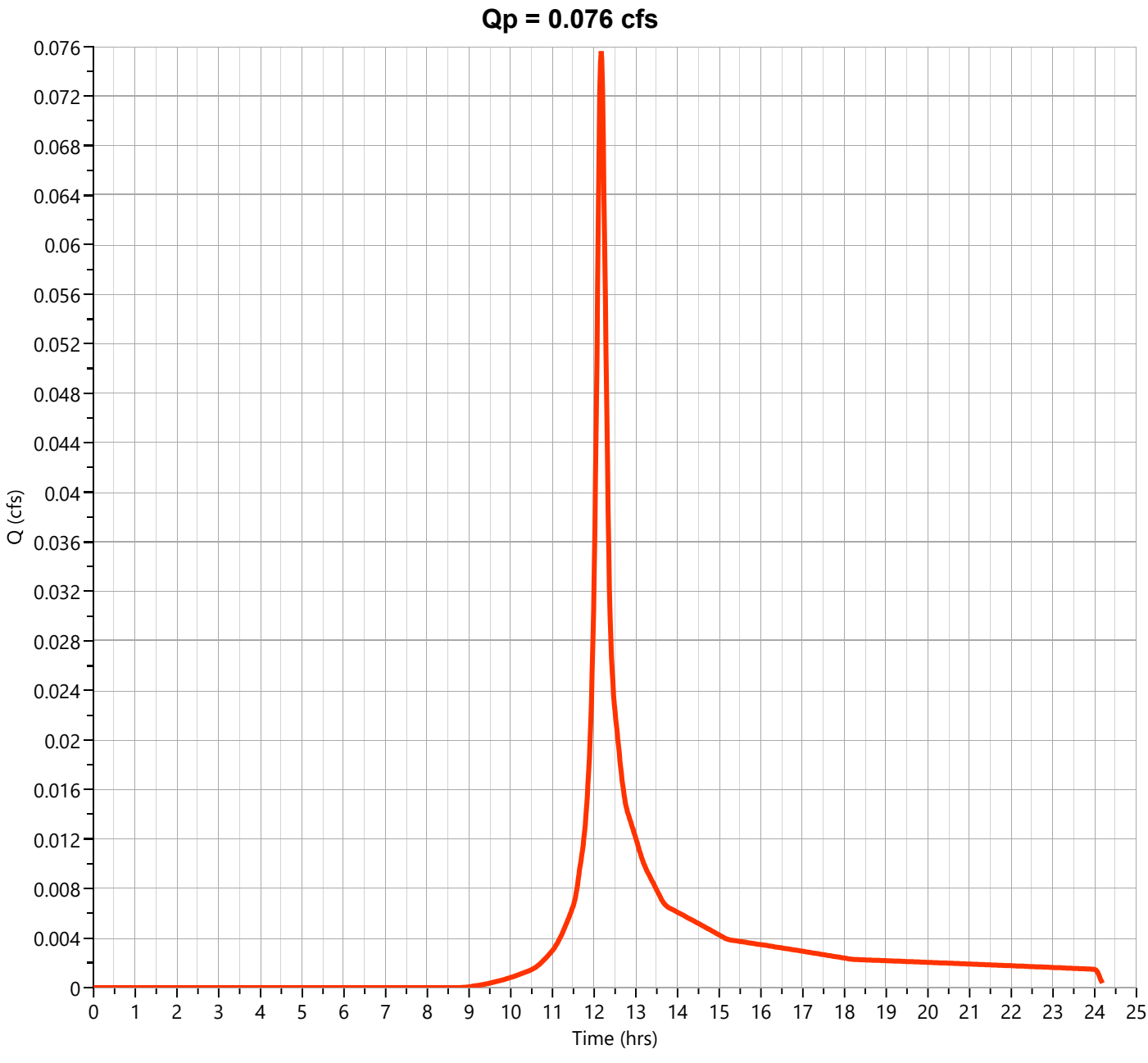
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD C PerPOA 2

Hyd. No. 32

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.076 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.17 hrs
Time Interval	= 1 min	Runoff Volume	= 280 cuft
Drainage Area	= 0.05 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 11.0 min
Total Rainfall	= 3.35 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph 10-yr Summary

Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys

07-31-2025

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	NRCS Runoff	PR WSDH A2 Imperv	5.839	12.10	19,305	----		
2	Pond Route	Pervious Pavement A2	0.361	13.42	18,888	1	2.04	11,284
4	NRCS Runoff	PR WSHD A3 Imperv	12.41	12.10	41,901	----		
5	Pond Route	Stormtrap A3	0.156	22.30	19,400	4	2.72	34,475
7	NRCS Runoff	PR WSHD A4 Imperv	11.19	12.10	37,783	----		
8	Pond Route	Stormtrap A4	0.146	21.92	18,079	7	2.60	30,889
10	NRCS Runoff	PR WSHD A5 Imperv	3.910	12.10	12,926	----		
11	NRCS Runoff	PR WSHD A5 Perv	0.187	12.10	518	----		
12	Junction	PR WSHD A5 Combined	4.097	12.10	13,444	10, 11		
13	Pond Route	Pervious Pavemnt A5	0.596	12.53	13,413	12	5.16	6,536
15	NRCS Runoff	PR WSHD A6 Imperv	2.082	12.10	6,883	----		
16	NRCS Runoff	PR WSHD A6 Perv	0.537	12.15	1,877	----		
17	Junction	PR WSHD A6 Combined	2.566	12.10	8,760	15, 16		
18	Pond Route	Pervious Pavement A6	0.192	13.33	8,743	17	4.12	4,404
20	NRCS Runoff	PR WSHD A7 Imperv	8.479	12.10	28,035	----		
21	Pond Route	Stormtrap A7	5.423	12.13	27,770	20	5.10	10,743
23	Junction	Watershed A Combined	6.247	12.13	106,293	2, 5, 8, 13, 18, 21		
25	NRCS Runoff	PR WSHD B Perv	7.003	13.52	95,525	----		
26	NRCS Runoff	PR WSHD B Imperv	2.639	12.87	28,723	----		
27	Junction	PR WSHD B Combined	8.963	13.40	124,249	25, 26		
29	Junction	PR POA - 1	10.78	13.37	230,542	23, 27		
32	NRCS Runoff	PR WSHD C PerPOA 2	0.151	12.17	562	----		

Hydrograph Report

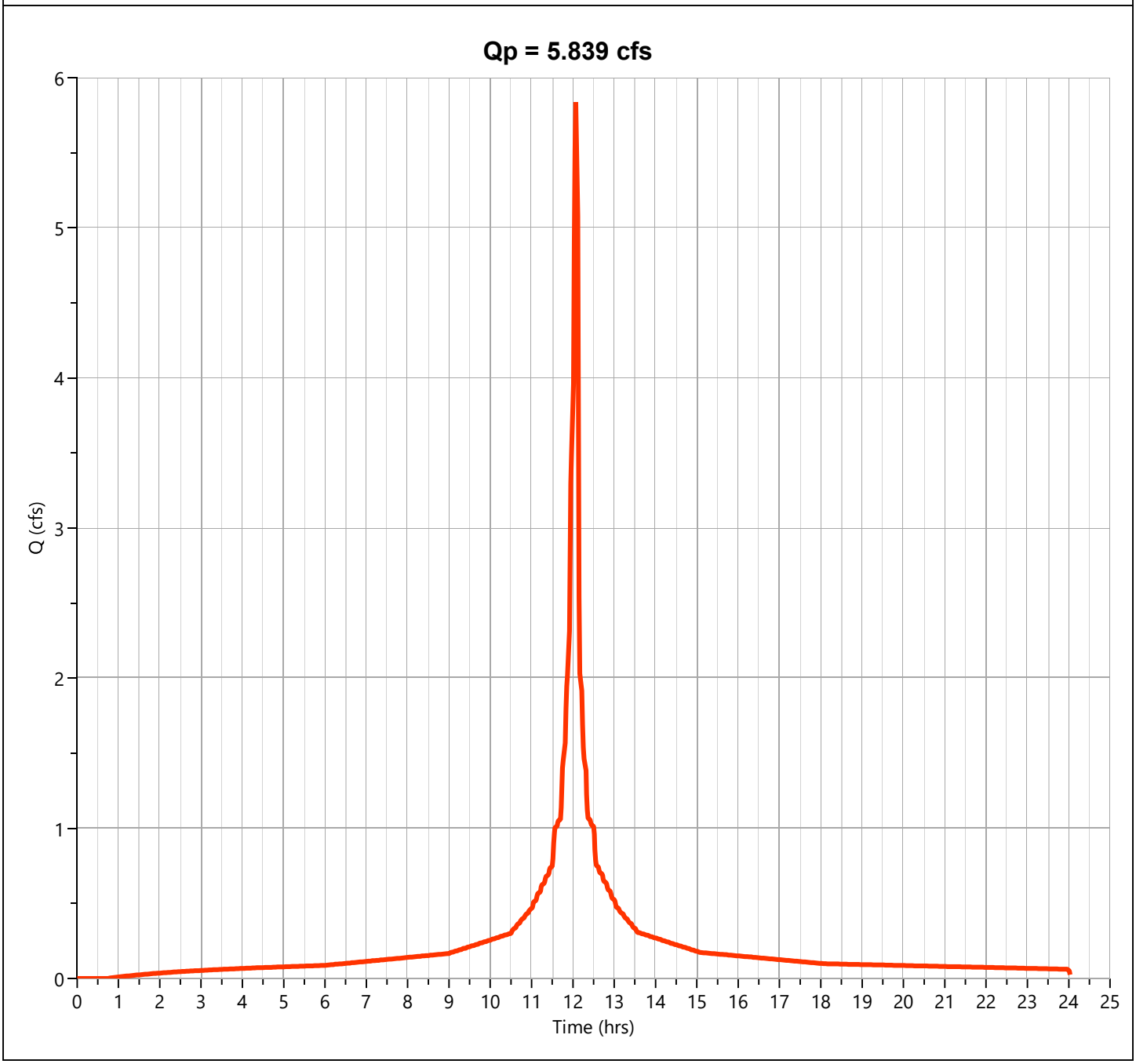
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSDH A2 Imperv

Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 5.839 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 19,305 cuft
Drainage Area	= 1.15 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 3.0 min
Total Rainfall	= 5.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.38

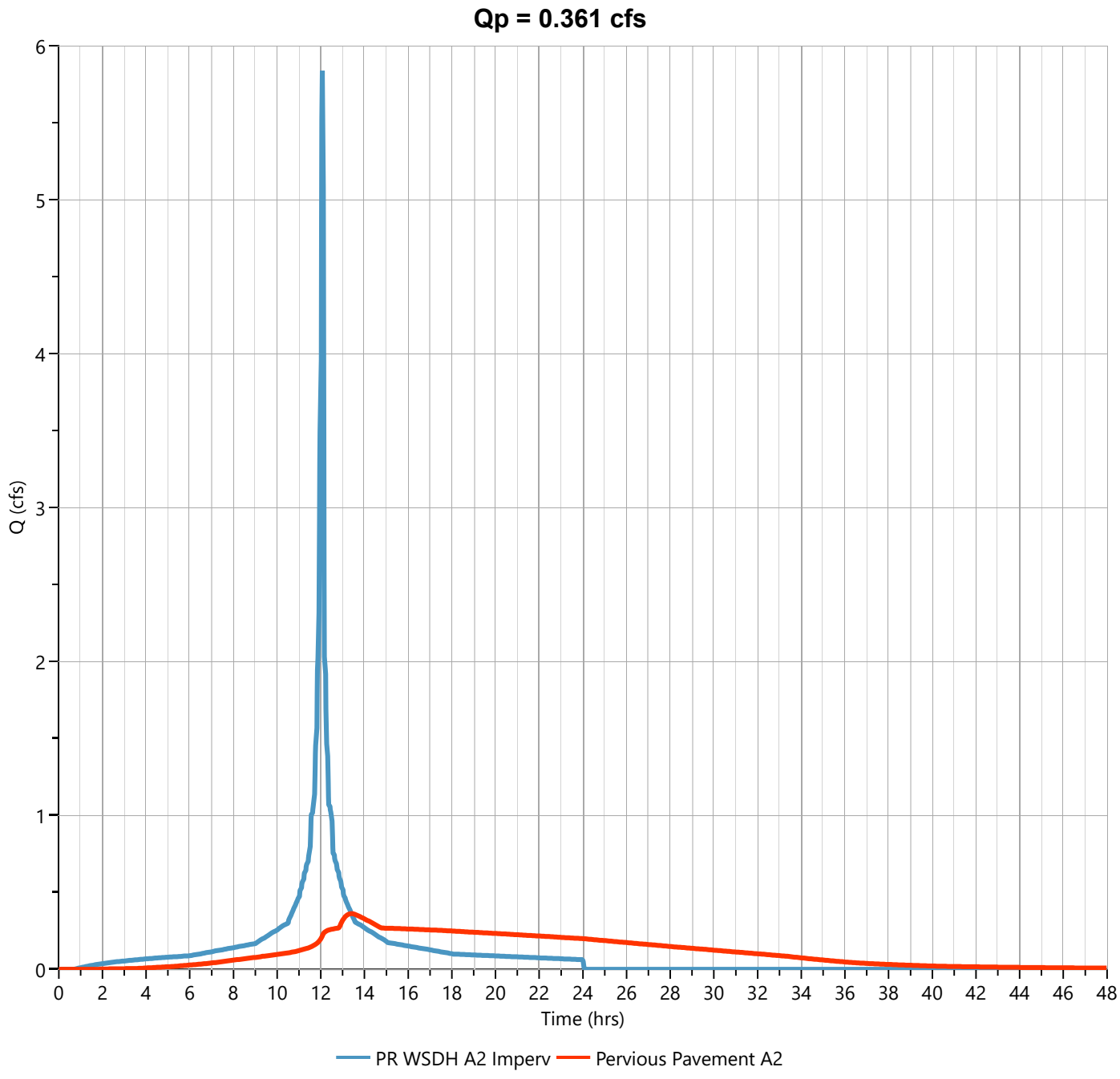
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07-31-2025

Pervious Pavement A2 — Hyd. No. 2

Hydrograph Type	= Pond Route	Peak Flow	= 0.361 cfs
Storm Frequency	= 10-yr	Time to Peak	= 13.42 hrs
Time Interval	= 1 min	Hydrograph Volume	= 18,888 cuft
Inflow Hydrograph	= 1 - PR WSDH A2 Imperv	Max. Elevation	= 2.04 ft
Pond Name	= Pervious Pavement A2	Max. Storage	= 11,284 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 8.47 hrs



Hydrograph Report

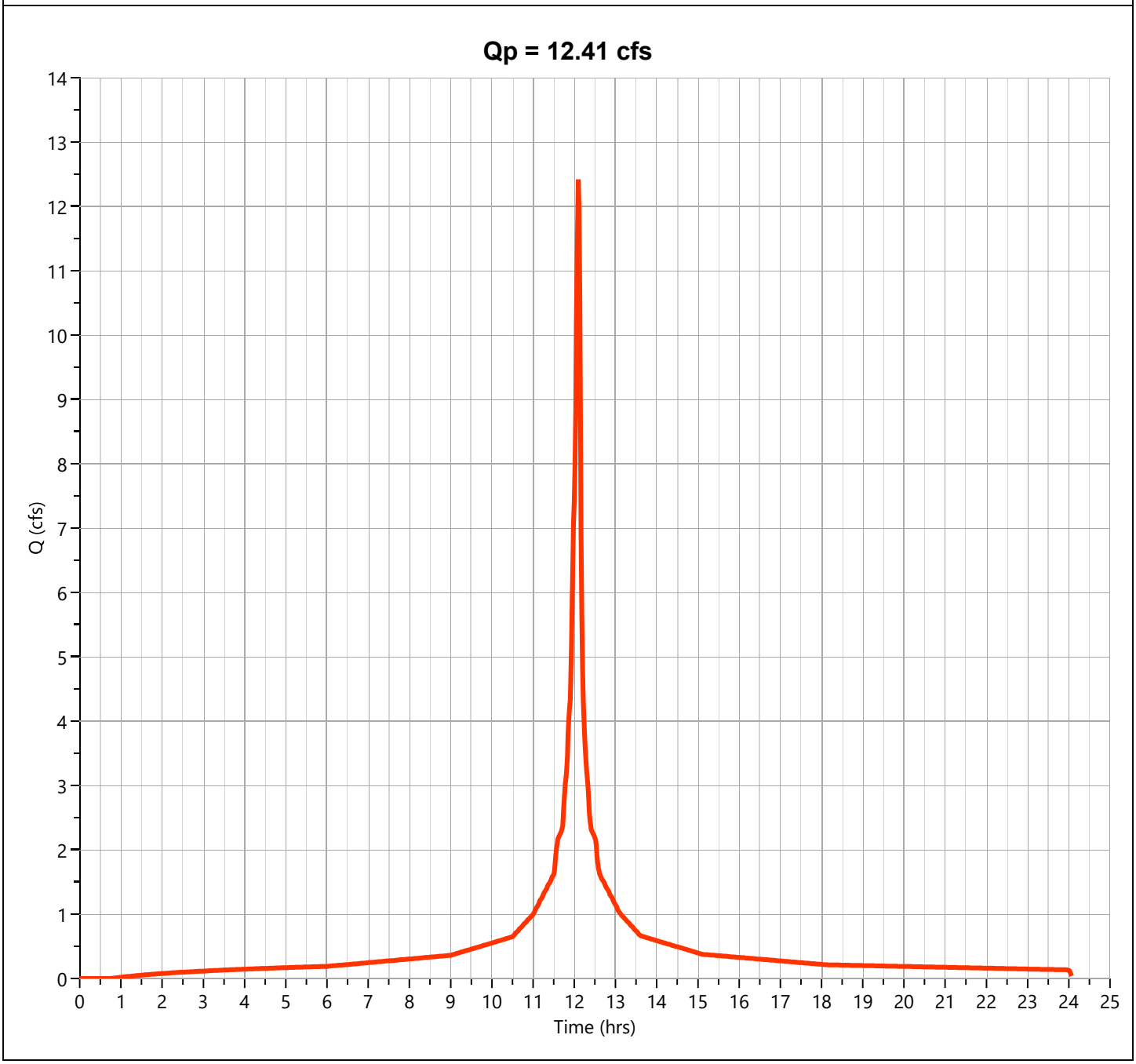
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A3 Imperv

Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 12.41 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 41,901 cuft
Drainage Area	= 2.34 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 4.0 min
Total Rainfall	= 5.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

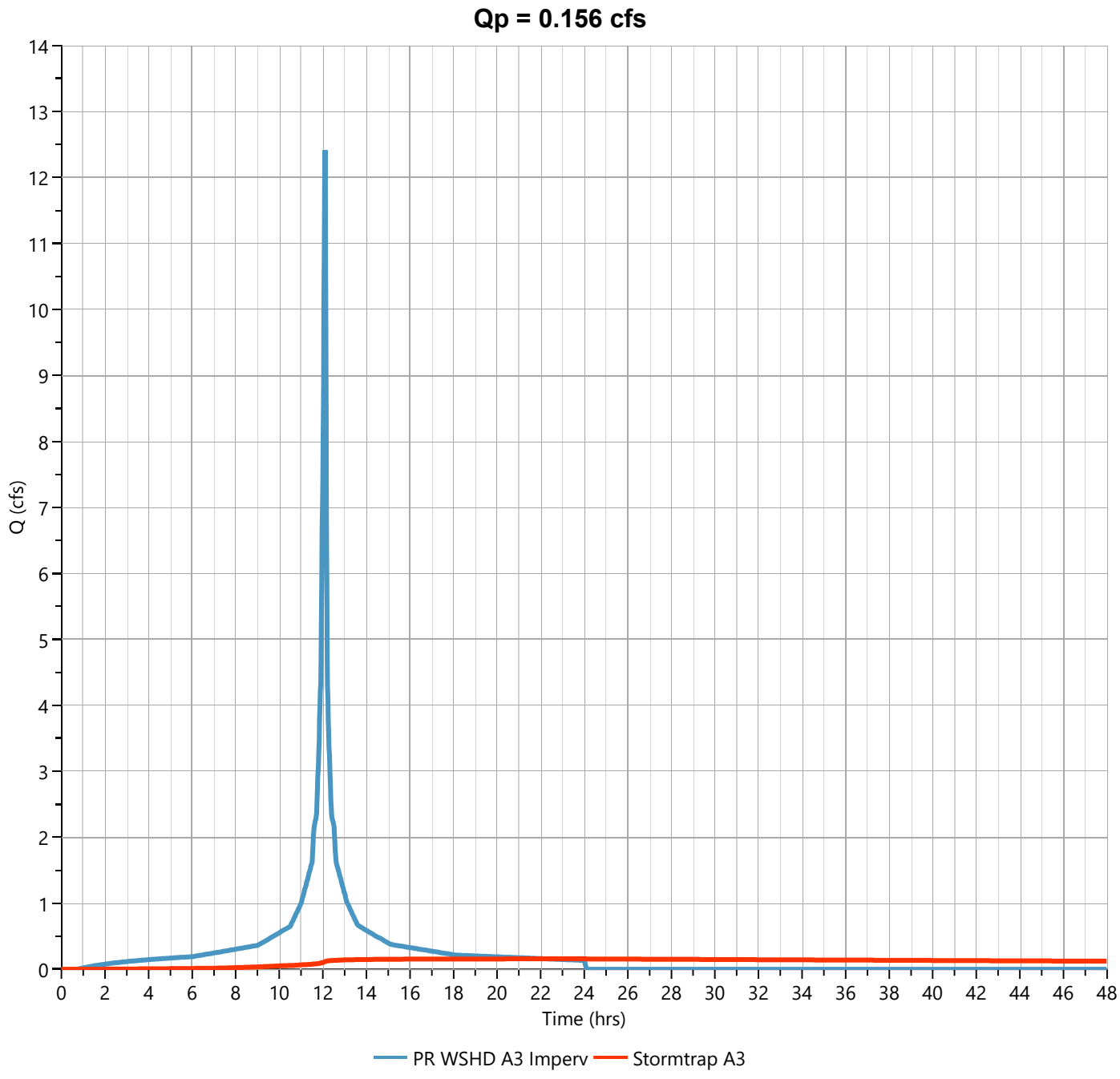
Stormtrap A3

Hyd. No. 5

Hydrograph Type	= Pond Route	Peak Flow	= 0.156 cfs
Storm Frequency	= 10-yr	Time to Peak	= 22.30 hrs
Time Interval	= 1 min	Hydrograph Volume	= 19,400 cuft
Inflow Hydrograph	= 4 - PR WSHD A3 Imperv	Max. Elevation	= 2.72 ft
Pond Name	= Stormtrap A3	Max. Storage	= 34,475 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 15.97 hrs



Hydrograph Report

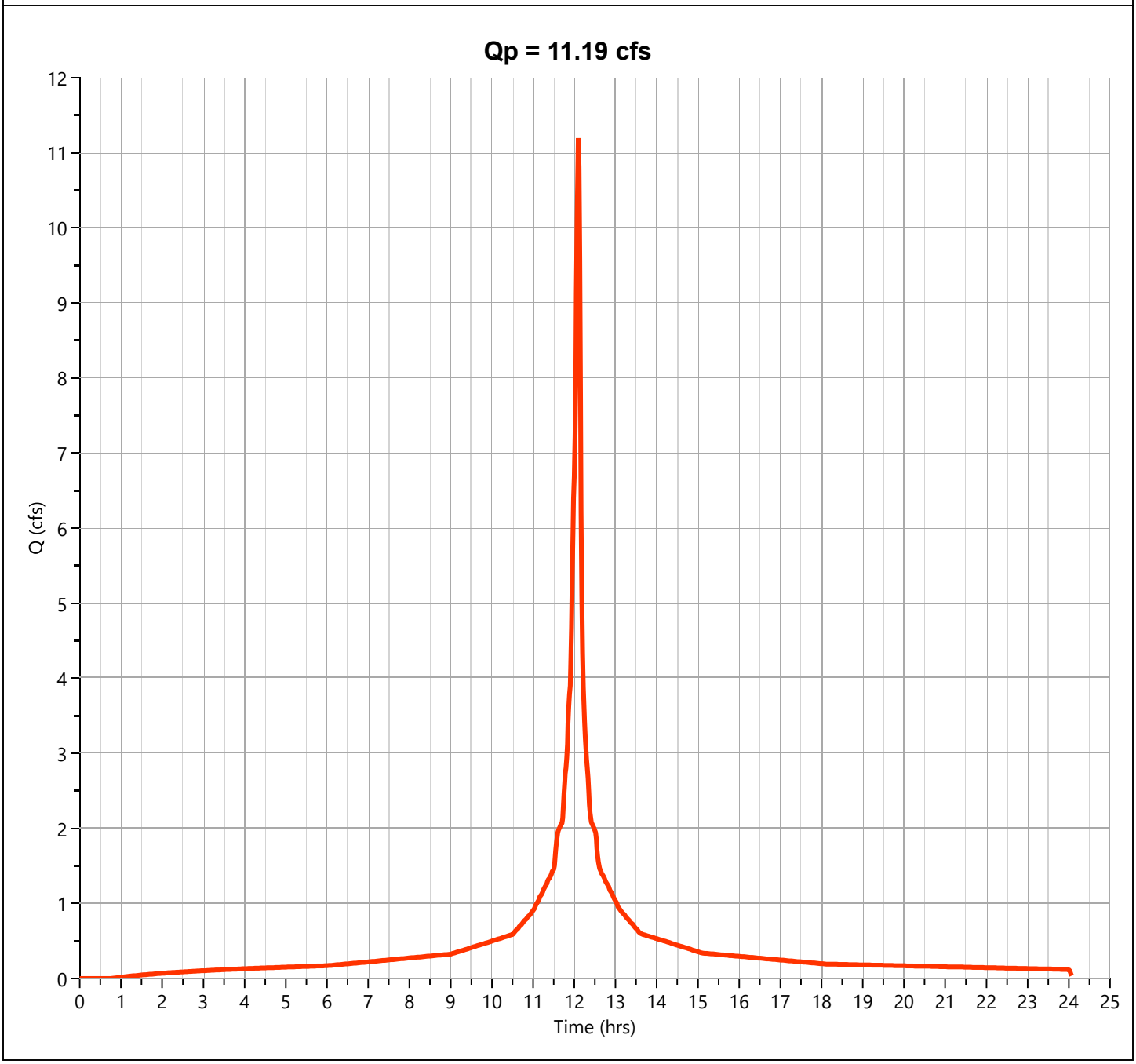
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File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A4 Imperv

Hyd. No. 7

Hydrograph Type	= NRCS Runoff	Peak Flow	= 11.19 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 37,783 cuft
Drainage Area	= 2.11 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 4.0 min
Total Rainfall	= 5.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.38

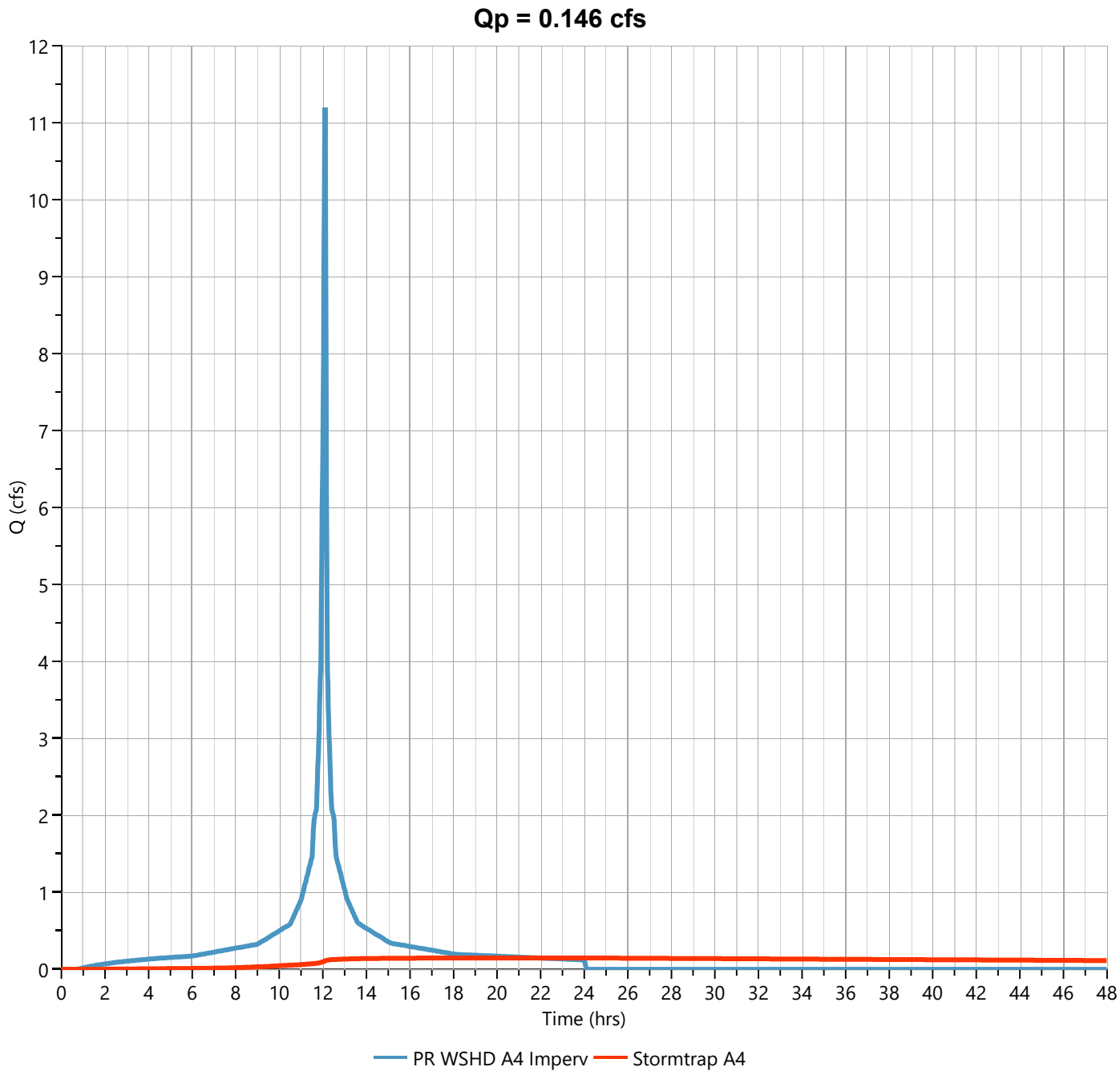
File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

Stormtrap A4 — Hyd. No. 8

Hydrograph Type	= Pond Route	Peak Flow	= 0.146 cfs
Storm Frequency	= 10-yr	Time to Peak	= 21.92 hrs
Time Interval	= 1 min	Hydrograph Volume	= 18,079 cuft
Inflow Hydrograph	= 7 - PR WSHD A4 Imperv	Max. Elevation	= 2.60 ft
Pond Name	= Stormtrap A4	Max. Storage	= 30,889 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 15.96 hrs



Hydrograph Report

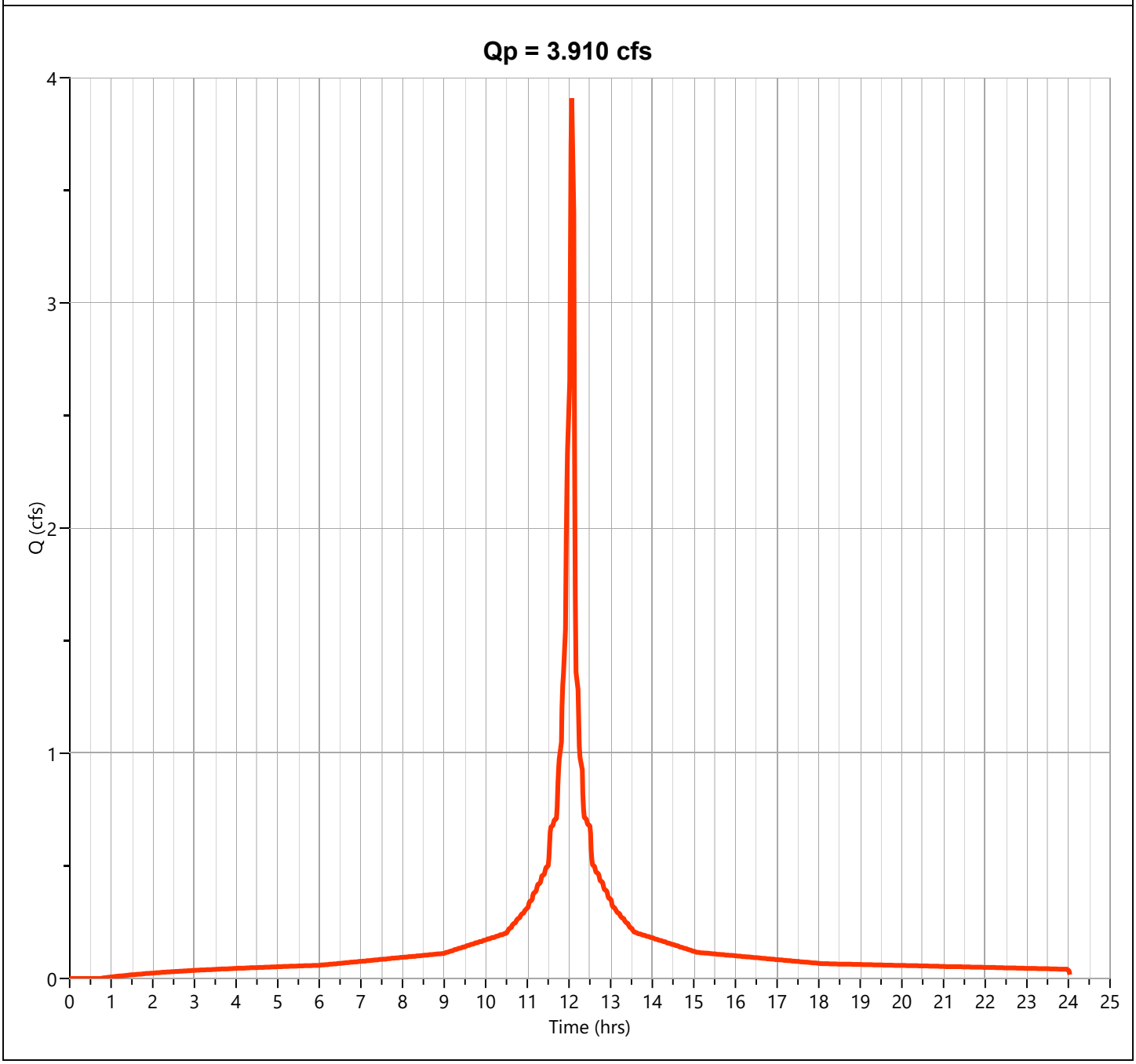
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A5 Imperv

Hyd. No. 10

Hydrograph Type	= NRCS Runoff	Peak Flow	= 3.910 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 12,926 cuft
Drainage Area	= 0.77 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 5.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

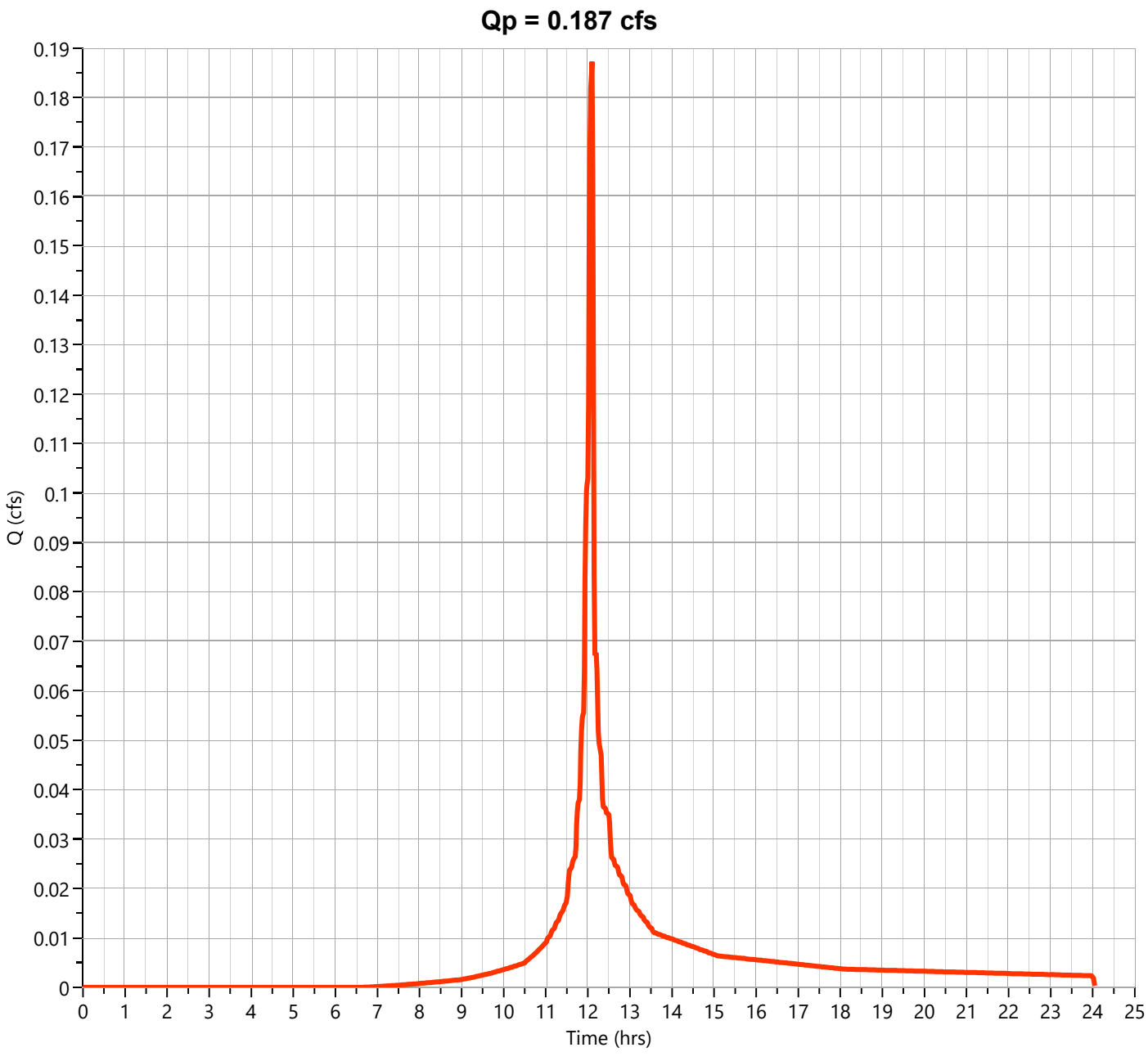
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A5 Perv

Hyd. No. 11

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.187 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 518 cuft
Drainage Area	= 0.05 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 5.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

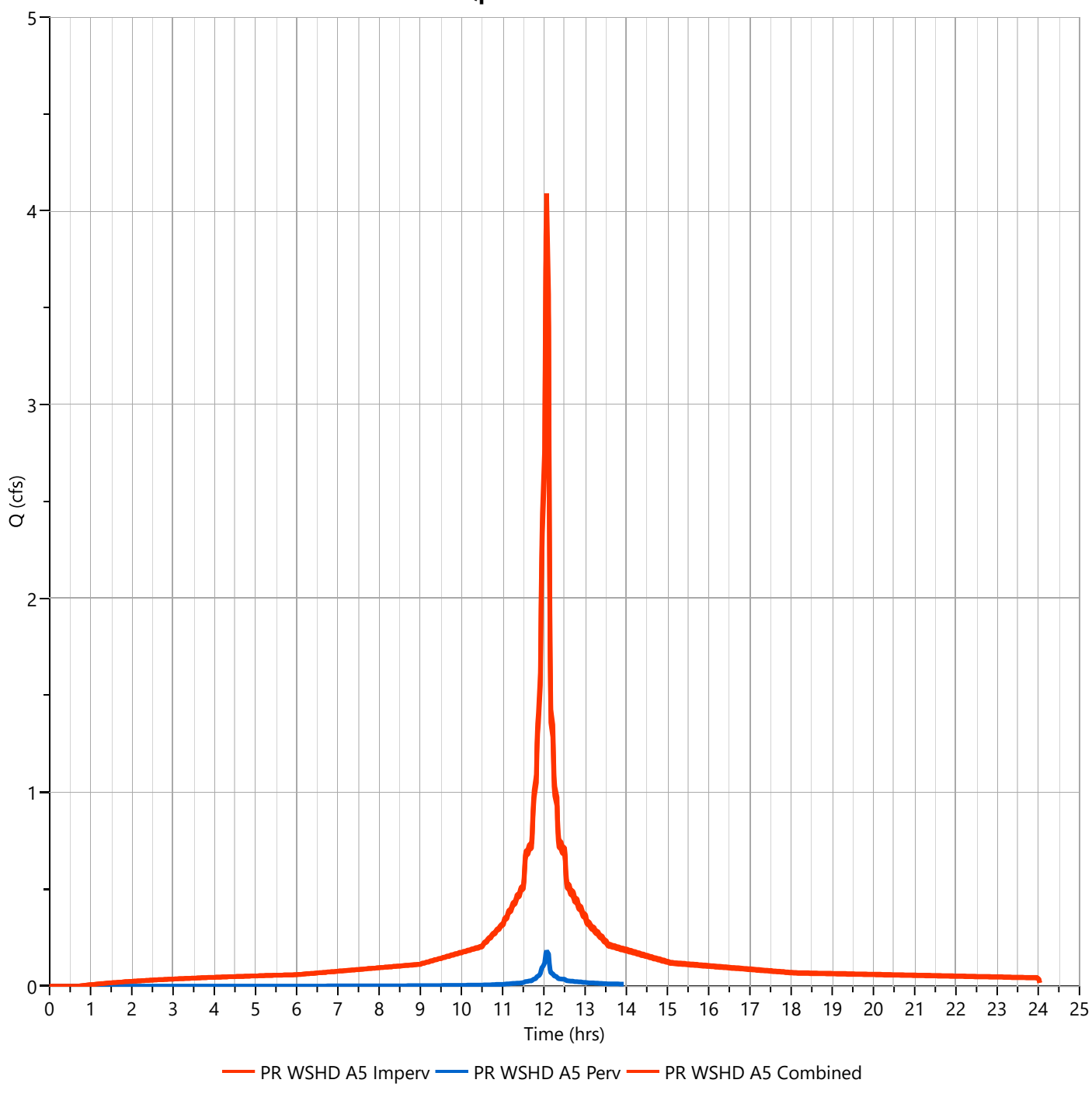
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A5 Combined

Hyd. No. 12

Hydrograph Type	= Junction	Peak Flow	= 4.097 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Hydrograph Volume	= 13,444 cuft
Inflow Hydrographs	= 10, 11	Total Contrib. Area	= 0.82 ac



Hydrograph Report

Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

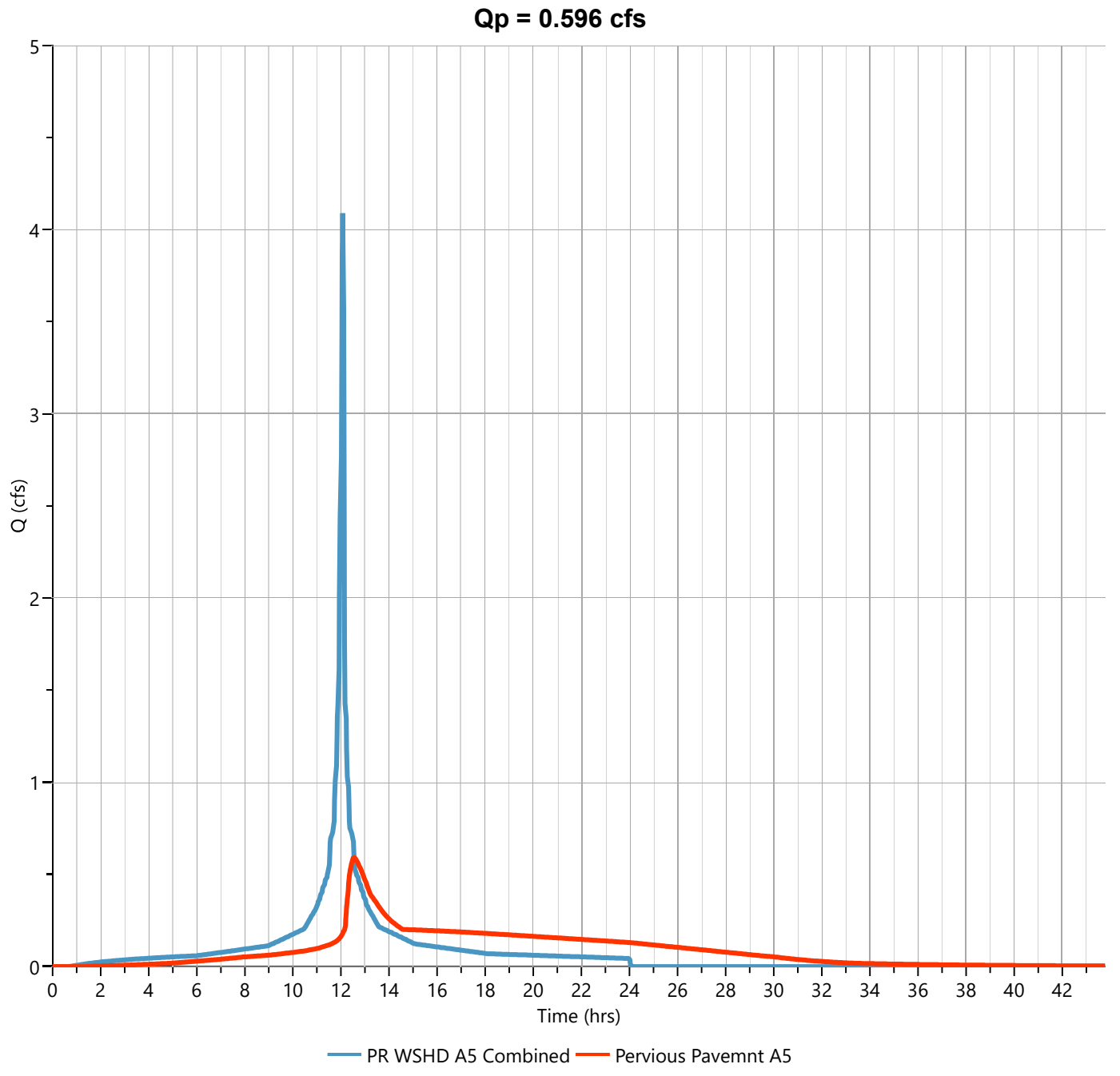
Pervious Pavemnt A5

Hyd. No. 13

Hydrograph Type	= Pond Route	Peak Flow	= 0.596 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.53 hrs
Time Interval	= 1 min	Hydrograph Volume	= 13,413 cuft
Inflow Hydrograph	= 12 - PR WSHD A5 Combined	Max. Elevation	= 5.16 ft
Pond Name	= Pervious Pavement A5	Max. Storage	= 6,536 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 5.59 hrs



Hydrograph Report

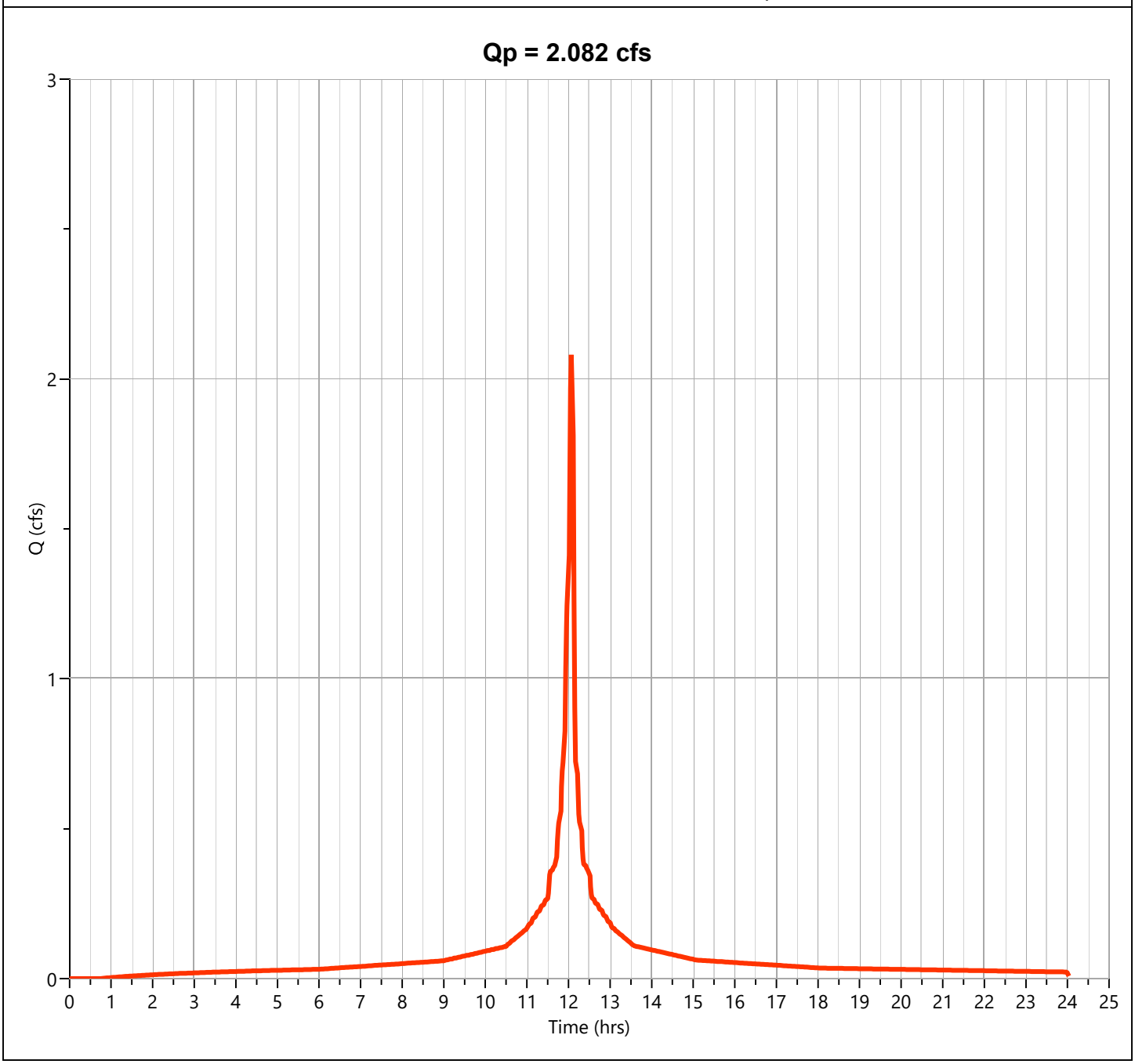
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A6 Imperv

Hyd. No. 15

Hydrograph Type	= NRCS Runoff	Peak Flow	= 2.082 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 6,883 cuft
Drainage Area	= 0.41 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 5.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

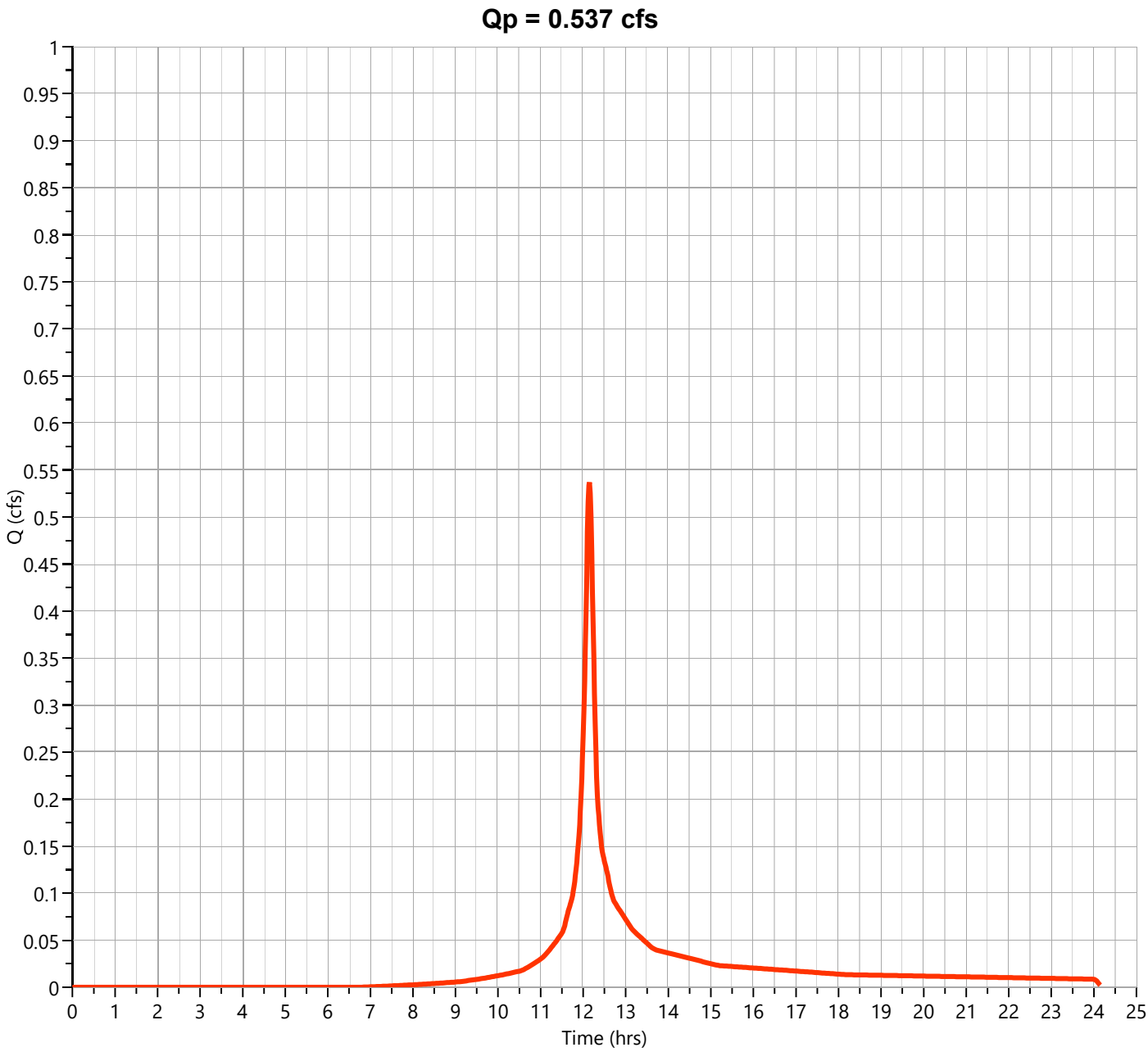
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A6 Perv

Hyd. No. 16

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.537 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.15 hrs
Time Interval	= 1 min	Runoff Volume	= 1,877 cuft
Drainage Area	= 0.17 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 9.0 min
Total Rainfall	= 5.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

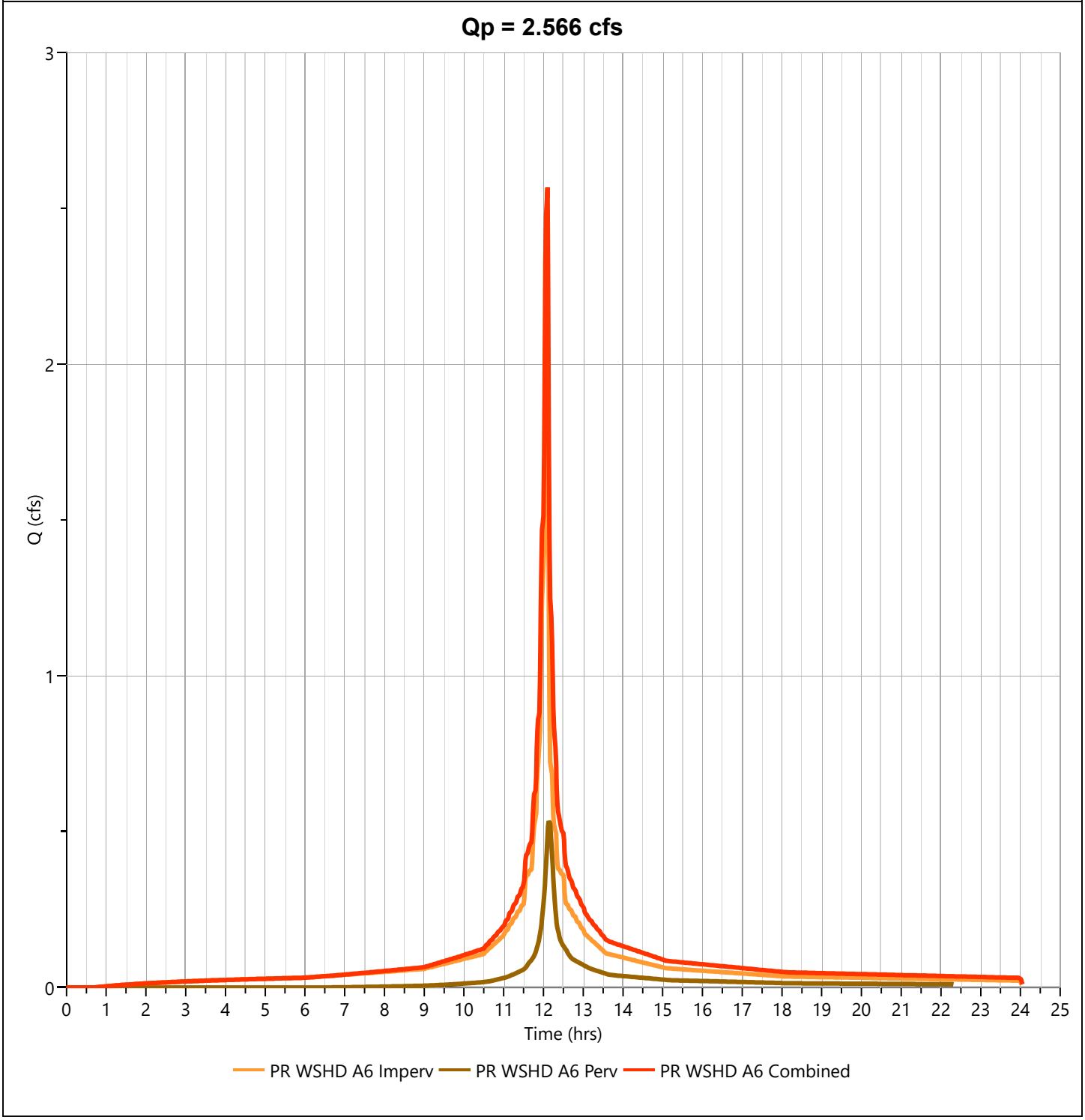
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A6 Combined

Hyd. No. 17

Hydrograph Type	= Junction	Peak Flow	= 2.566 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Hydrograph Volume	= 8,760 cuft
Inflow Hydrographs	= 15, 16	Total Contrib. Area	= 0.58 ac



Hydrograph Report

Hydrology Studio v 3.0.0.38

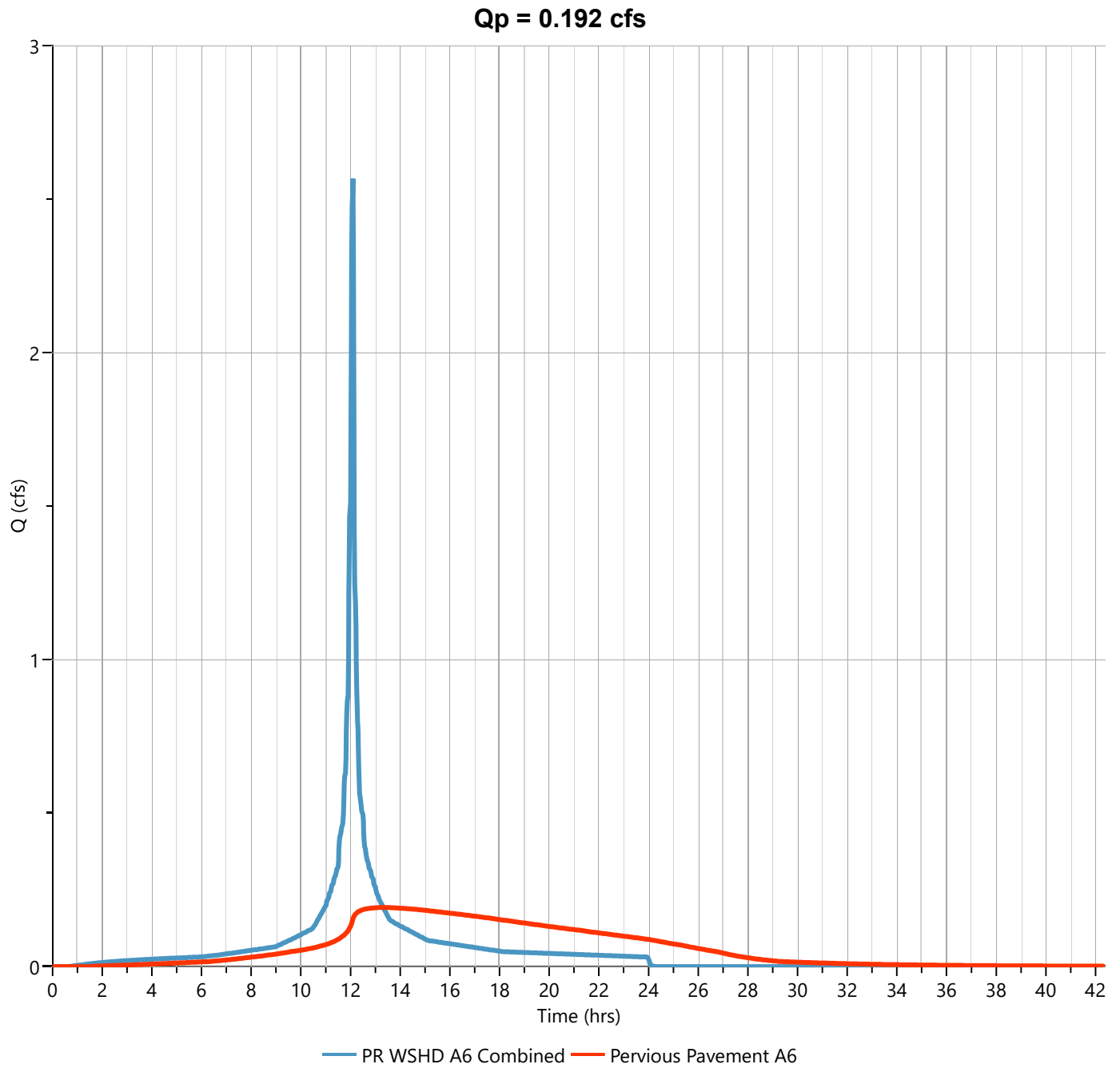
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07-31-2025

Pervious Pavement A6 — Hyd. No. 18

Hydrograph Type	= Pond Route	Peak Flow	= 0.192 cfs
Storm Frequency	= 10-yr	Time to Peak	= 13.33 hrs
Time Interval	= 1 min	Hydrograph Volume	= 8,743 cuft
Inflow Hydrograph	= 17 - PR WSHD A6 Combined	Max. Elevation	= 4.12 ft
Pond Name	= Pervious Pavement A6	Max. Storage	= 4,404 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 4.90 hrs



Hydrograph Report

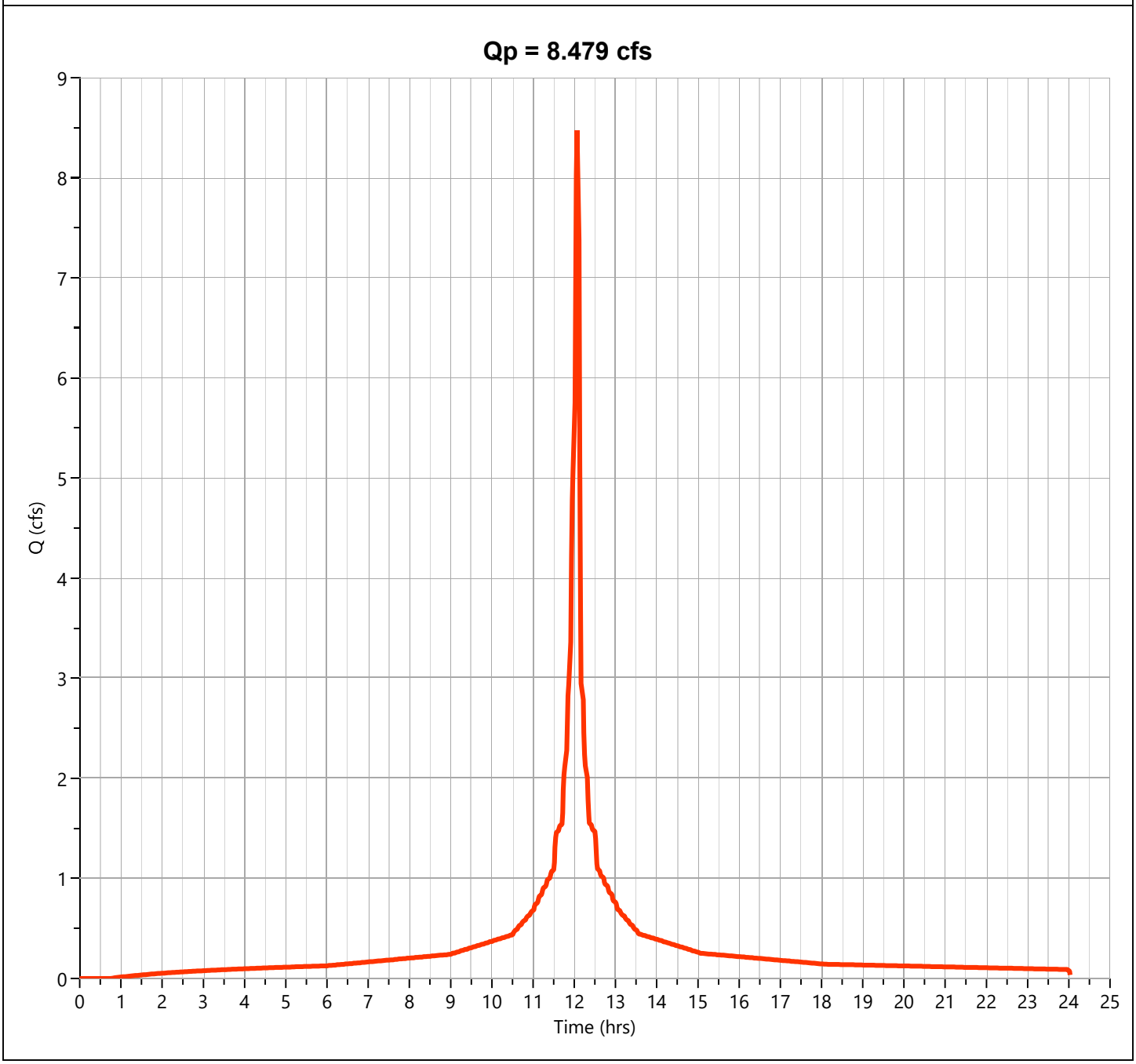
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A7 Imperv

Hyd. No. 20

Hydrograph Type	= NRCS Runoff	Peak Flow	= 8.479 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 28,035 cuft
Drainage Area	= 1.67 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 5.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

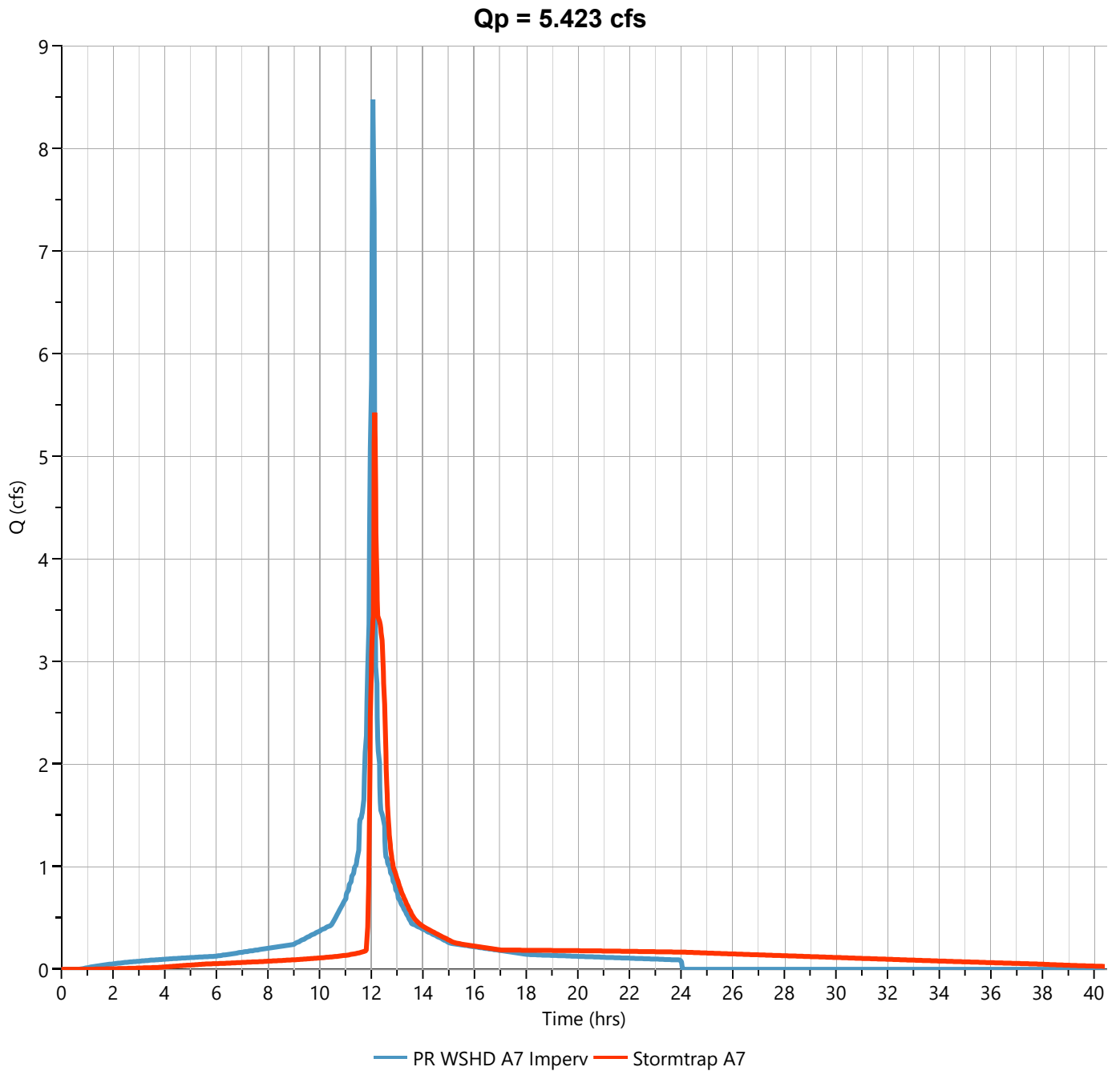
Stormtrap A7

Hyd. No. 21

Hydrograph Type	= Pond Route	Peak Flow	= 5.423 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.13 hrs
Time Interval	= 1 min	Hydrograph Volume	= 27,770 cuft
Inflow Hydrograph	= 20 - PR WSHD A7 Imperv	Max. Elevation	= 5.10 ft
Pond Name	= Stormtrap A7	Max. Storage	= 10,743 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 4.66 hrs



Hydrograph Report

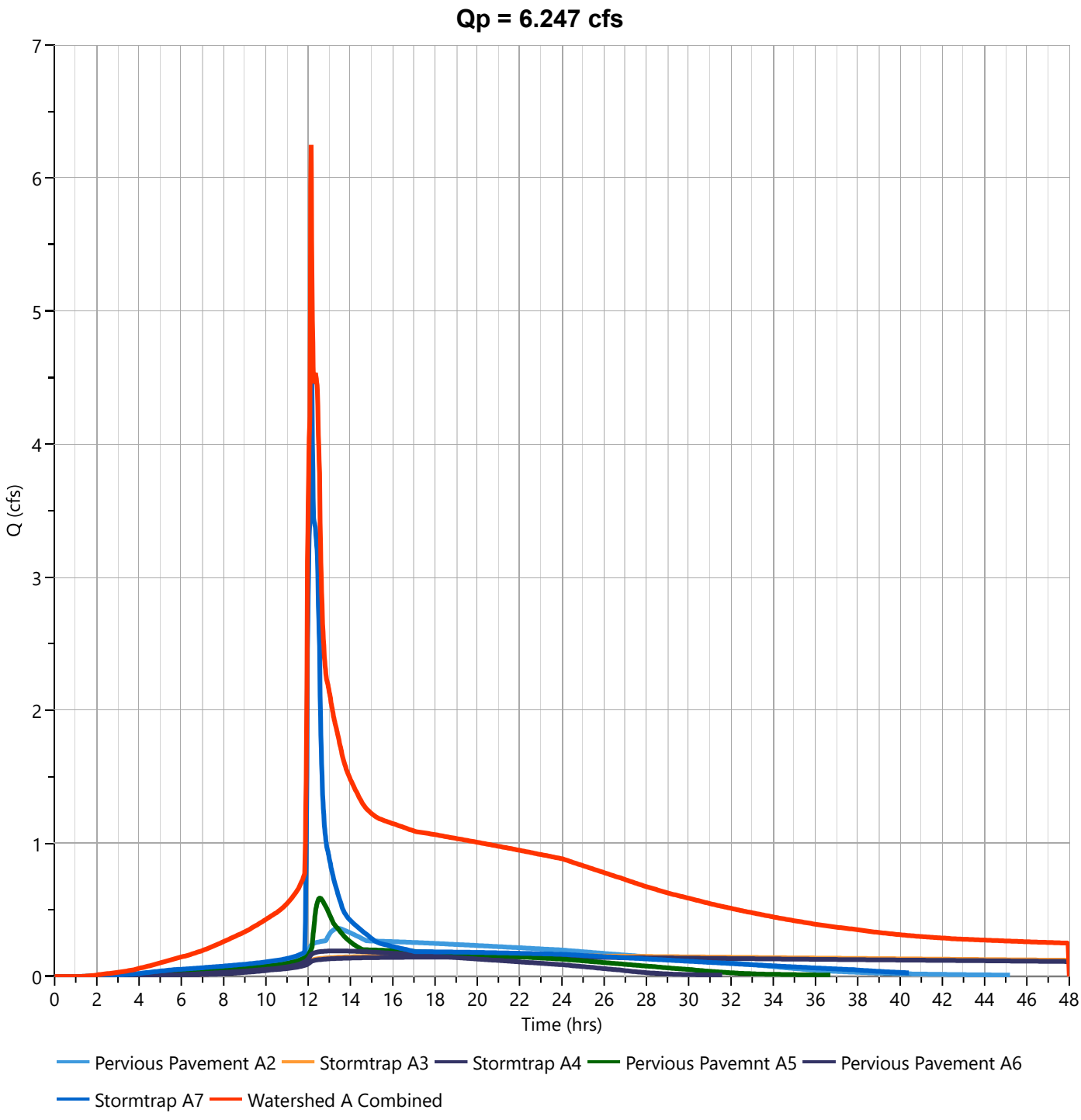
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

Watershed A Combined

Hyd. No. 23

Hydrograph Type	= Junction	Peak Flow	= 6.247 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.13 hrs
Time Interval	= 1 min	Hydrograph Volume	= 106,293 cuft
Inflow Hydrographs		Total Contrib. Area	= 0.0 ac



Hydrograph Report

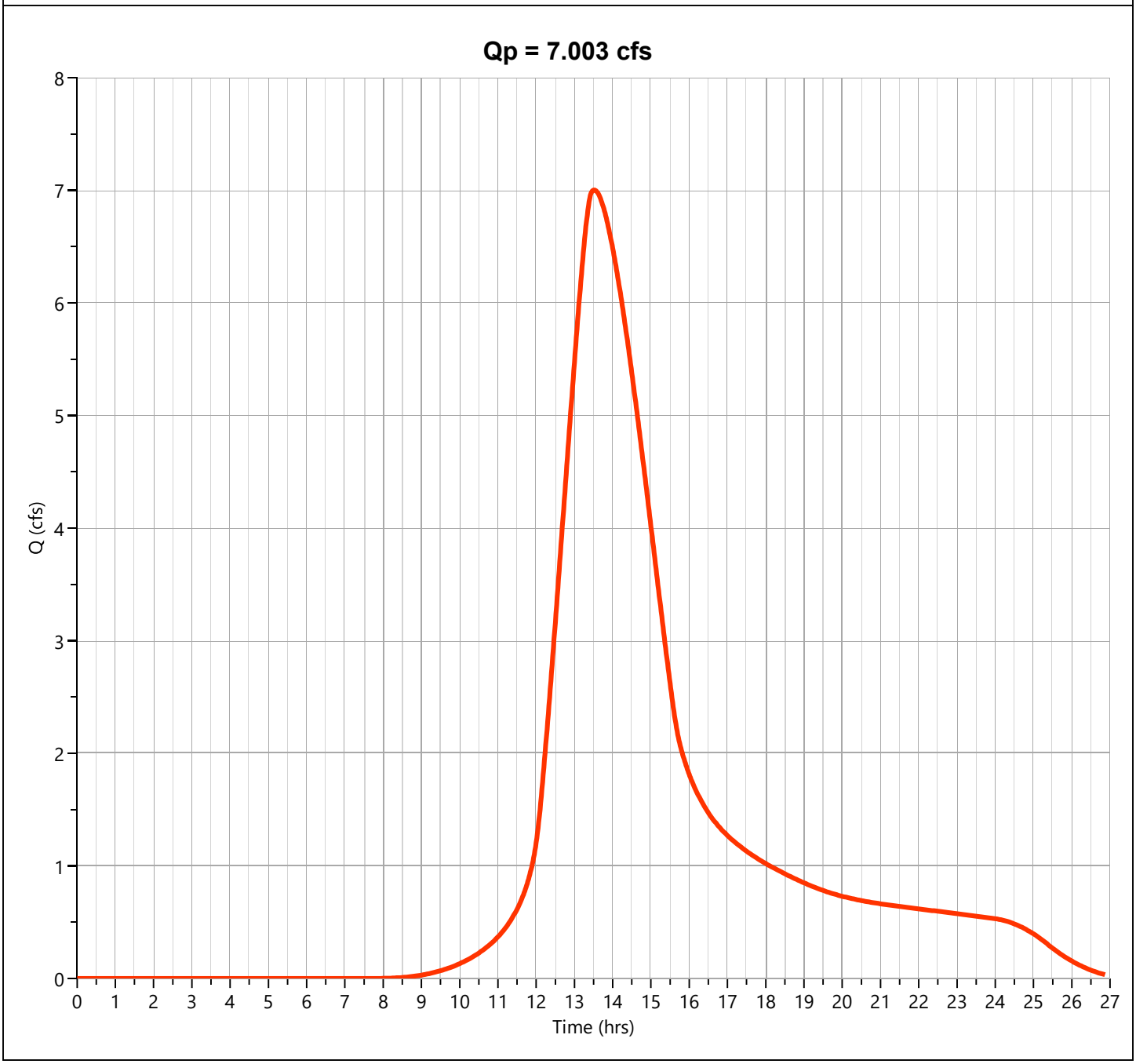
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD B Perv

Hyd. No. 25

Hydrograph Type	= NRCS Runoff	Peak Flow	= 7.003 cfs
Storm Frequency	= 10-yr	Time to Peak	= 13.52 hrs
Time Interval	= 1 min	Runoff Volume	= 95,525 cuft
Drainage Area	= 9.5 ac	Curve Number	= 77.00
Tc Method	= User	Time of Conc. (Tc)	= 138.0 min
Total Rainfall	= 5.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

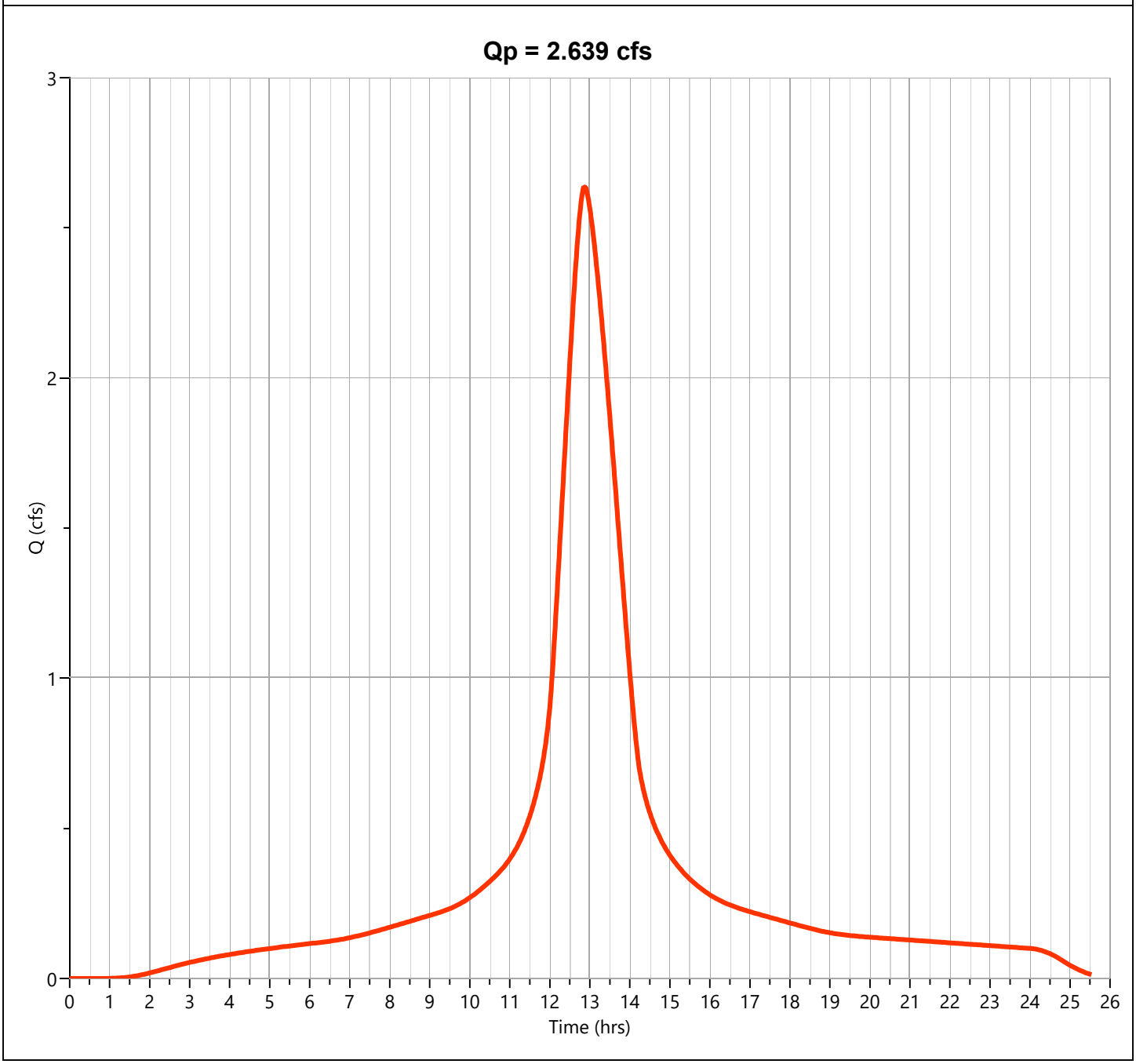
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File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD B Imperv

Hyd. No. 26

Hydrograph Type	= NRCS Runoff	Peak Flow	= 2.639 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.87 hrs
Time Interval	= 1 min	Runoff Volume	= 28,723 cuft
Drainage Area	= 1.6 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 82.0 min
Total Rainfall	= 5.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

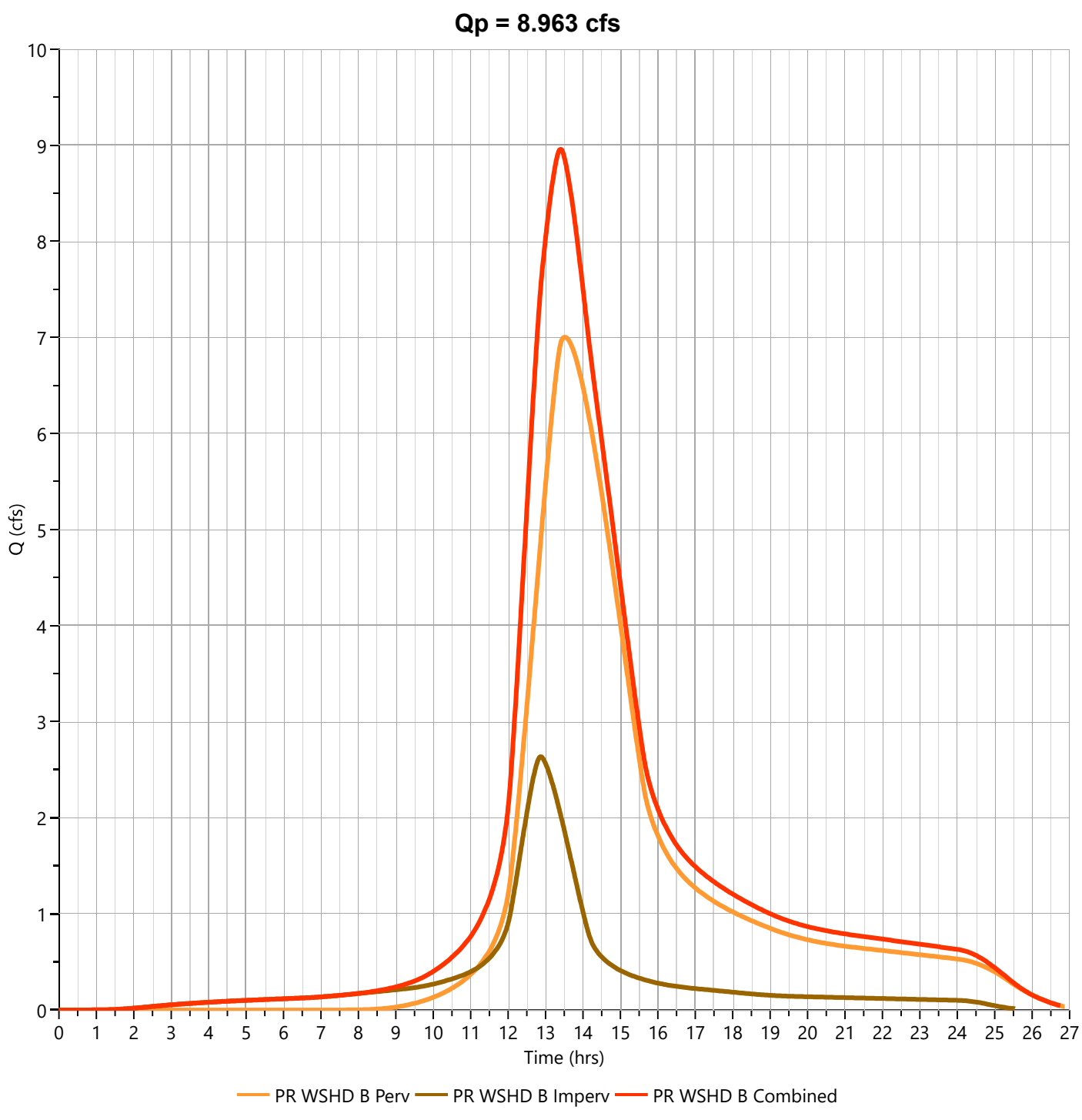
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07-31-2025

PR WSHD B Combined

Hyd. No. 27

Hydrograph Type	= Junction	Peak Flow	= 8.963 cfs
Storm Frequency	= 10-yr	Time to Peak	= 13.40 hrs
Time Interval	= 1 min	Hydrograph Volume	= 124,249 cuft
Inflow Hydrographs	= 25, 26	Total Contrib. Area	= 11.1 ac



Hydrograph Report

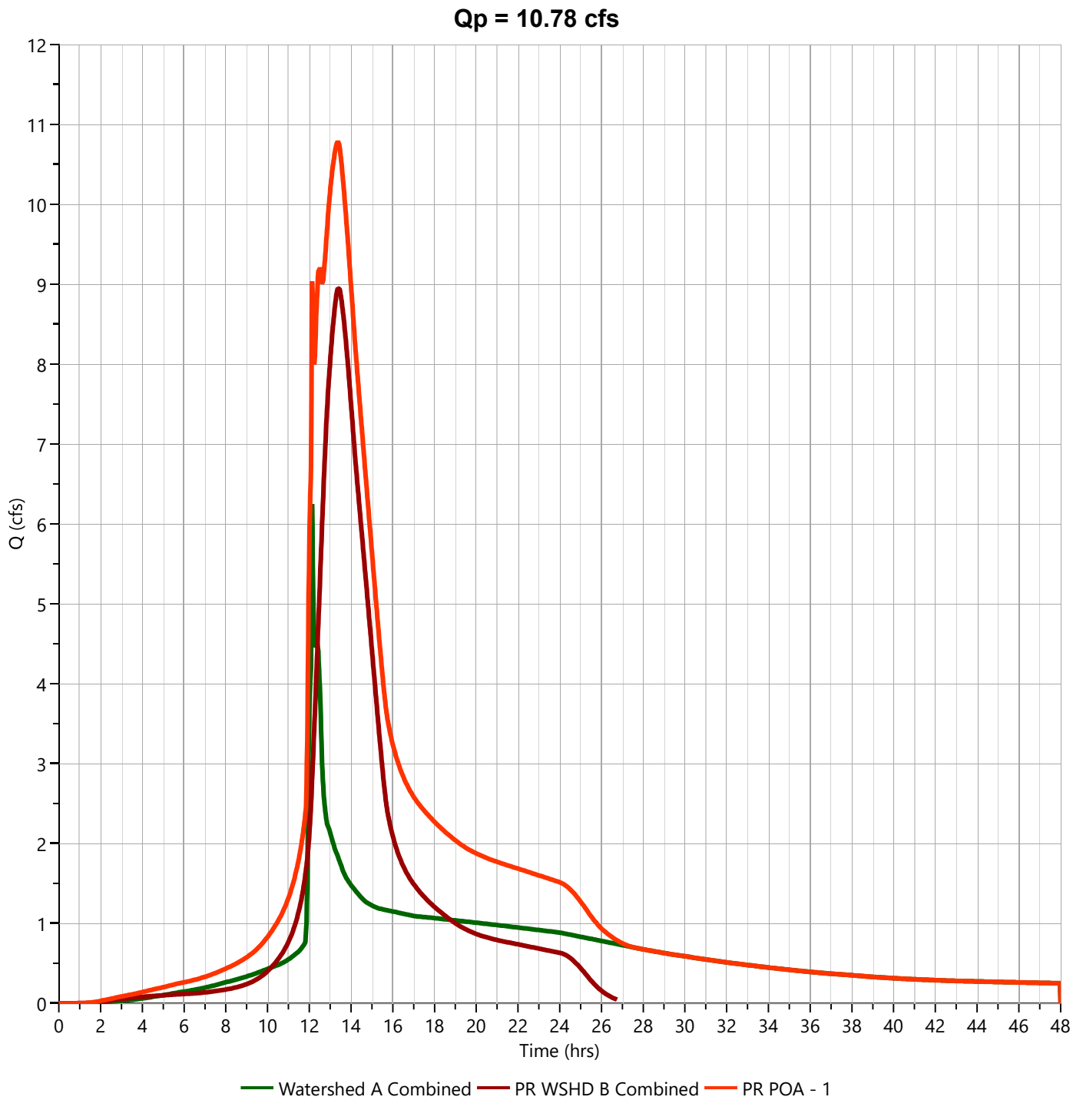
Hydrology Studio v 3.0.0.38

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07-31-2025

PR POA - 1

Hyd. No. 29

Hydrograph Type	= Junction	Peak Flow	= 10.78 cfs
Storm Frequency	= 10-yr	Time to Peak	= 13.37 hrs
Time Interval	= 1 min	Hydrograph Volume	= 230,542 cuft
Inflow Hydrographs		Total Contrib. Area	= 0.0 ac



Hydrograph Report

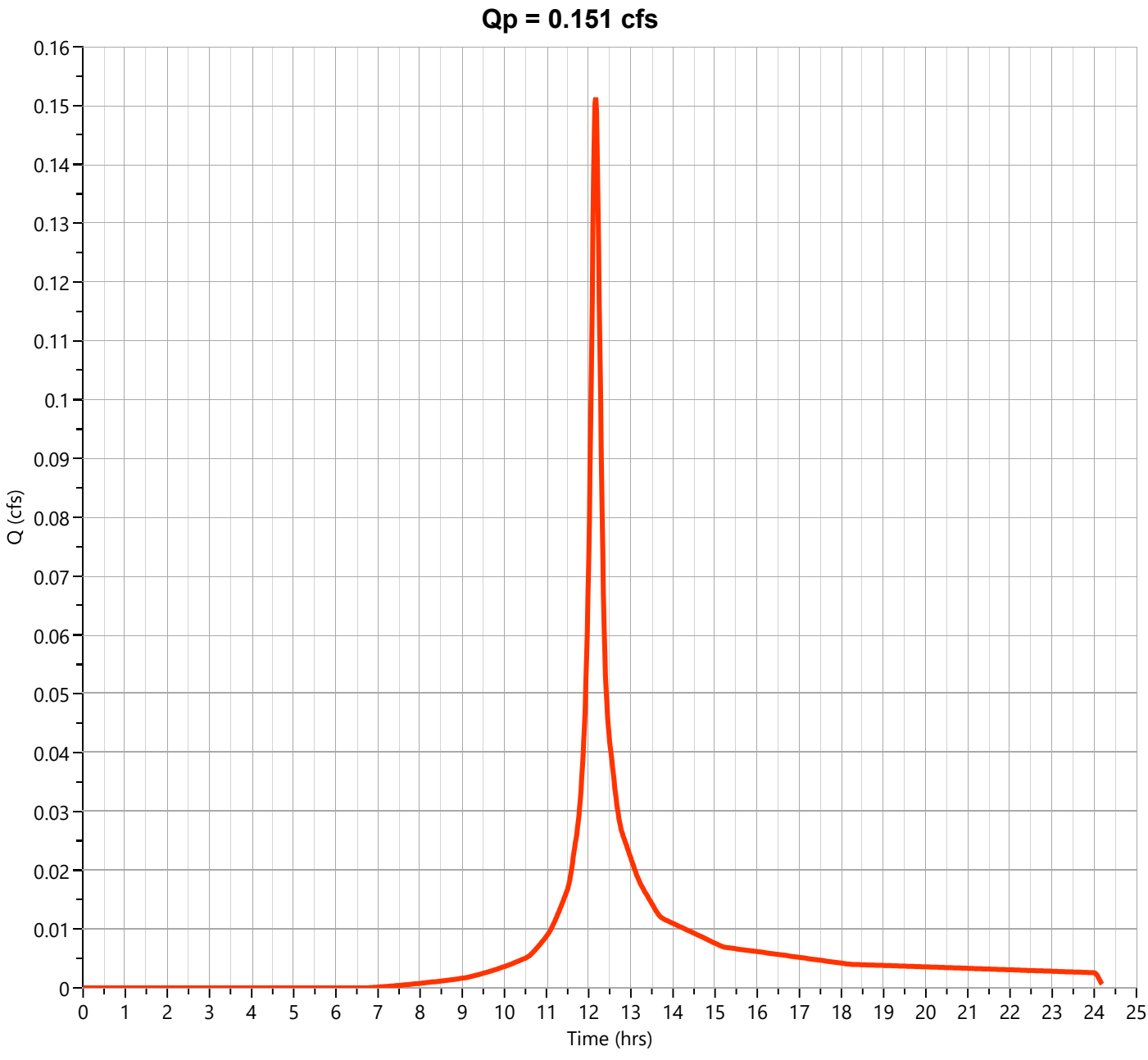
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD C PerPOA 2

Hyd. No. 32

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.151 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.17 hrs
Time Interval	= 1 min	Runoff Volume	= 562 cuft
Drainage Area	= 0.05 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 11.0 min
Total Rainfall	= 5.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph 100-yr Summary

Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys

07-31-2025

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	NRCS Runoff	PR WSDH A2 Imperv	9.979	12.10	33,539	----		
2	Pond Route	Pervious Pavement A2	4.237	12.15	33,054	1	2.50	14,714
4	NRCS Runoff	PR WSHD A3 Imperv	21.22	12.10	72,793	----		
5	Pond Route	Stormtrap A3	0.217	24.02	27,953	4	3.61	61,986
7	NRCS Runoff	PR WSHD A4 Imperv	19.14	12.10	65,638	----		
8	Pond Route	Stormtrap A4	0.204	24.00	26,179	7	3.40	55,506
10	NRCS Runoff	PR WSHD A5 Imperv	6.682	12.10	22,456	----		
11	NRCS Runoff	PR WSHD A5 Perv	0.377	12.10	1,087	----		
12	Junction	PR WSHD A5 Combined	7.059	12.10	23,543	10, 11		
13	Pond Route	Pervious Pavemnt A5	4.200	12.13	23,499	12	5.78	8,825
15	NRCS Runoff	PR WSHD A6 Imperv	3.558	12.10	11,957	----		
16	NRCS Runoff	PR WSHD A6 Perv	1.095	12.15	3,942	----		
17	Junction	PR WSHD A6 Combined	4.560	12.10	15,899	15, 16		
18	Pond Route	Pervious Pavement A6	1.284	12.32	15,883	17	5.01	6,828
20	NRCS Runoff	PR WSHD A7 Imperv	14.49	12.10	48,704	----		
21	Pond Route	Stormtrap A7	11.71	12.12	48,369	20	5.43	12,419
23	Junction	Watershed A Combined	20.49	12.13	174,935	2, 5, 8, 13, 18, 21		
25	NRCS Runoff	PR WSHD B Perv	15.31	13.45	207,994	----		
26	NRCS Runoff	PR WSHD B Imperv	4.517	12.87	49,900	----		
27	Junction	PR WSHD B Combined	18.71	13.40	257,894	25, 26		
29	Junction	PR POA - 1	26.86	12.13	432,829	23, 27		
32	NRCS Runoff	PR WSHD C PerPOA 2	0.309	12.17	1,180	----		

Hydrograph Report

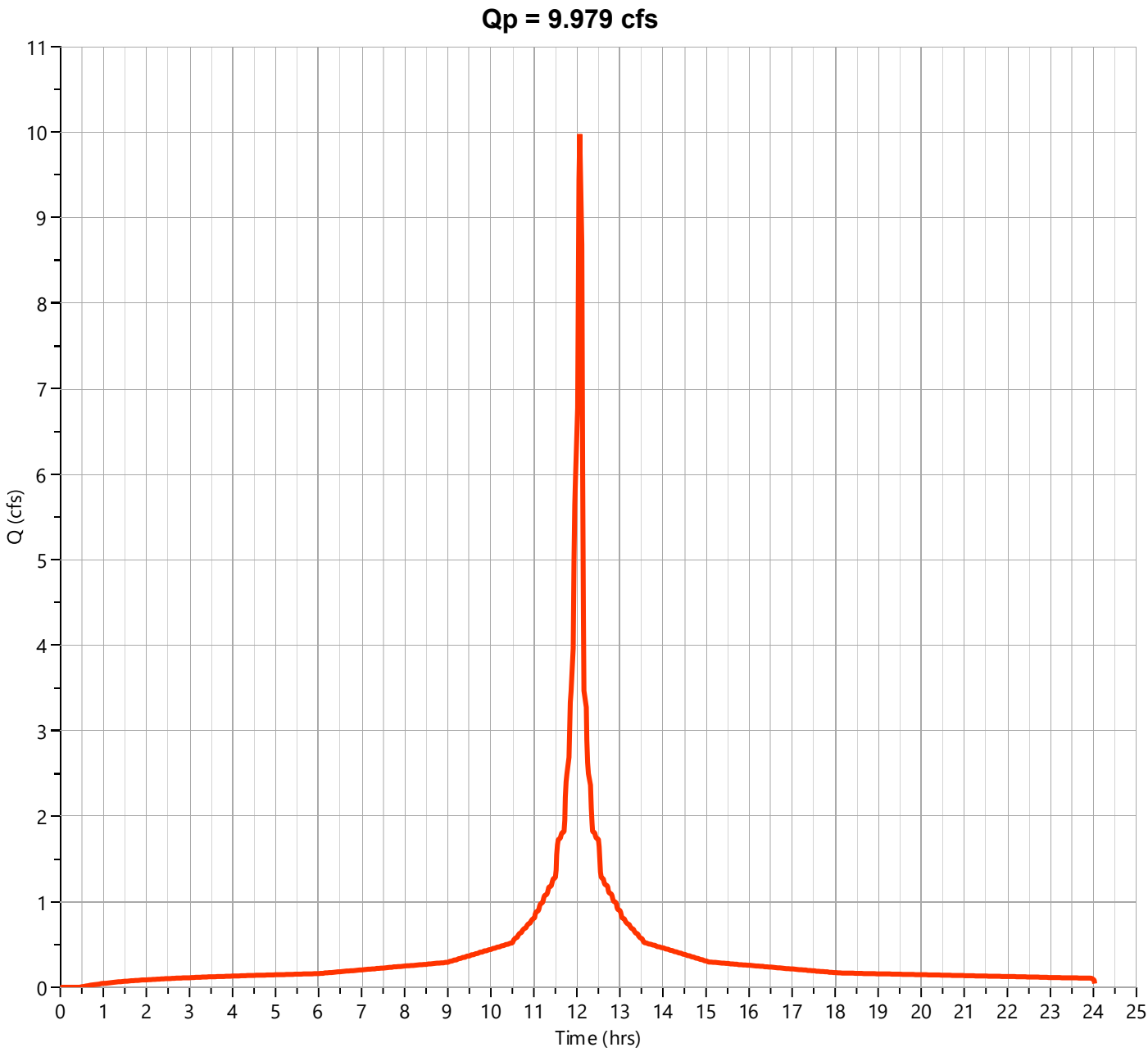
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSDH A2 Imperv

Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 9.979 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 33,539 cuft
Drainage Area	= 1.15 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 3.0 min
Total Rainfall	= 8.81 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.38

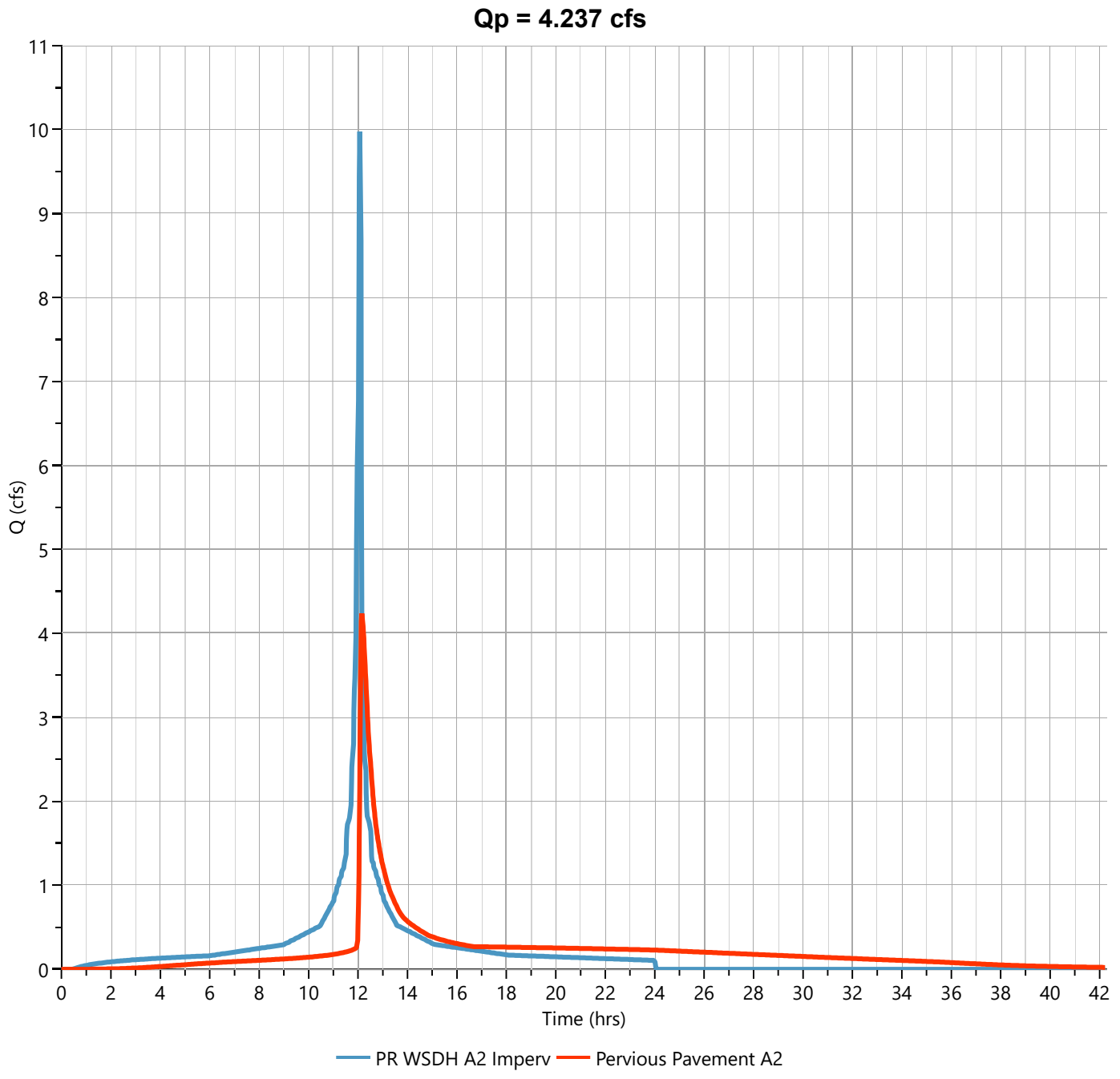
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07-31-2025

Pervious Pavement A2 — Hyd. No. 2

Hydrograph Type	= Pond Route	Peak Flow	= 4.237 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.15 hrs
Time Interval	= 1 min	Hydrograph Volume	= 33,054 cuft
Inflow Hydrograph	= 1 - PR WSDH A2 Imperv	Max. Elevation	= 2.50 ft
Pond Name	= Pervious Pavement A2	Max. Storage	= 14,714 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 5.59 hrs



Hydrograph Report

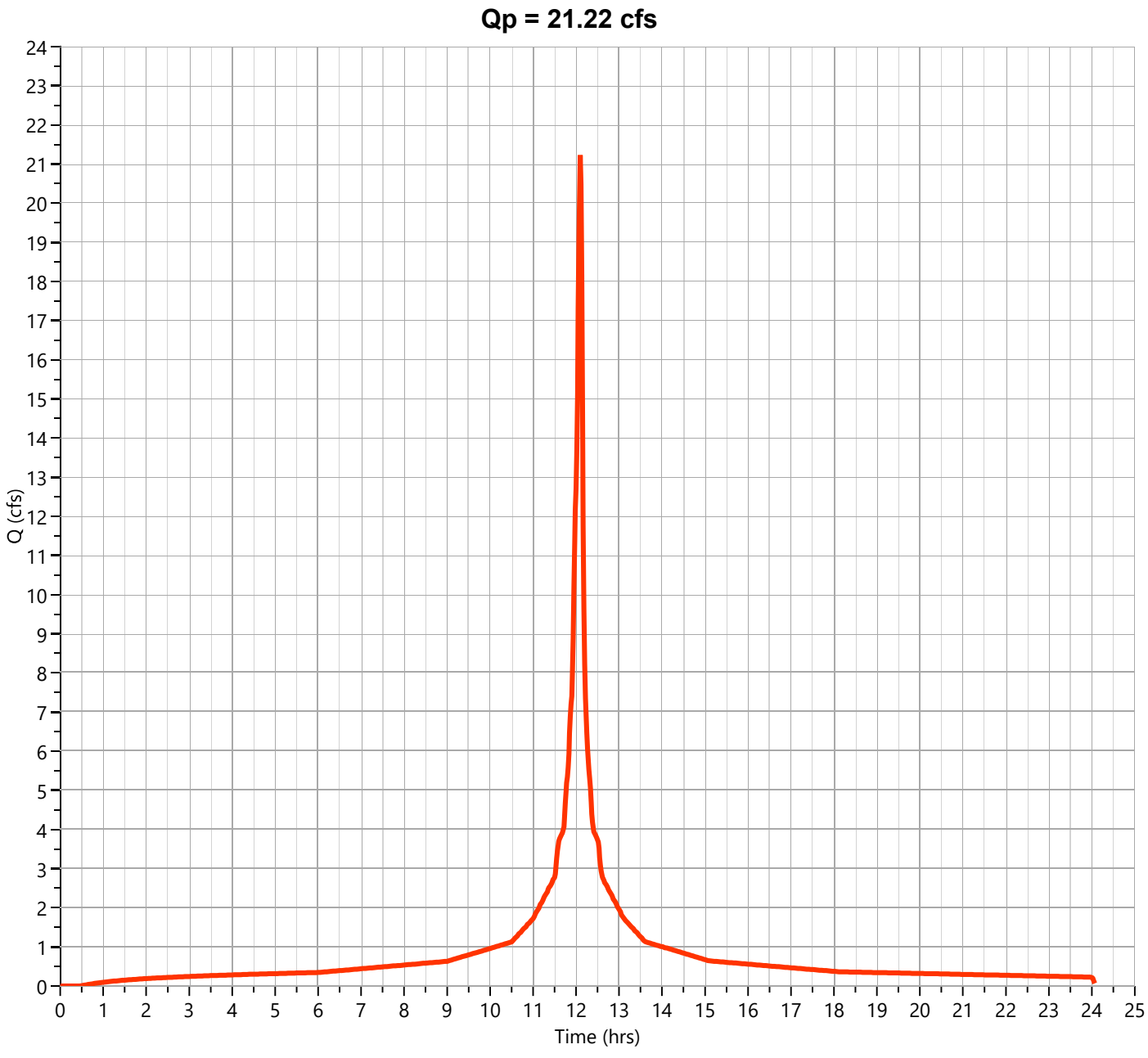
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File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A3 Imperv

Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 21.22 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 72,793 cuft
Drainage Area	= 2.34 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 4.0 min
Total Rainfall	= 8.81 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.38

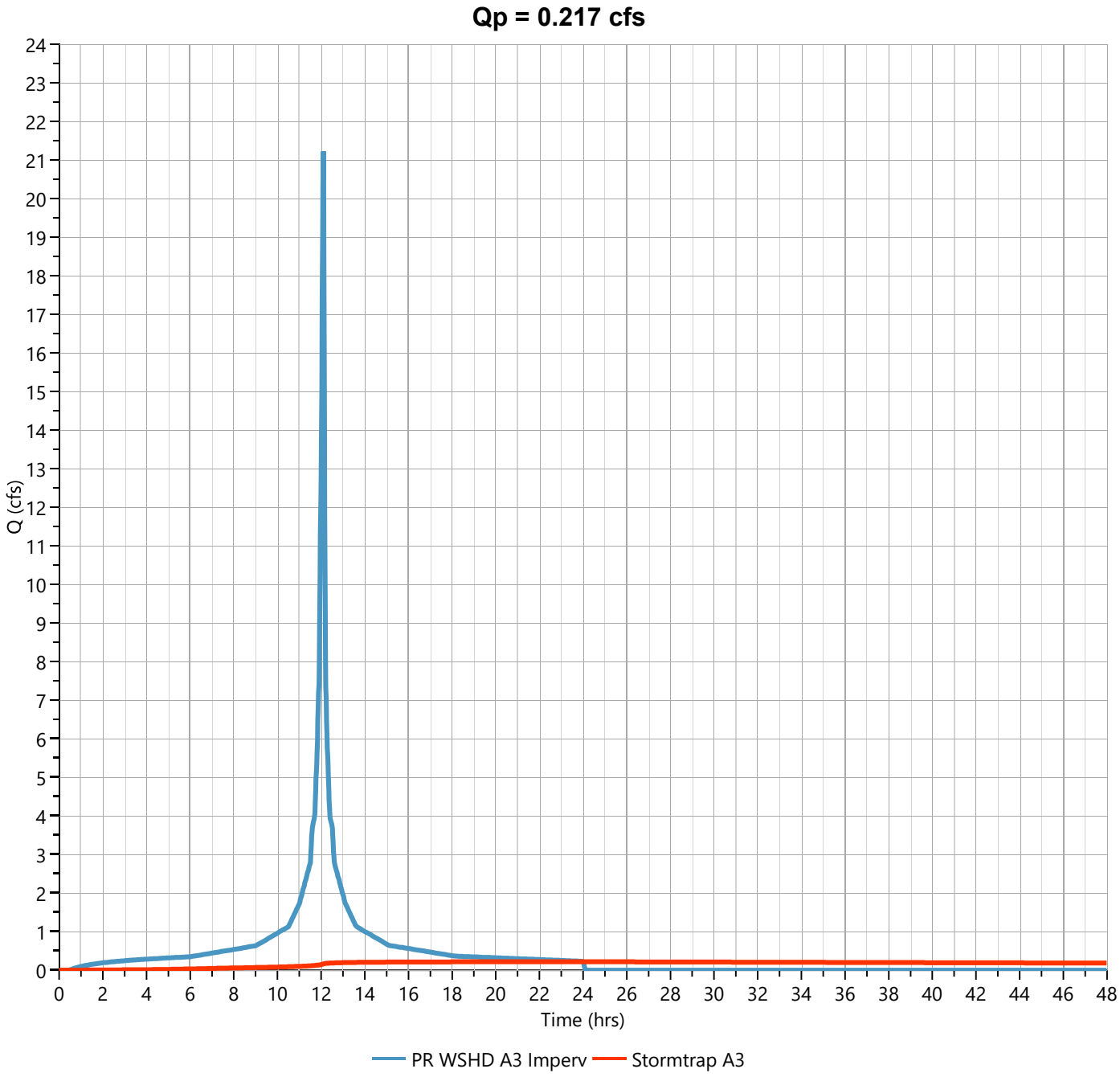
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07-31-2025

Stormtrap A3 — Hyd. No. 5

Hydrograph Type	= Pond Route	Peak Flow	= 0.217 cfs
Storm Frequency	= 100-yr	Time to Peak	= 24.02 hrs
Time Interval	= 1 min	Hydrograph Volume	= 27,953 cuft
Inflow Hydrograph	= 4 - PR WSHD A3 Imperv	Max. Elevation	= 3.61 ft
Pond Name	= Stormtrap A3	Max. Storage	= 61,986 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 16.04 hrs



Hydrograph Report

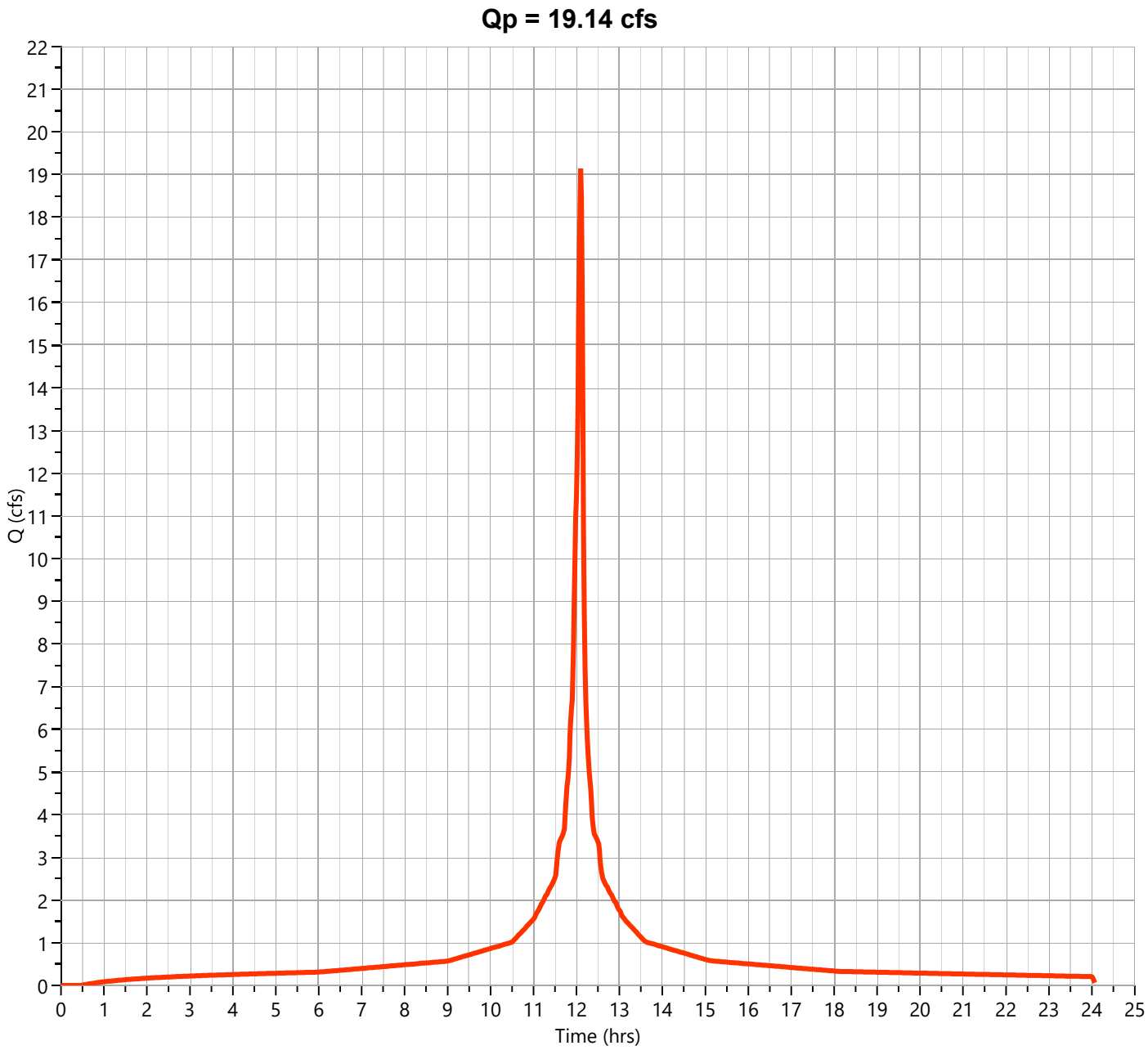
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File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A4 Imperv

Hyd. No. 7

Hydrograph Type	= NRCS Runoff	Peak Flow	= 19.14 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 65,638 cuft
Drainage Area	= 2.11 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 4.0 min
Total Rainfall	= 8.81 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.38

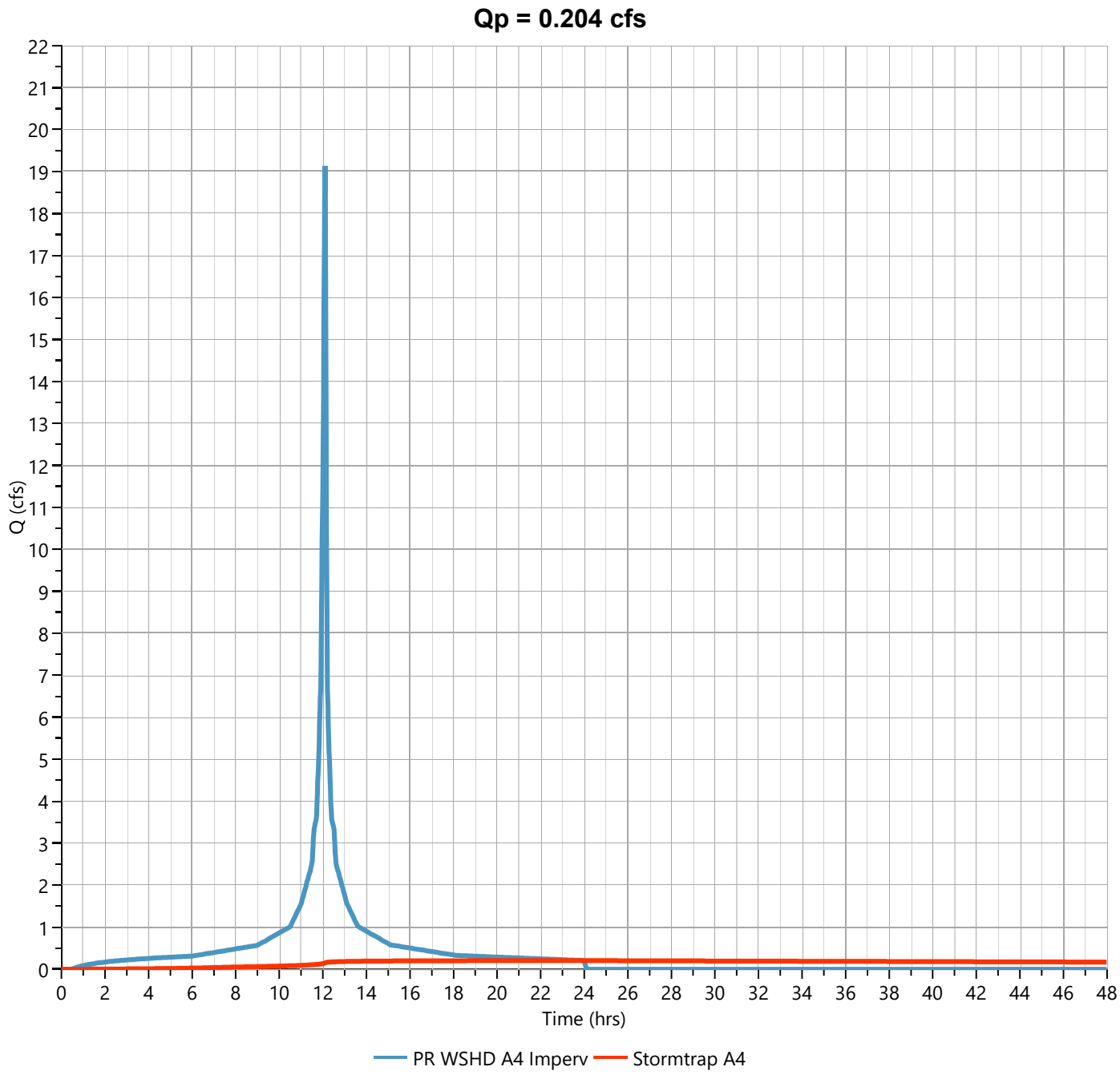
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07-31-2025

Stormtrap A4 — Hyd. No. 8

Hydrograph Type	= Pond Route	Peak Flow	= 0.204 cfs
Storm Frequency	= 100-yr	Time to Peak	= 24.00 hrs
Time Interval	= 1 min	Hydrograph Volume	= 26,179 cuft
Inflow Hydrograph	= 7 - PR WSHD A4 Imperv	Max. Elevation	= 3.40 ft
Pond Name	= Stormtrap A4	Max. Storage	= 55,506 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 16.04 hrs



Hydrograph Report

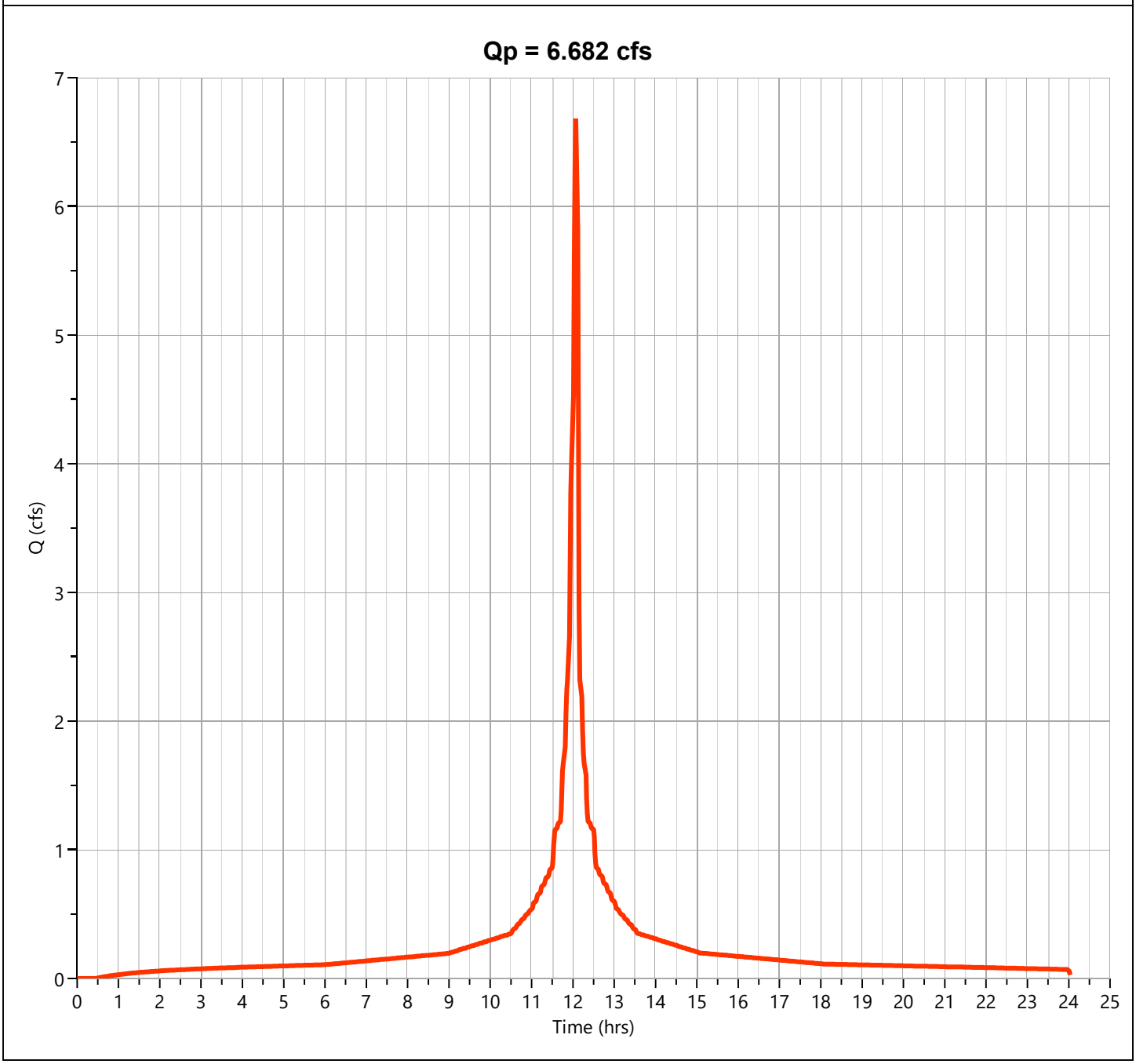
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A5 Imperv

Hyd. No. 10

Hydrograph Type	= NRCS Runoff	Peak Flow	= 6.682 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 22,456 cuft
Drainage Area	= 0.77 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 8.81 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

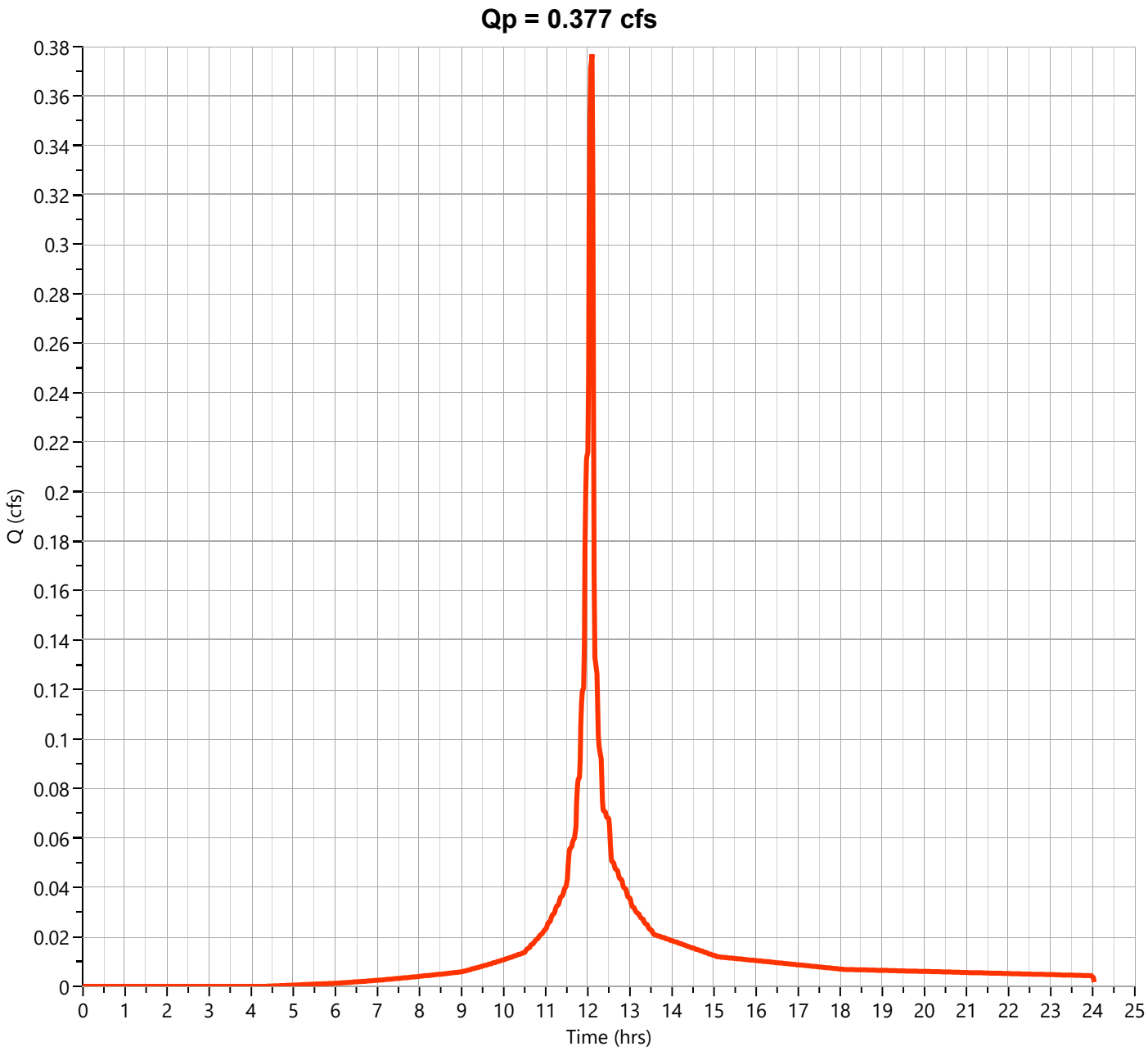
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A5 Perv

Hyd. No. 11

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.377 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 1,087 cuft
Drainage Area	= 0.05 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 8.81 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

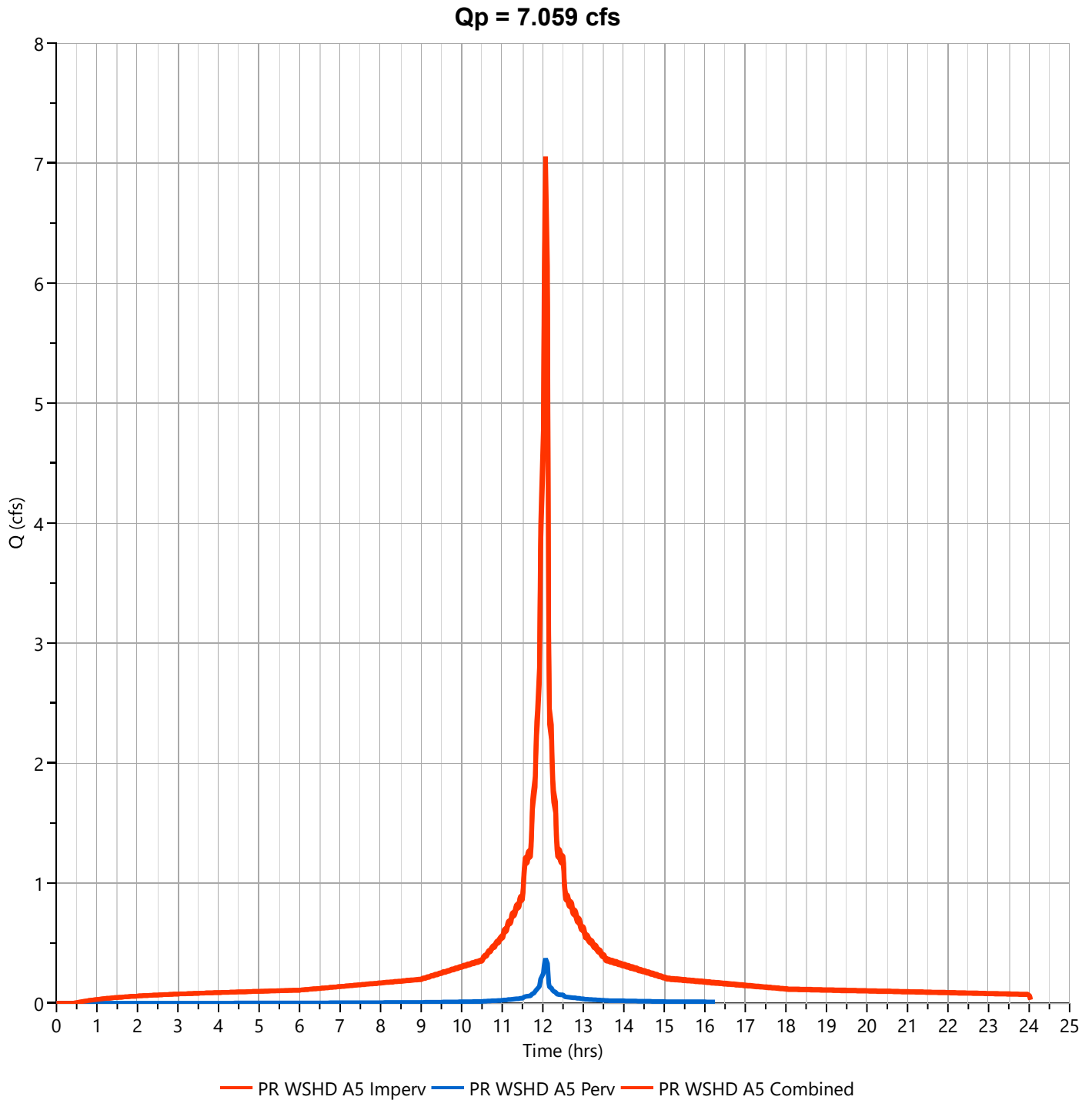
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File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A5 Combined

Hyd. No. 12

Hydrograph Type	= Junction	Peak Flow	= 7.059 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Hydrograph Volume	= 23,543 cuft
Inflow Hydrographs	= 10, 11	Total Contrib. Area	= 0.82 ac



Hydrograph Report

Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

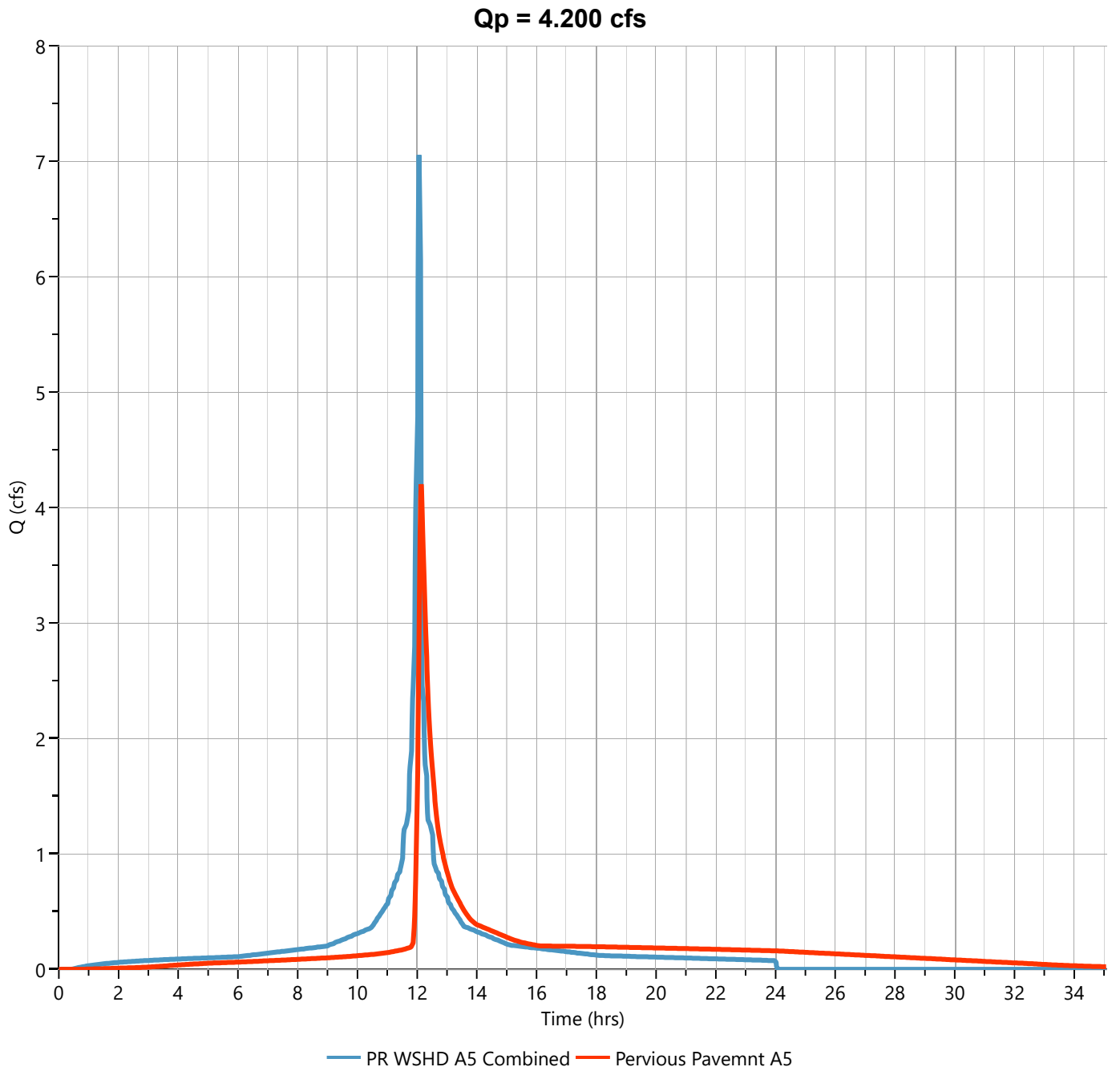
Pervious Pavemnt A5

Hyd. No. 13

Hydrograph Type	= Pond Route	Peak Flow	= 4.200 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 1 min	Hydrograph Volume	= 23,499 cuft
Inflow Hydrograph	= 12 - PR WSHD A5 Combined	Max. Elevation	= 5.78 ft
Pond Name	= Pervious Pavement A5	Max. Storage	= 8,825 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 3.76 hrs



Hydrograph Report

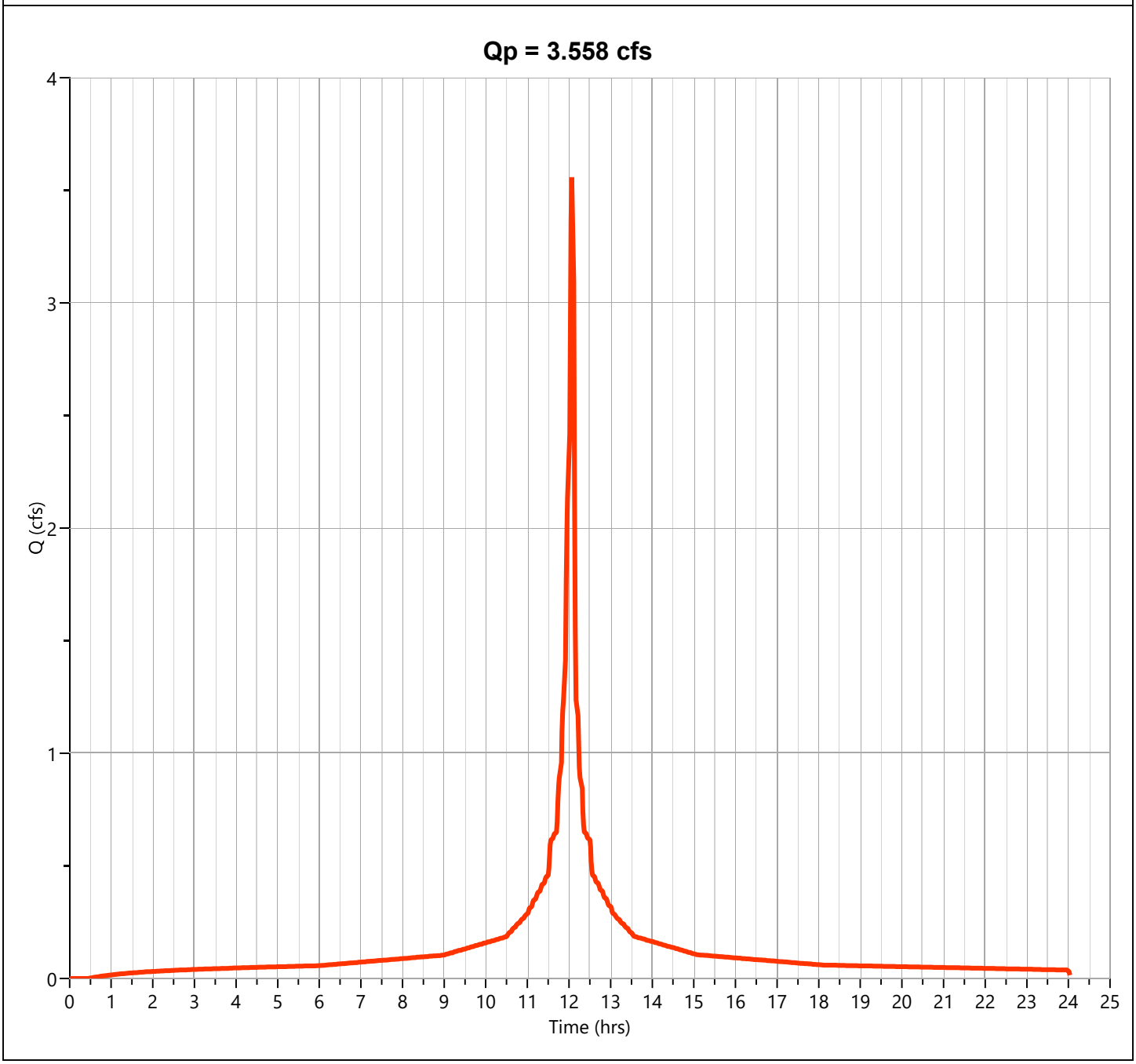
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A6 Imperv

Hyd. No. 15

Hydrograph Type	= NRCS Runoff	Peak Flow	= 3.558 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 11,957 cuft
Drainage Area	= 0.41 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 8.81 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

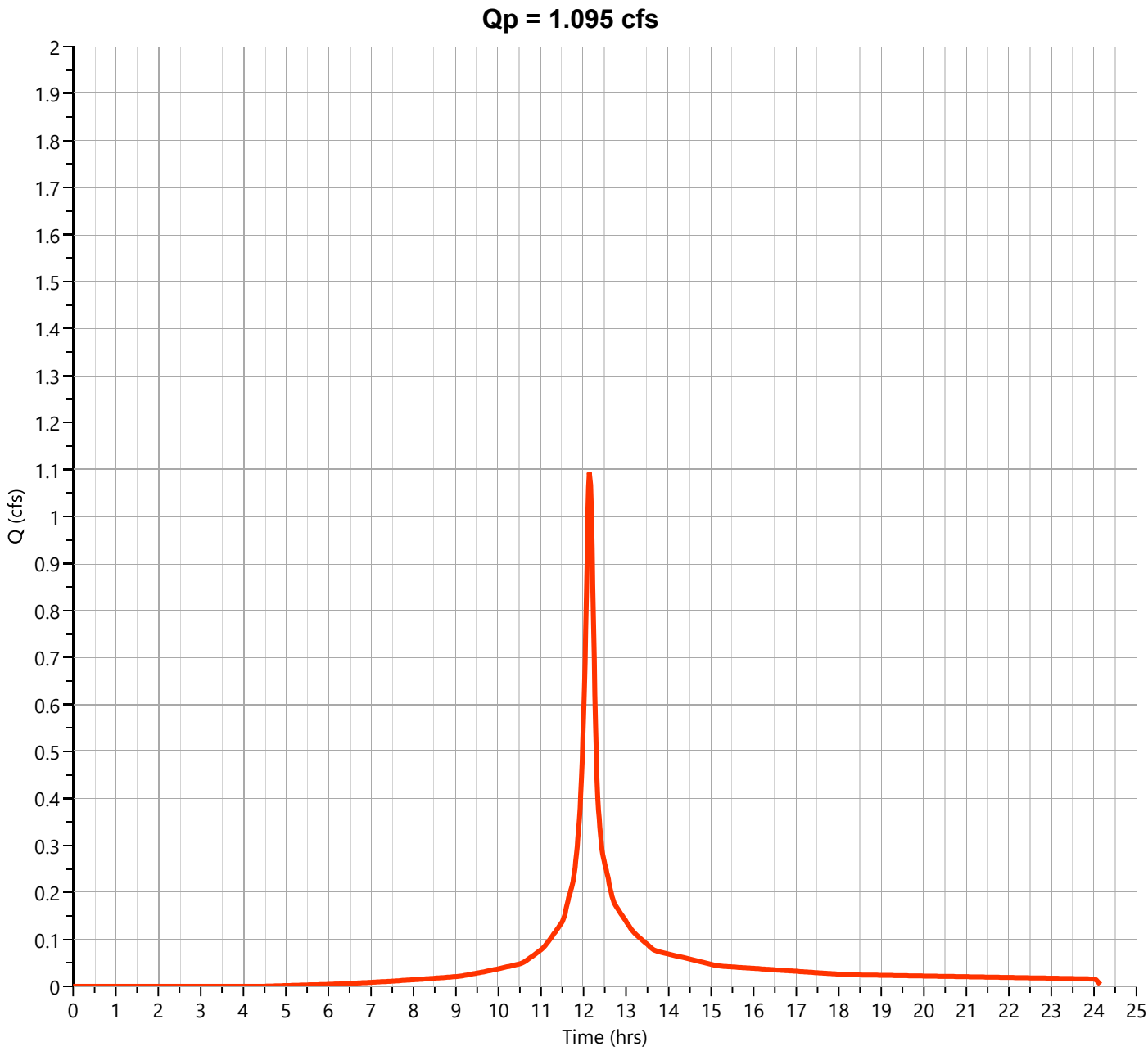
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A6 Perv

Hyd. No. 16

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.095 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.15 hrs
Time Interval	= 1 min	Runoff Volume	= 3,942 cuft
Drainage Area	= 0.17 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 9.0 min
Total Rainfall	= 8.81 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

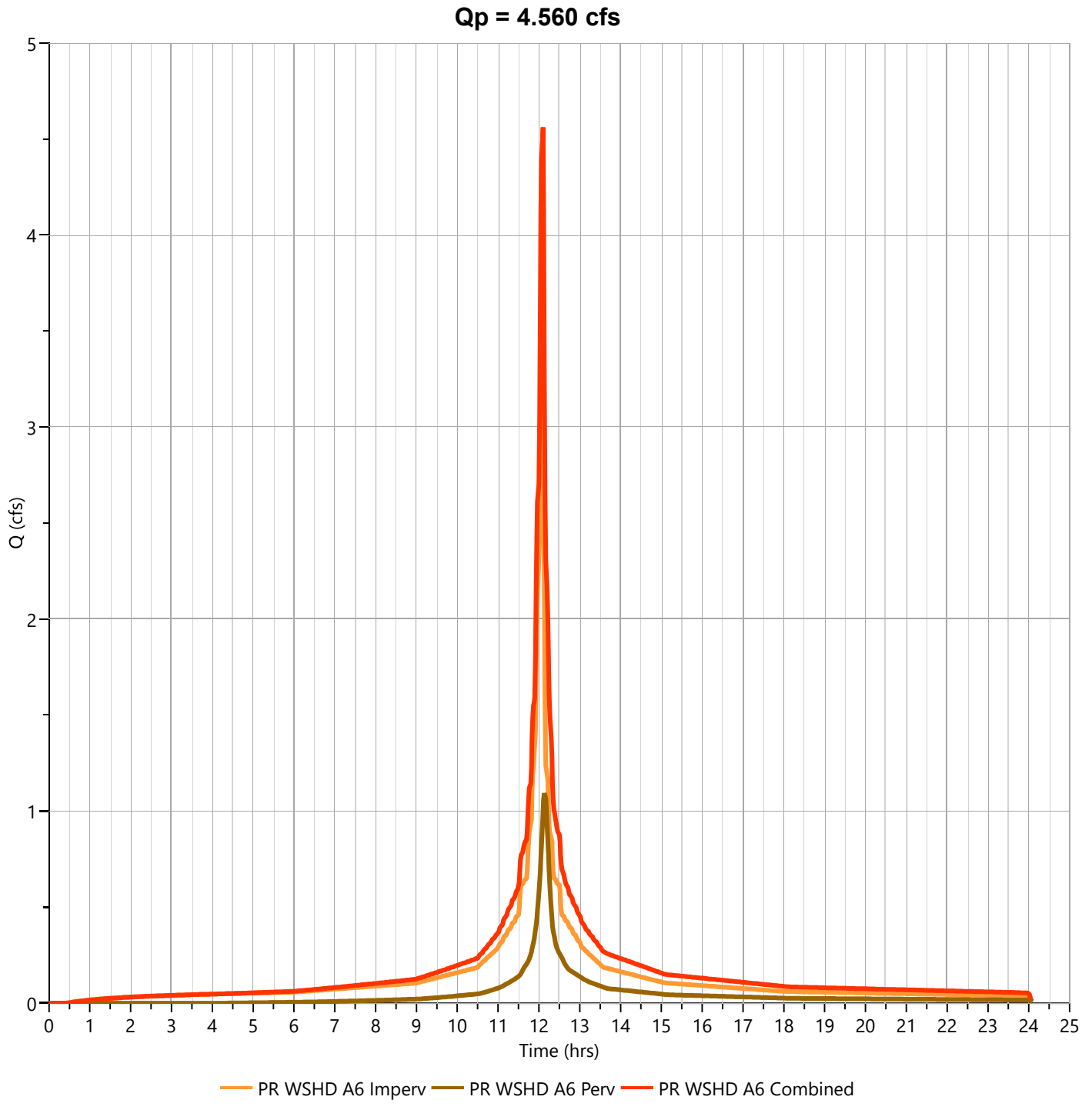
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A6 Combined

Hyd. No. 17

Hydrograph Type	= Junction	Peak Flow	= 4.560 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Hydrograph Volume	= 15,899 cuft
Inflow Hydrographs	= 15, 16	Total Contrib. Area	= 0.58 ac



Hydrograph Report

Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

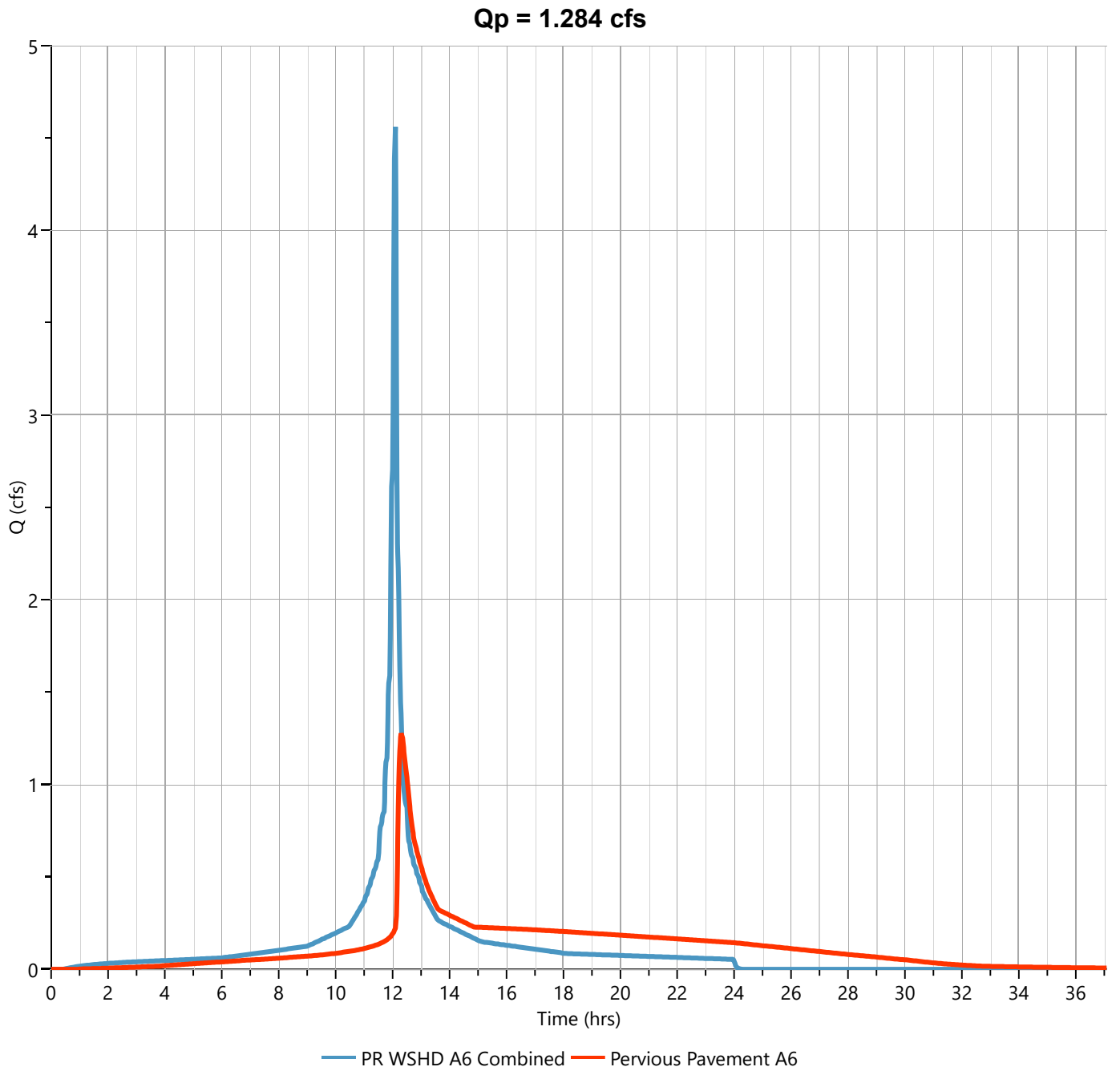
Pervious Pavement A6

Hyd. No. 18

Hydrograph Type	= Pond Route	Peak Flow	= 1.284 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.32 hrs
Time Interval	= 1 min	Hydrograph Volume	= 15,883 cuft
Inflow Hydrograph	= 17 - PR WSHD A6 Combined	Max. Elevation	= 5.01 ft
Pond Name	= Pervious Pavement A6	Max. Storage	= 6,828 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 4.54 hrs



Hydrograph Report

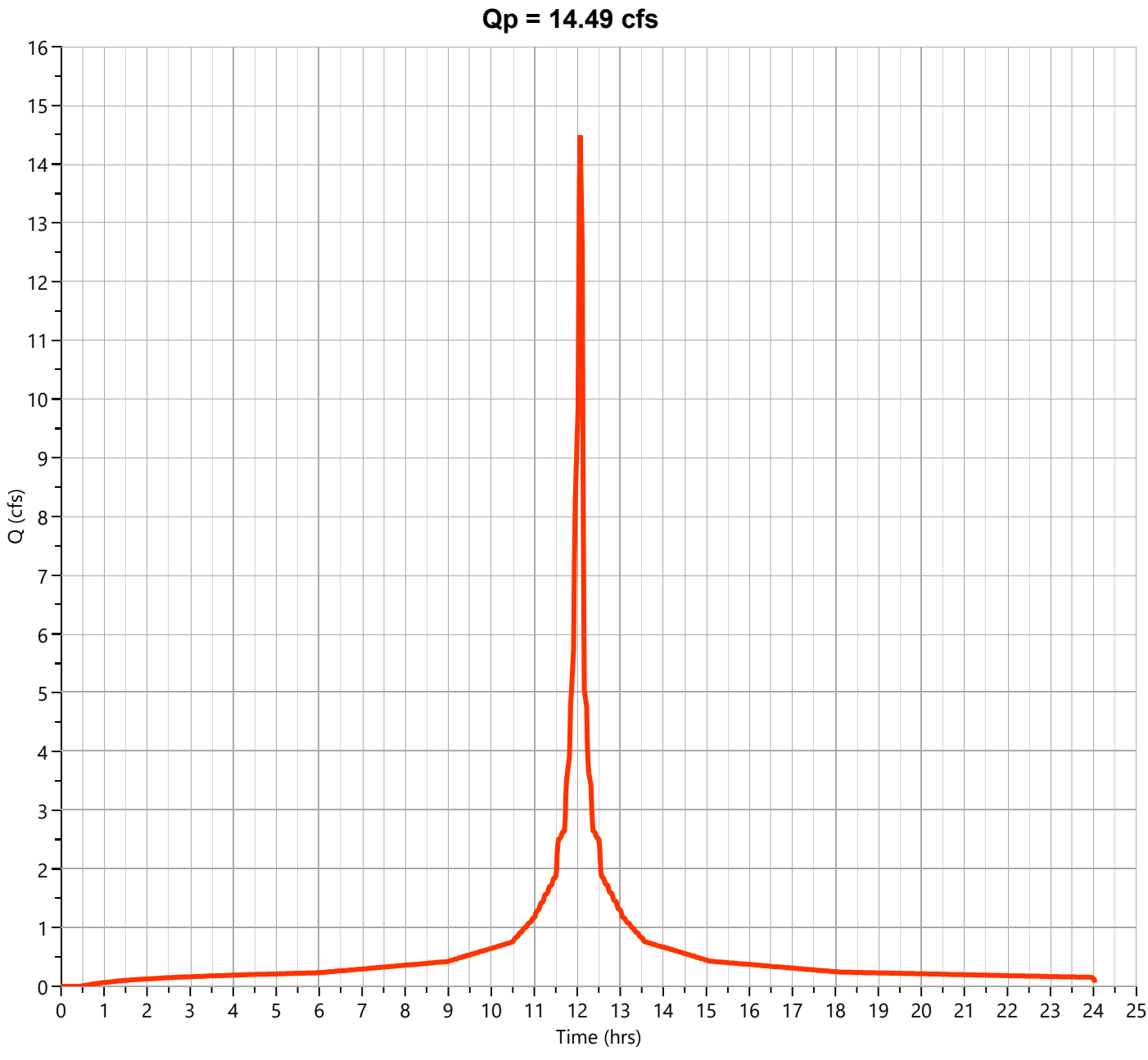
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD A7 Imperv

Hyd. No. 20

Hydrograph Type	= NRCS Runoff	Peak Flow	= 14.49 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 48,704 cuft
Drainage Area	= 1.67 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 8.81 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

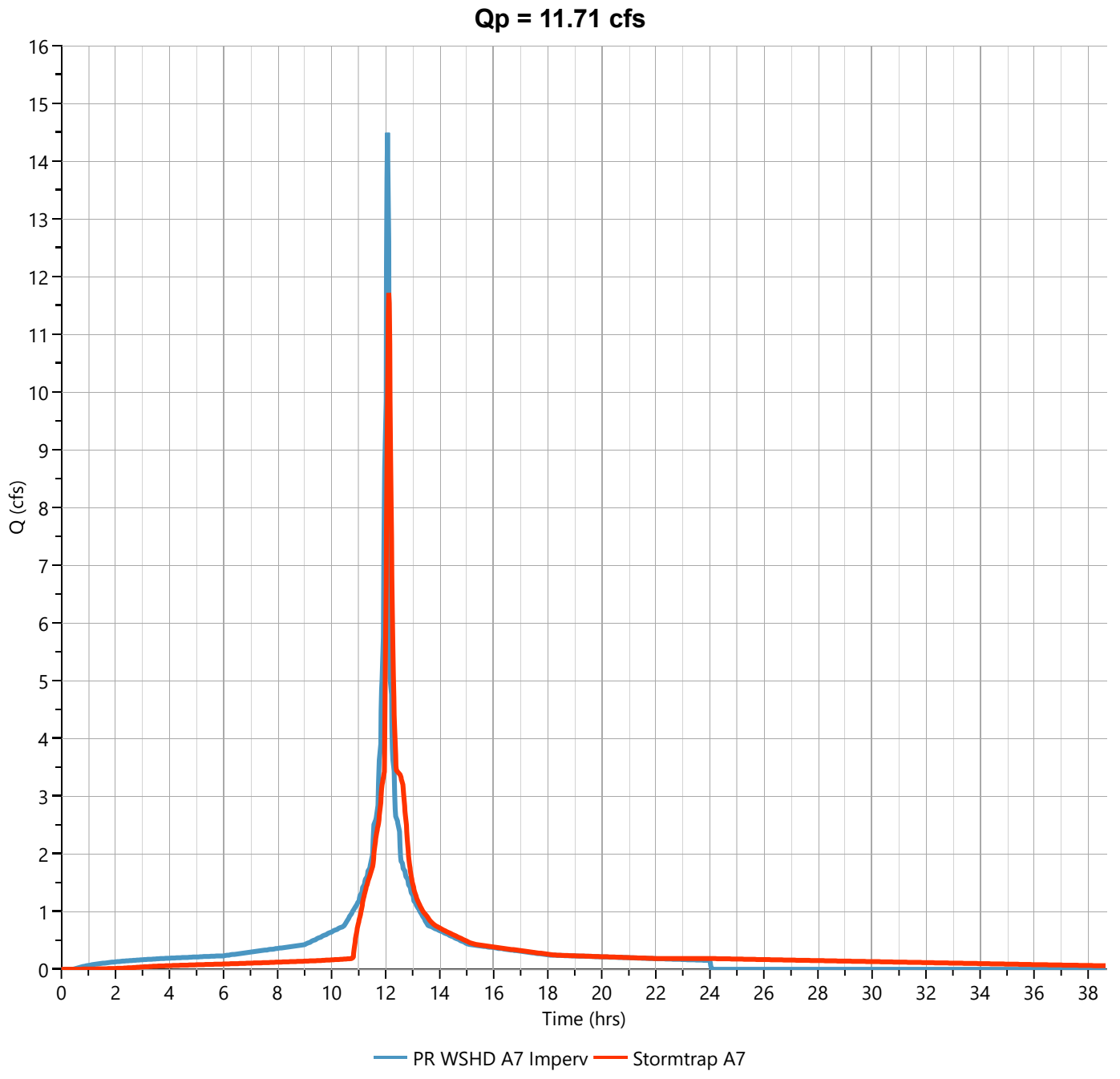
Stormtrap A7

Hyd. No. 21

Hydrograph Type	= Pond Route	Peak Flow	= 11.71 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.12 hrs
Time Interval	= 1 min	Hydrograph Volume	= 48,369 cuft
Inflow Hydrograph	= 20 - PR WSHD A7 Imperv	Max. Elevation	= 5.43 ft
Pond Name	= Stormtrap A7	Max. Storage	= 12,419 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 2.35 hrs



Hydrograph Report

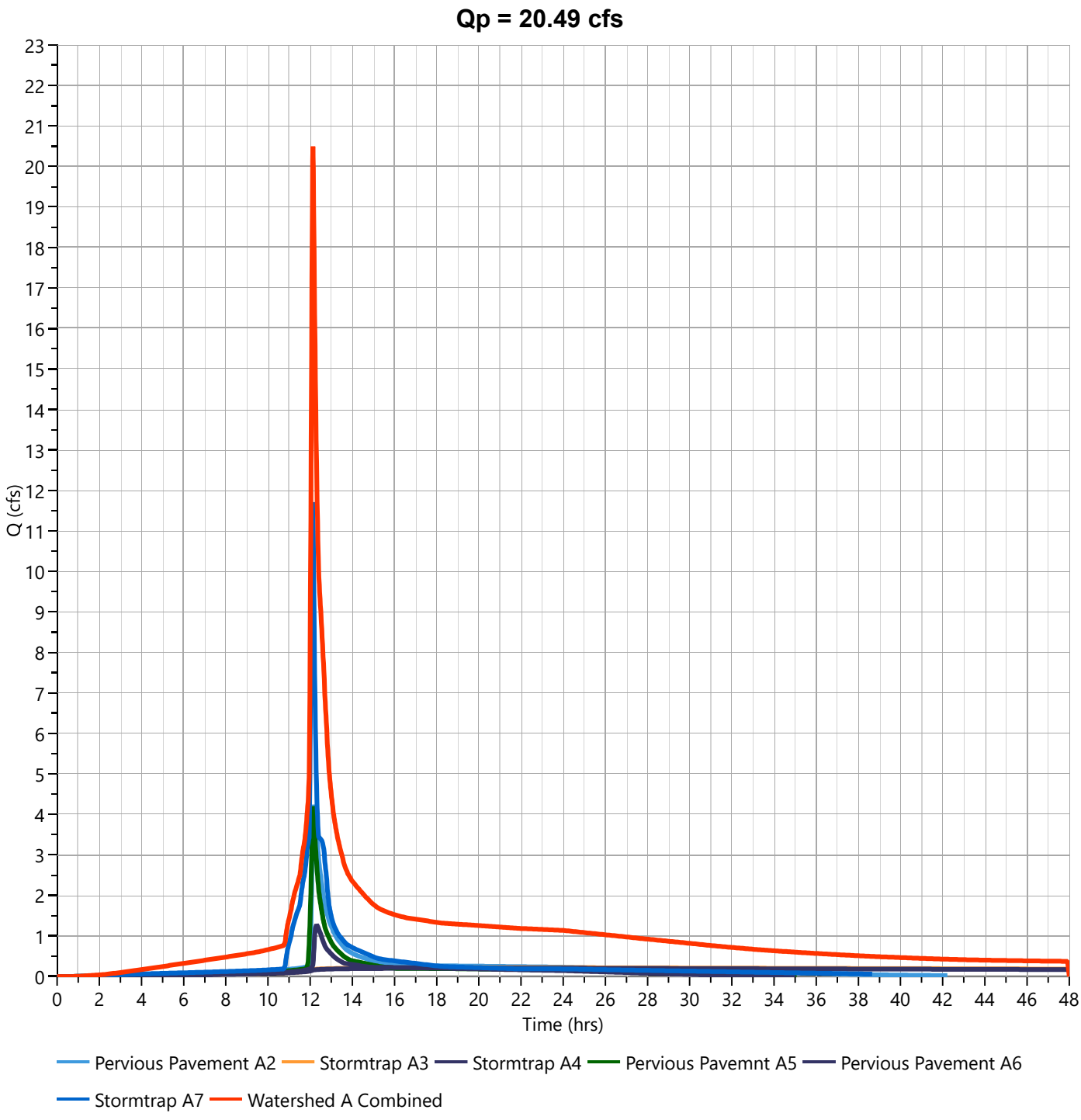
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

Watershed A Combined

Hyd. No. 23

Hydrograph Type	= Junction	Peak Flow	= 20.49 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 1 min	Hydrograph Volume	= 174,935 cuft
Inflow Hydrographs		Total Contrib. Area	= 0.0 ac



Hydrograph Report

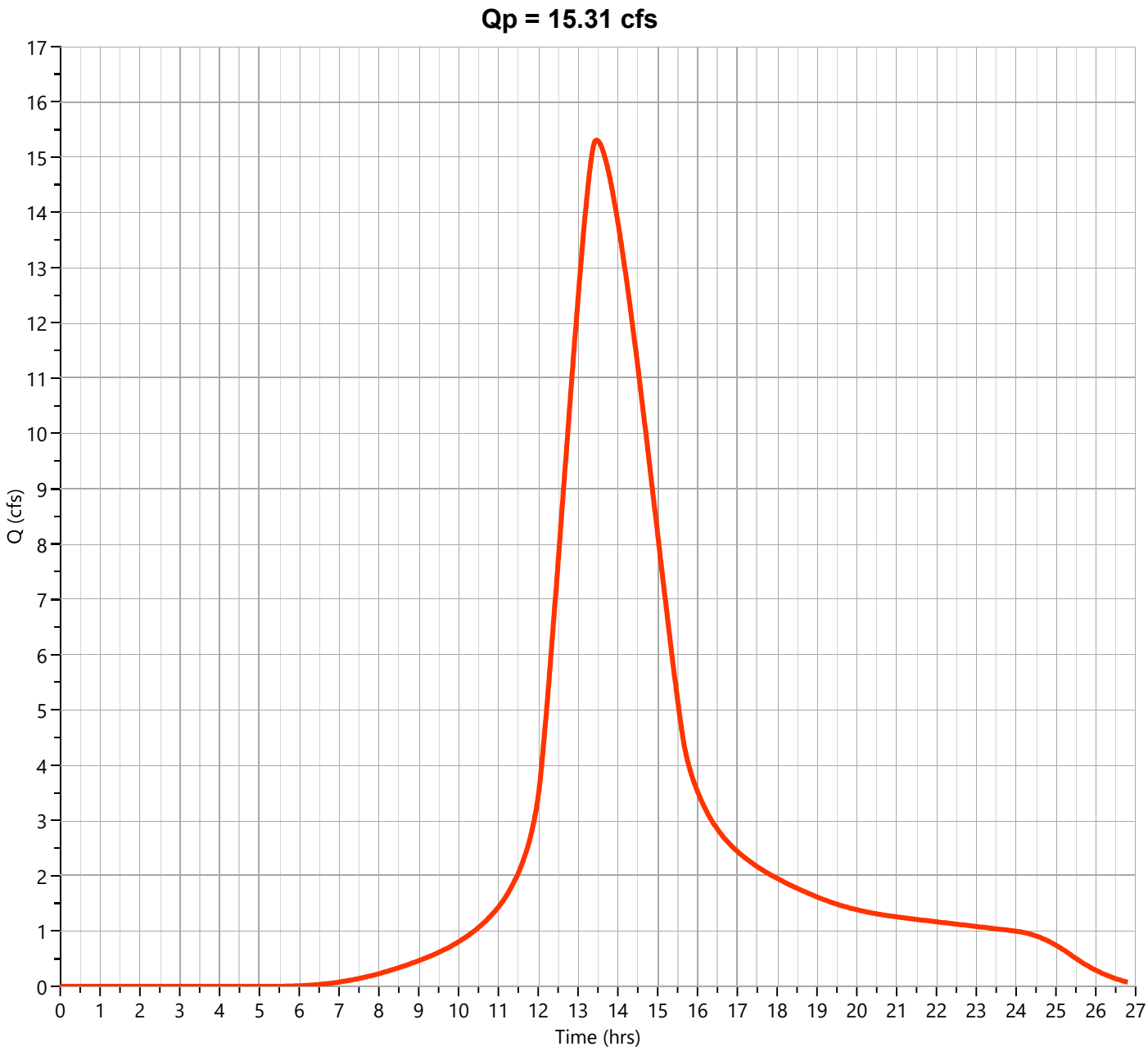
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD B Perv

Hyd. No. 25

Hydrograph Type	= NRCS Runoff	Peak Flow	= 15.31 cfs
Storm Frequency	= 100-yr	Time to Peak	= 13.45 hrs
Time Interval	= 1 min	Runoff Volume	= 207,994 cuft
Drainage Area	= 9.5 ac	Curve Number	= 77.00
Tc Method	= User	Time of Conc. (Tc)	= 138.0 min
Total Rainfall	= 8.81 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

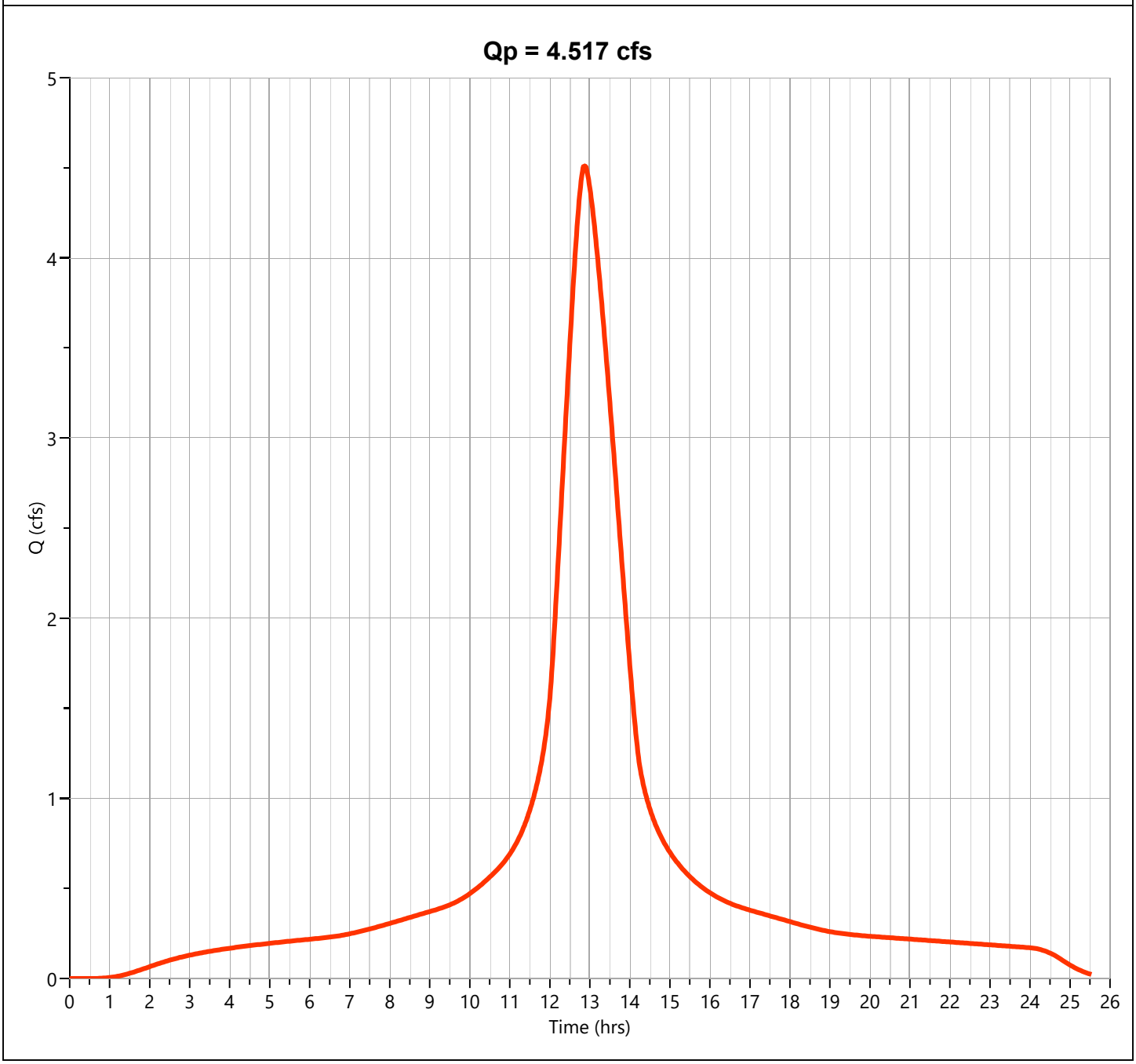
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD B Imperv

Hyd. No. 26

Hydrograph Type	= NRCS Runoff	Peak Flow	= 4.517 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.87 hrs
Time Interval	= 1 min	Runoff Volume	= 49,900 cuft
Drainage Area	= 1.6 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 82.0 min
Total Rainfall	= 8.81 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

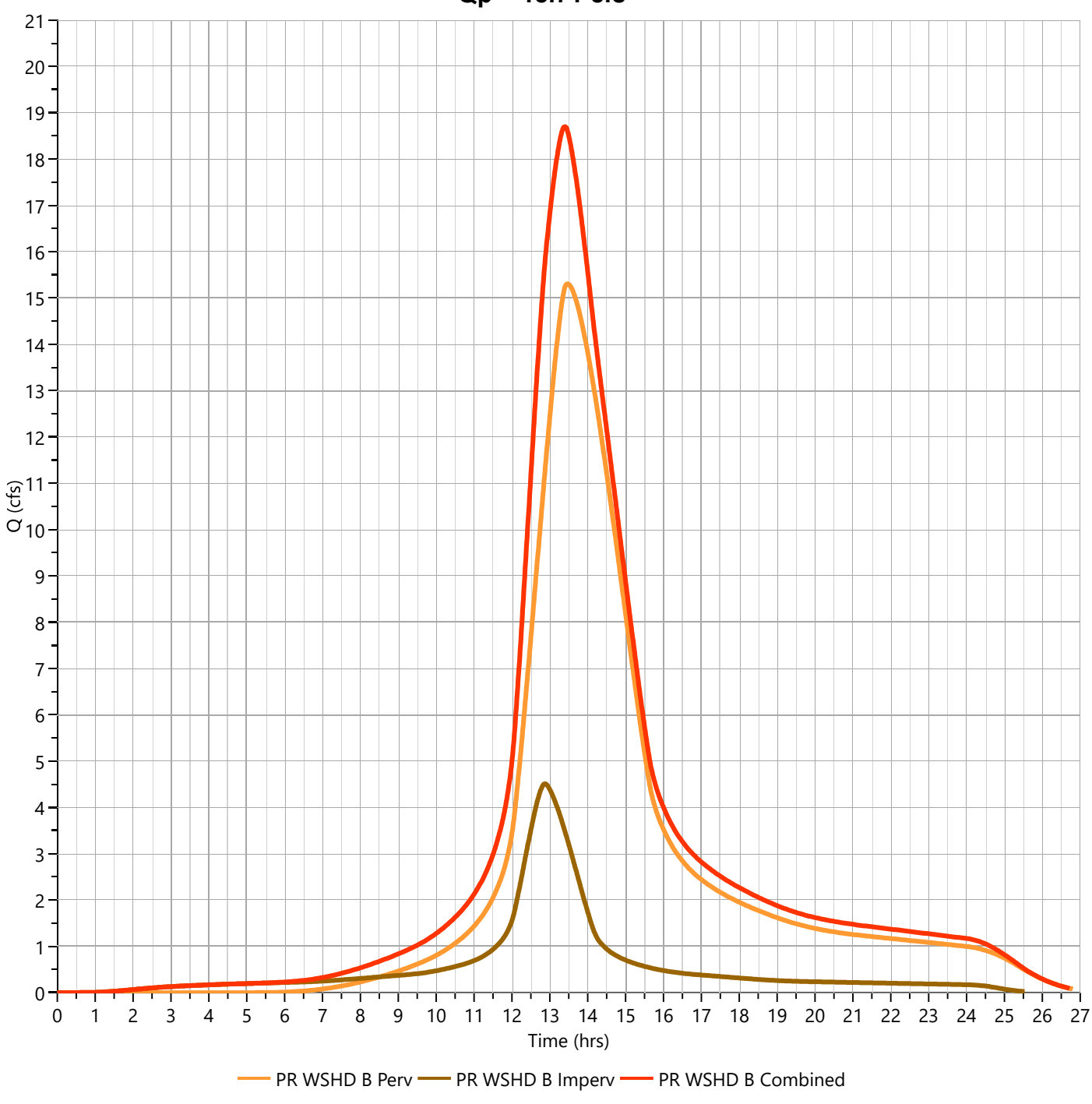
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD B Combined

Hyd. No. 27

Hydrograph Type	= Junction	Peak Flow	= 18.71 cfs
Storm Frequency	= 100-yr	Time to Peak	= 13.40 hrs
Time Interval	= 1 min	Hydrograph Volume	= 257,894 cuft
Inflow Hydrographs	= 25, 26	Total Contrib. Area	= 11.1 ac



Hydrograph Report

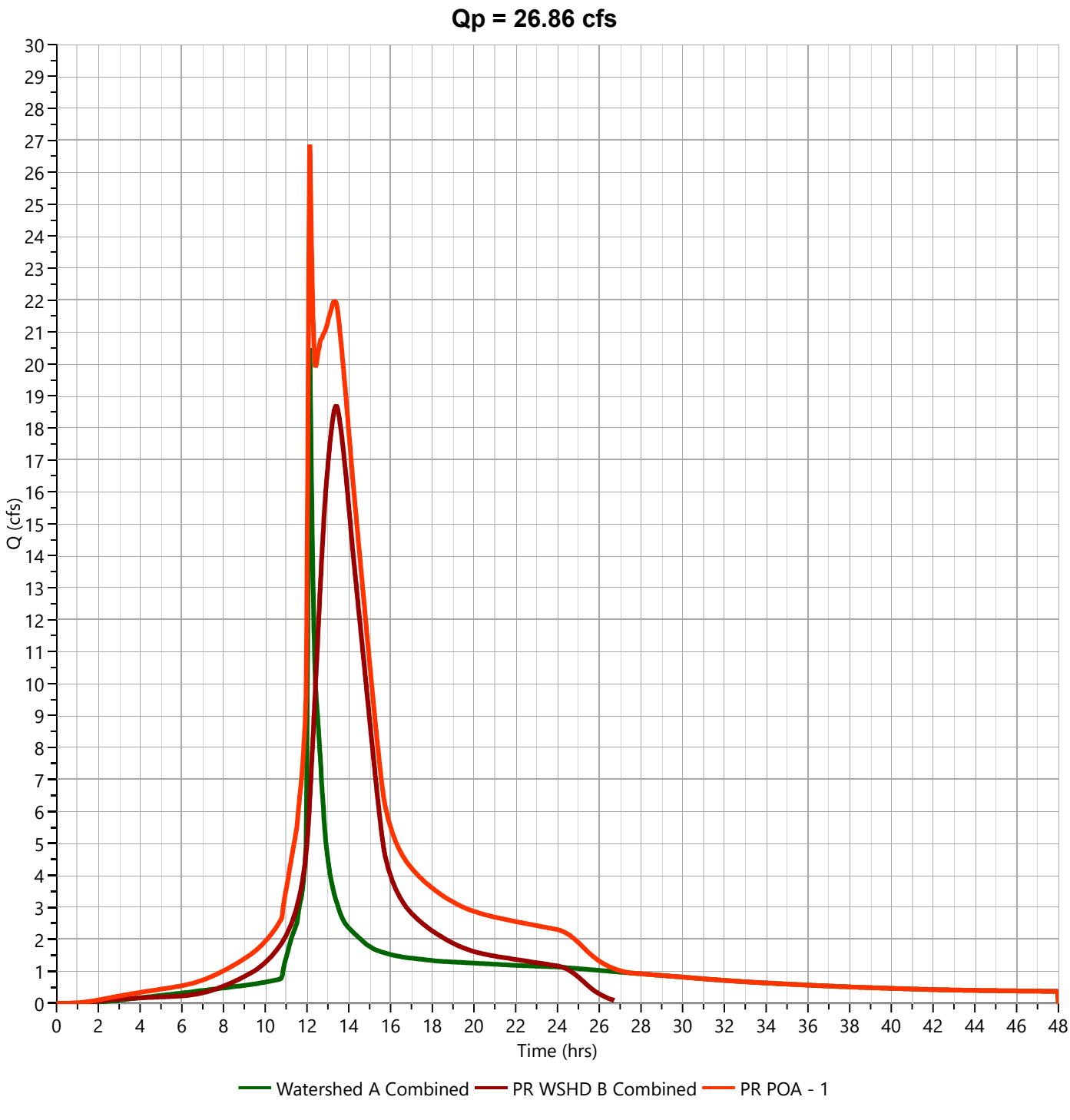
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR POA - 1

Hyd. No. 29

Hydrograph Type	= Junction	Peak Flow	= 26.86 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 1 min	Hydrograph Volume	= 432,829 cuft
Inflow Hydrographs		Total Contrib. Area	= 0.0 ac



Hydrograph Report

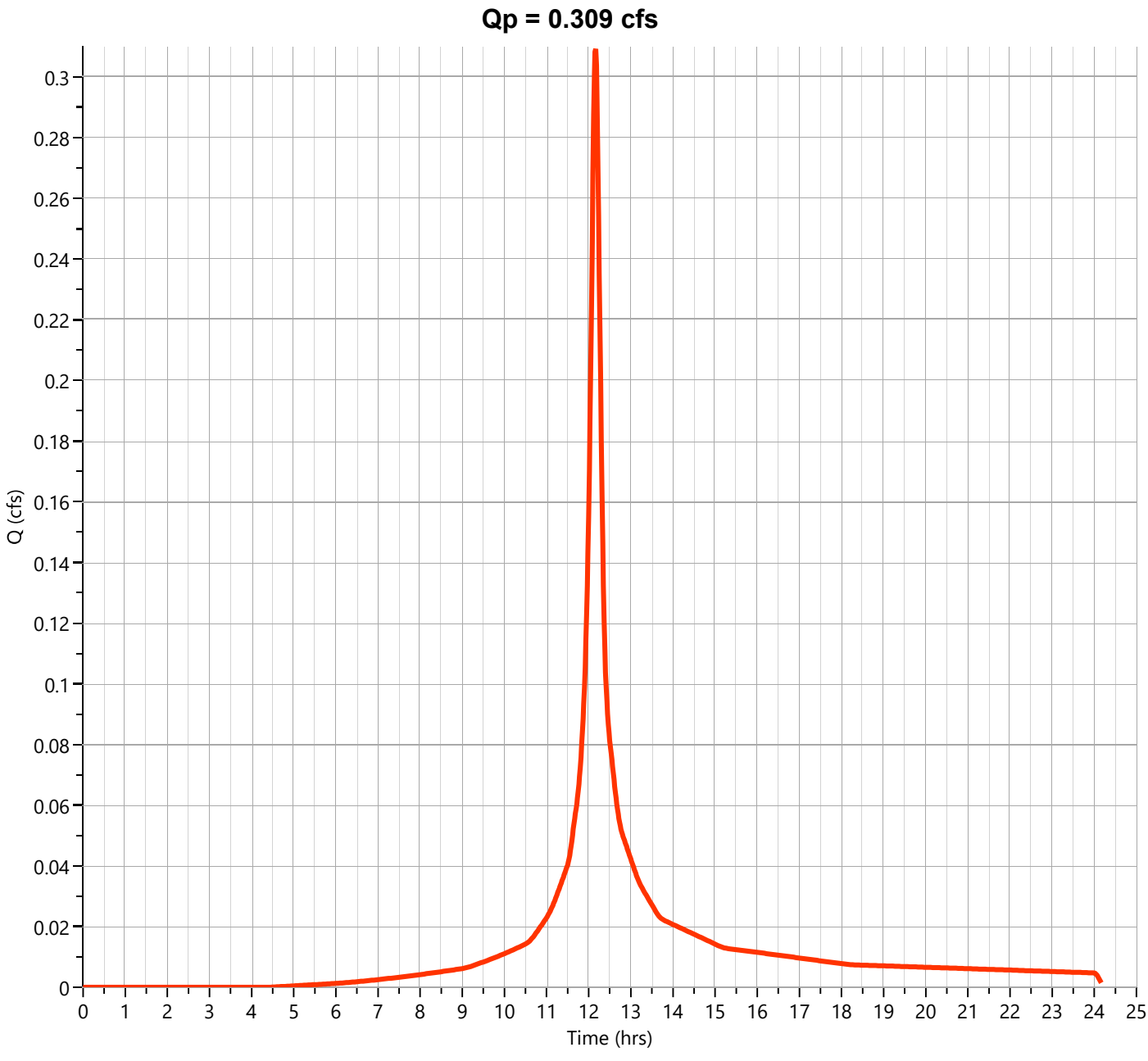
Hydrology Studio v 3.0.0.38

File: Proposed HydrologyCurrent Precipitation.hys
07-31-2025

PR WSHD C PerPOA 2

Hyd. No. 32

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.309 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.17 hrs
Time Interval	= 1 min	Runoff Volume	= 1,180 cuft
Drainage Area	= 0.05 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 11.0 min
Total Rainfall	= 8.81 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Basin Model

Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026



Hydrograph by Return Period

Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

03-19-2026

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Outflow (cfs)							
			1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
1	NRCS Runoff	PR WSDH A2 Imperv		4.482			6.978			12.90
2	Pond Route	Pervious Pavement A2		0.232			0.969			3.905
4	NRCS Runoff	PR WSHD A3 Imperv		9.527			14.84			27.43
5	Pond Route	Stormtrap A3		0.135			0.178			0.904
7	NRCS Runoff	PR WSHD A4 Imperv		8.590			13.38			24.74
8	Pond Route	Stormtrap A4		0.126			0.167			0.538
9	Junction	A3 &A4 outflow		0.261			0.345			1.352
11	NRCS Runoff	PR WSHD A5 Imperv		3.001			4.672			8.637
12	NRCS Runoff	PR WSHD A5 Perv		0.127			0.239			0.510
13	Junction	PR WSHD A5 Combined		3.127			4.911			9.147
14	Pond Route	Pervious Pavemnt A5		0.192			1.268			6.404
16	NRCS Runoff	PR WSHD A6 Imperv		1.598			2.488			4.599
17	NRCS Runoff	PR WSHD A6 Perv		0.374			0.716			1.543
18	Junction	PR WSHD A6 Combined		1.951			3.171			6.086
19	Pond Route	Pervious Pavement A6		0.162			0.219			4.747
21	NRCS Runoff	PR WSHD A7 Imperv		6.508			10.13			18.73
22	Pond Route	Stormtrap A7		3.861			8.908			12.83
24	Junction	Watershed A Combined		4.569			10.36			27.39
26	NRCS Runoff	PR WSHD B Perv		4.589			9.458			21.85
27	NRCS Runoff	PR WSHD B Imperv		2.046			3.191			5.904
28	Junction	PR WSHD B Combined		6.116			11.88			26.38
30	Junction	PR POA - 1		7.446			14.27			36.81
33	NRCS Runoff	PR WSHD C PerPOA-2		0.106			0.203			0.438

Hydrograph 2-yr Summary

Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

03-19-2026

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	NRCS Runoff	PR WSDH A2 Imperv	4.482	12.10	14,657	----		
2	Pond Route	Pervious Pavement A2	0.232	13.63	14,304	1	1.67	8,589
4	NRCS Runoff	PR WSHD A3 Imperv	9.527	12.10	31,812	----		
5	Pond Route	Stormtrap A3	0.135	20.90	16,425	4	2.43	25,615
7	NRCS Runoff	PR WSHD A4 Imperv	8.590	12.10	28,685	----		
8	Pond Route	Stormtrap A4	0.126	20.43	15,268	7	2.35	22,960
9	Junction	A3 &A4 outflow	0.261	20.67	31,693	5, 8		
11	NRCS Runoff	PR WSHD A5 Imperv	3.001	12.10	9,814	----		
12	NRCS Runoff	PR WSHD A5 Perv	0.127	12.10	345	----		
13	Junction	PR WSHD A5 Combined	3.127	12.10	10,158	11, 12		
14	Pond Route	Pervious Pavemnt A5	0.192	13.43	10,135	13	4.87	5,453
16	NRCS Runoff	PR WSHD A6 Imperv	1.598	12.10	5,226	----		
17	NRCS Runoff	PR WSHD A6 Perv	0.374	12.13	1,218	----		
18	Junction	PR WSHD A6 Combined	1.951	12.10	6,444	16, 17		
19	Pond Route	Pervious Pavement A6	0.162	13.15	6,428	18	3.64	3,117
21	NRCS Runoff	PR WSHD A7 Imperv	6.508	12.10	21,285	----		
22	Pond Route	Stormtrap A7	3.861	12.13	21,073	21	4.77	9,069
24	Junction	Watershed A Combined	4.569	12.13	83,632	2, 5, 8, 14, 19, 22		
26	NRCS Runoff	PR WSHD B Perv	4.589	13.50	62,045	----		
27	NRCS Runoff	PR WSHD B Imperv	2.046	12.85	21,752	----		
28	Junction	PR WSHD B Combined	6.116	13.35	83,797	26, 27		
30	Junction	PR POA - 1	7.446	13.33	167,428	24, 28		
33	NRCS Runoff	PR WSHD C PerPOA-2	0.106	12.15	368	----		

Hydrograph Report

Hydrology Studio v 3.0.0.43

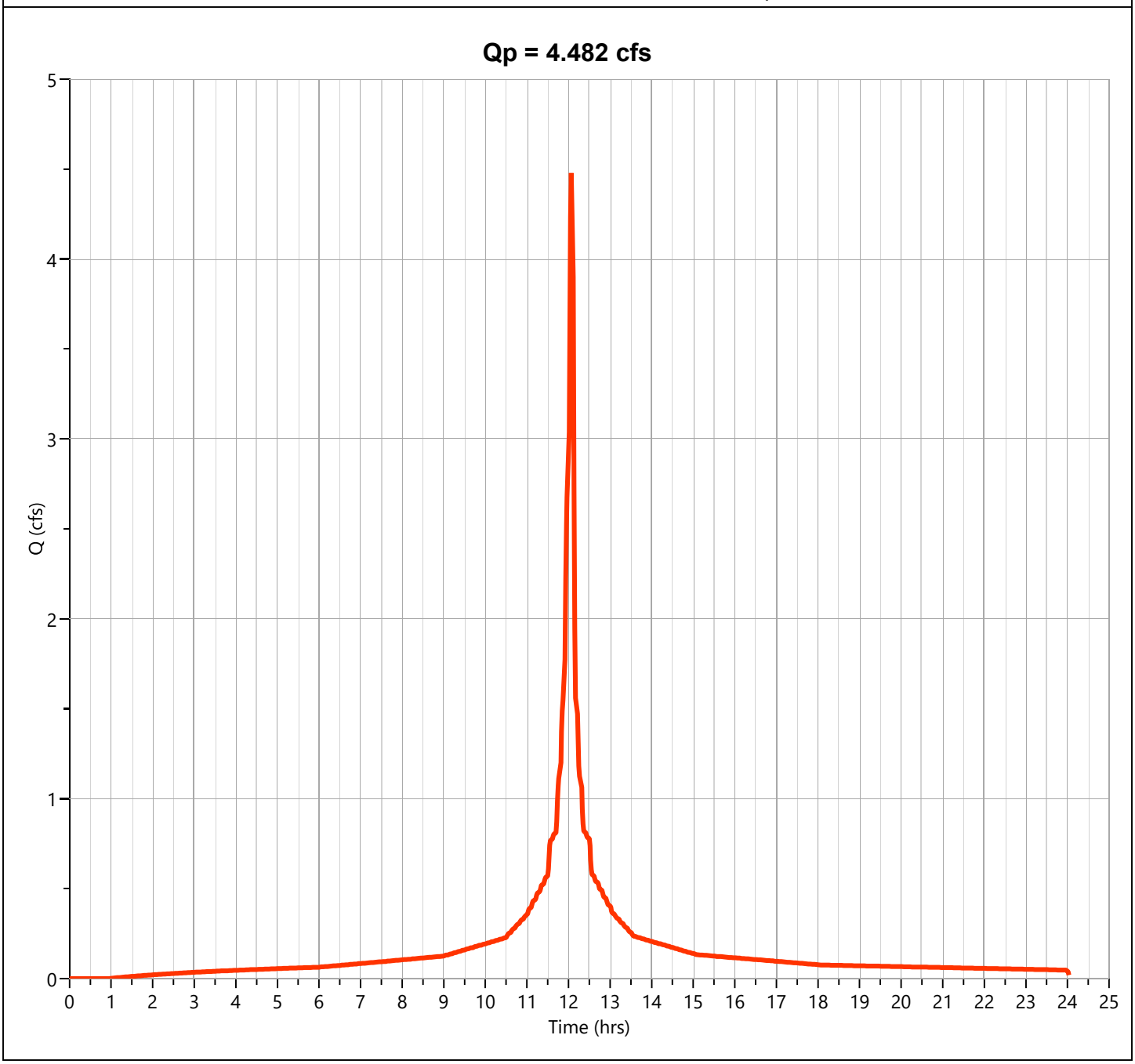
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

03-19-2026

PR WSDH A2 Imperv

Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 4.482 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 14,657 cuft
Drainage Area	= 1.15 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 3.0 min
Total Rainfall	= 3.98 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

03-19-2026

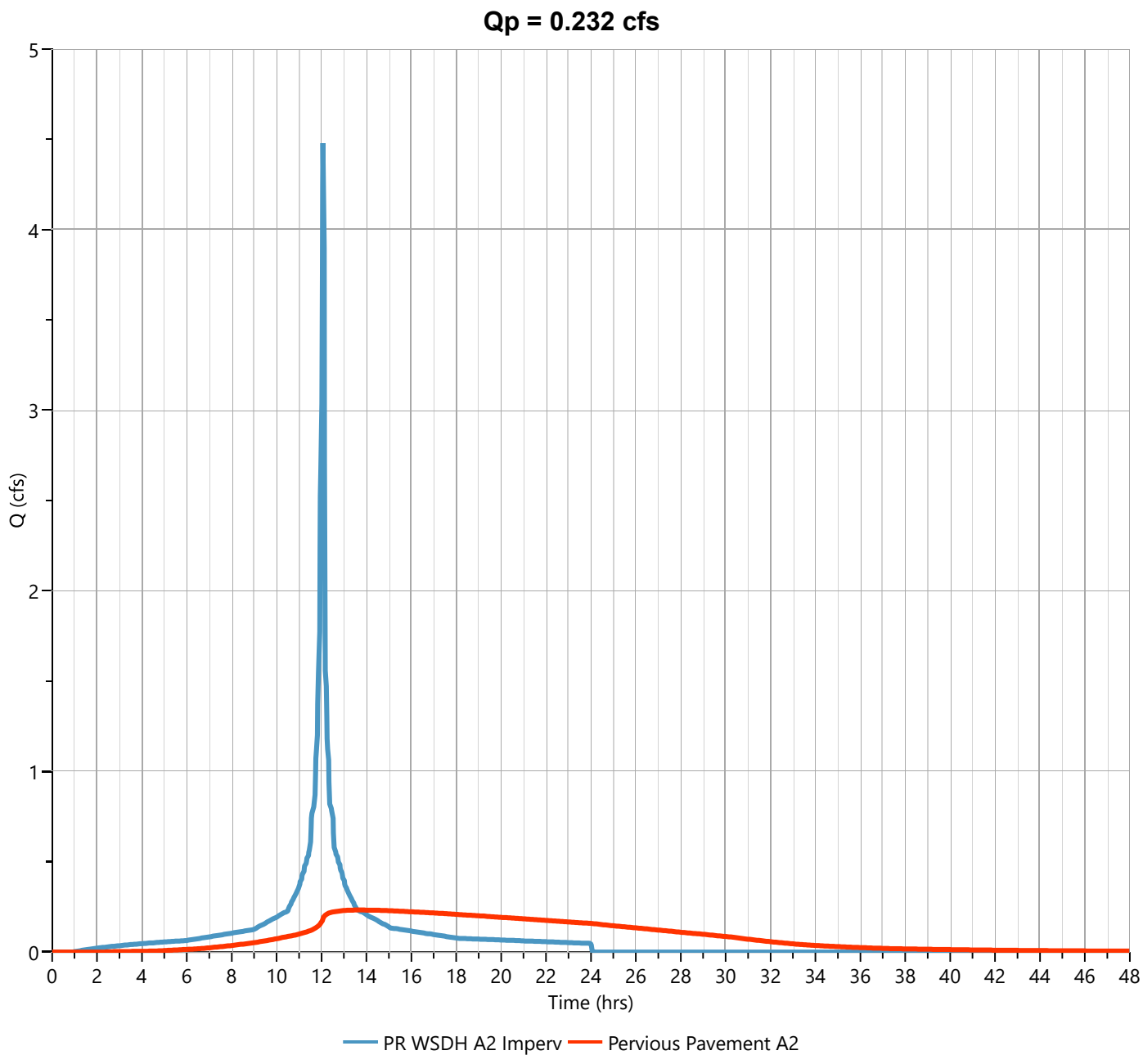
Pervious Pavement A2

Hyd. No. 2

Hydrograph Type	= Pond Route	Peak Flow	= 0.232 cfs
Storm Frequency	= 2-yr	Time to Peak	= 13.63 hrs
Time Interval	= 1 min	Hydrograph Volume	= 14,304 cuft
Inflow Hydrograph	= 1 - PR WSDH A2 Imperv	Max. Elevation	= 1.67 ft
Pond Name	= Pervious Pavement A2	Max. Storage	= 8,589 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 0.00 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 7.91 hrs



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

03-19-2026

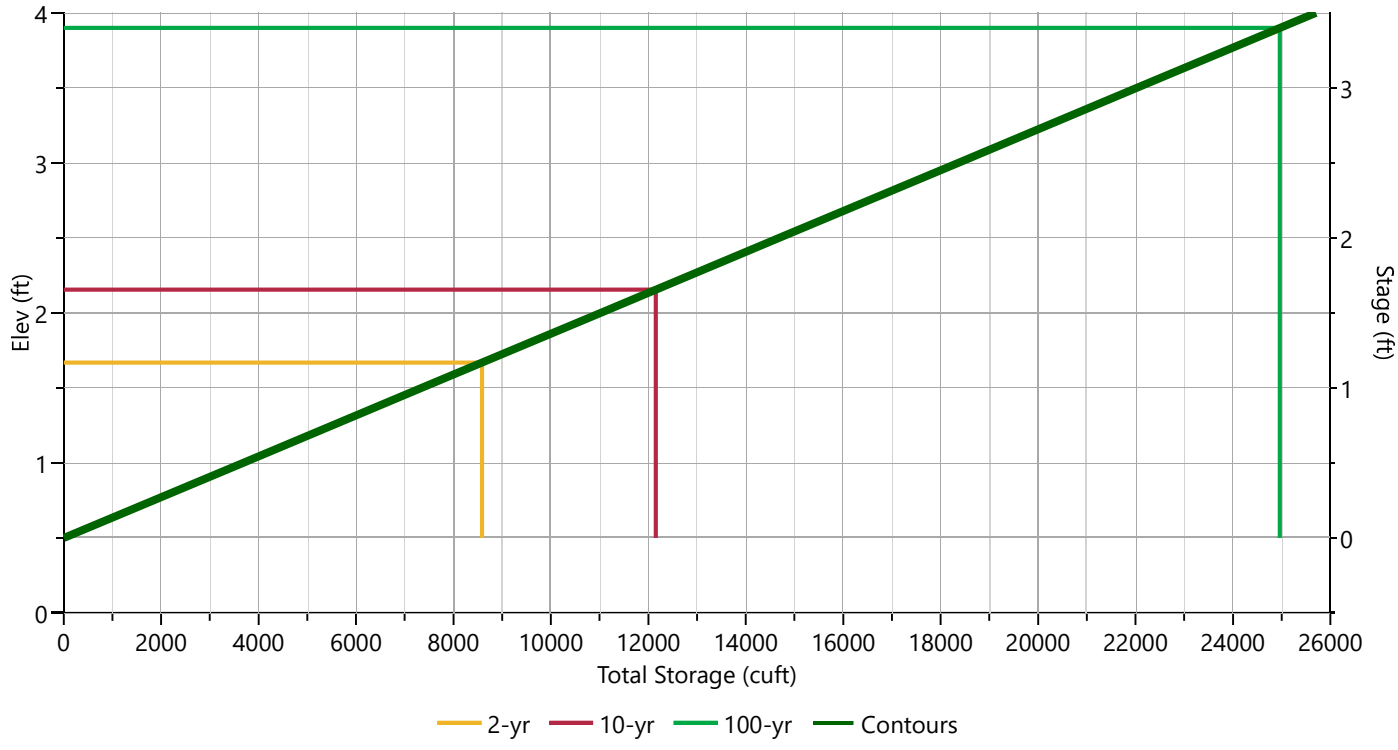
Pervious Pavement A2

Stage-Storage

User Defined Contours	
Description	**Input**
Bottom Elevation, ft	0.50
Voids (%)	40.00
Volume Calc	None

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)	Total Storage (cuft)
0.00	0.50	18,362	0.000	0.000
0.50	1.00	18,362	3,672	3,672
1.00	1.50	18,362	3,672	7,345
1.50	2.00	18,362	3,672	11,017
2.00	2.50	18,362	3,672	14,690
2.50	3.00	18,362	3,672	18,362
3.00	3.50	18,362	3,672	22,034
3.50	4.00	18,362	3,672	25,707



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

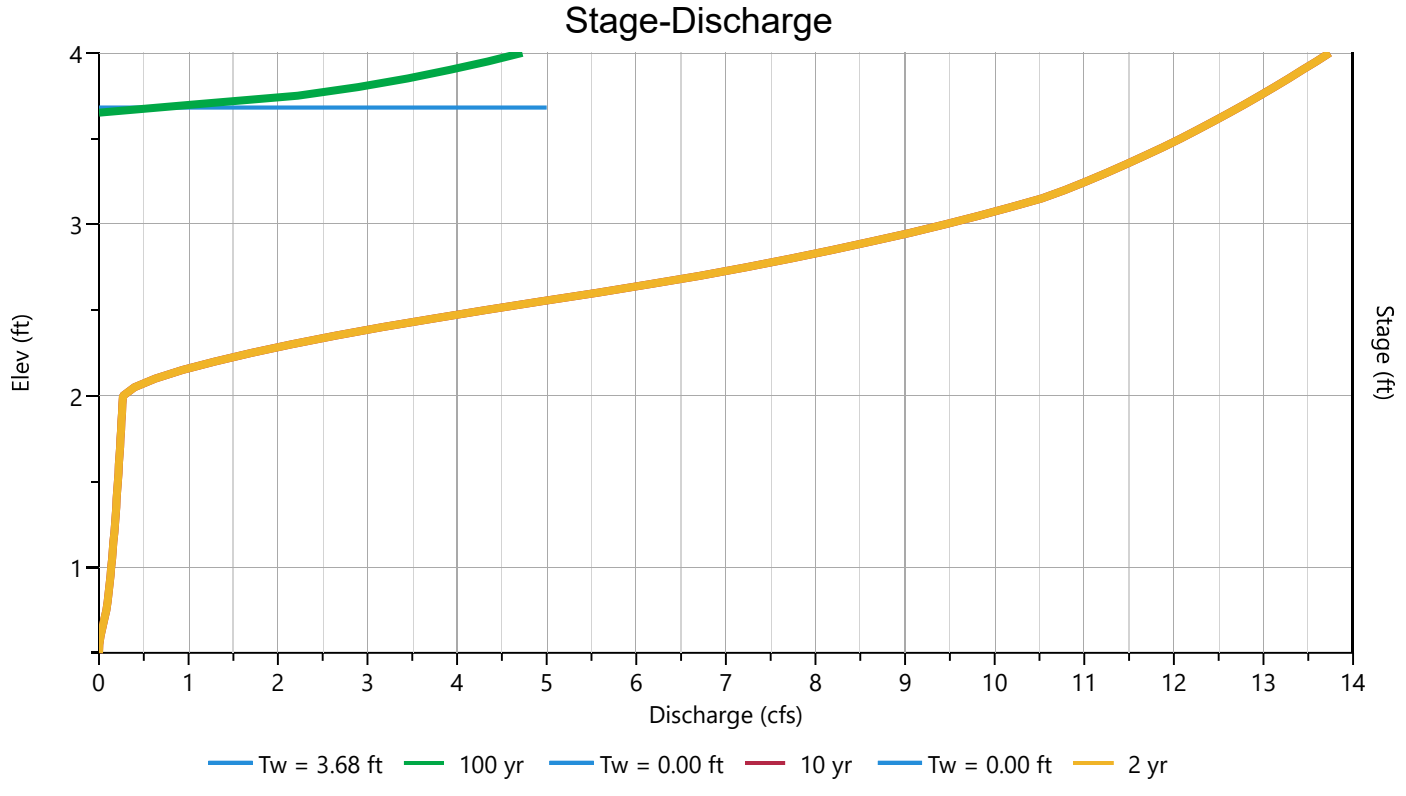
03-19-2026

Pervious Pavement A2

Stage-Discharge

Culvert / Orifices	Cir Culvert	Orifice			Perforated Riser	
		1 (m)	2	3		
Rise, in	18	3			Hole Diameter, in	
Span, in	18	3			No. holes	
No. Barrels	1	1			Invert Elevation, ft	
Invert Elevation, ft	0.50	0.50			Height, ft	
Orifice Coefficient, Co	0.60	0.60			Orifice Coefficient, Co	
Length, ft	30					
Barrel Slope, %	1					
N-Value, n	0.012					
Weirs	**Riser**	**Weir**			**Ancillary**	
		1 (m)	2	3		
Shape / Type		Rectangular			Exfiltration, in/hr	
Crest Elevation, ft		2				
Crest Length, ft		3.5				
Angle, deg						
Weir Coefficient, Cw		3.3				

m = Flows through Culvert, i = Independent



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

03-19-2026

Pervious Pavement A2

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total* (cfs)
				1	2	3		1	2	3				
0.00	0.50	0.000	0.000	0.000				0.000						0.000
0.50	1.00	3,672	0.138 ic	0.138				0.000						0.138
1.00	1.50	7,345	0.212 ic	0.212				0.000						0.212
1.50	2.00	11,017	0.267 ic	0.267				0.000						0.267
2.00	2.50	14,690	4.313 oc	0.229				4.084						4.313
2.50	3.00	18,362	9.448 oc	0.155				9.293 s						9.448
3.00	3.50	22,034	12.08 ic	0.115				11.96 s						12.08
3.50	4.00	25,707	13.74 ic	0.089				13.65 s						13.74

** No Tailwater. See Stage-Discharge plot for Tw effects. Suffix key: ic = inlet control, oc = outlet control, s = submerged weir*

Pond Report

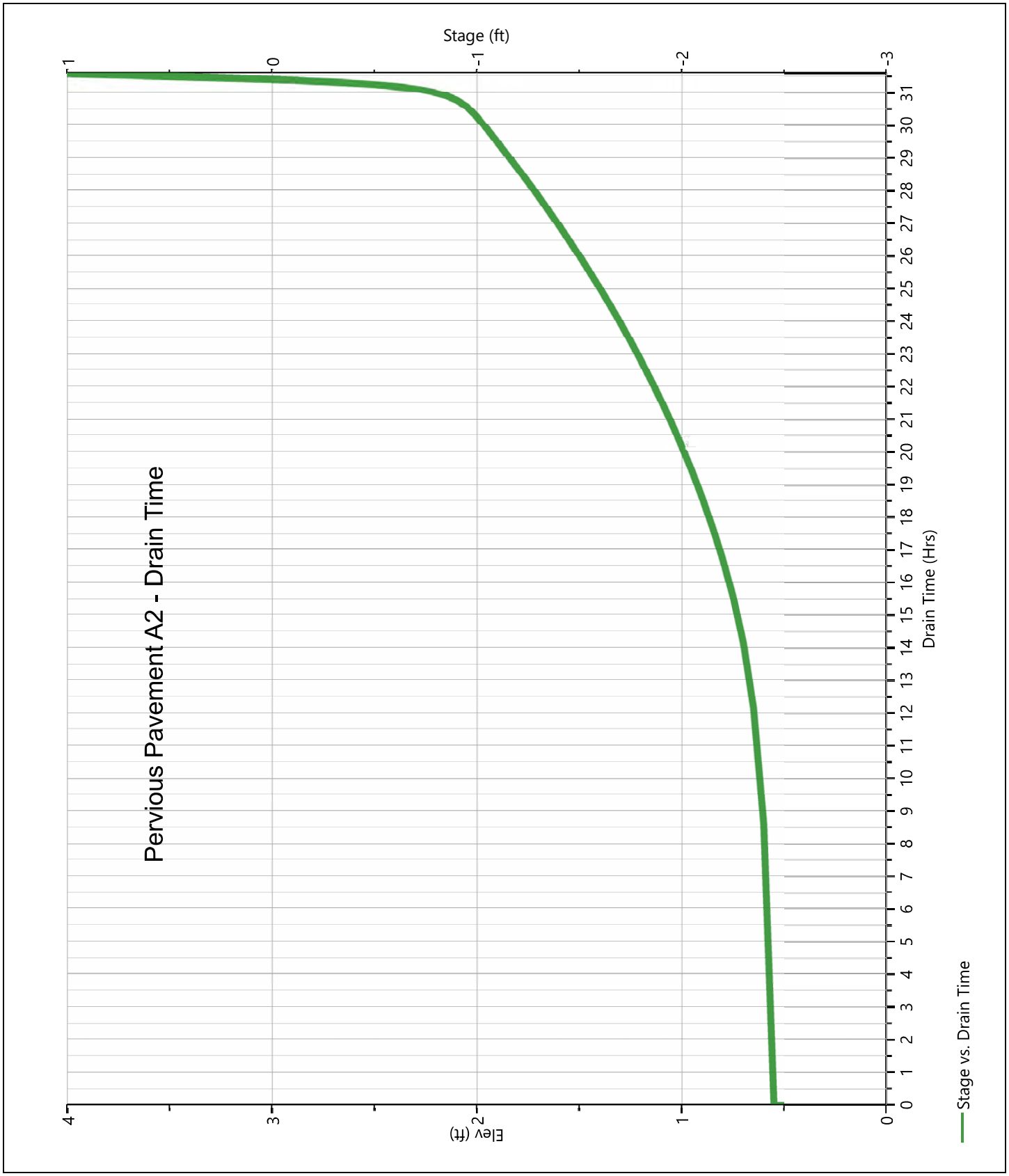
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

03-19-2026

Pervious Pavement A2

Pond Drawdown



Hydrograph Report

Hydrology Studio v 3.0.0.43

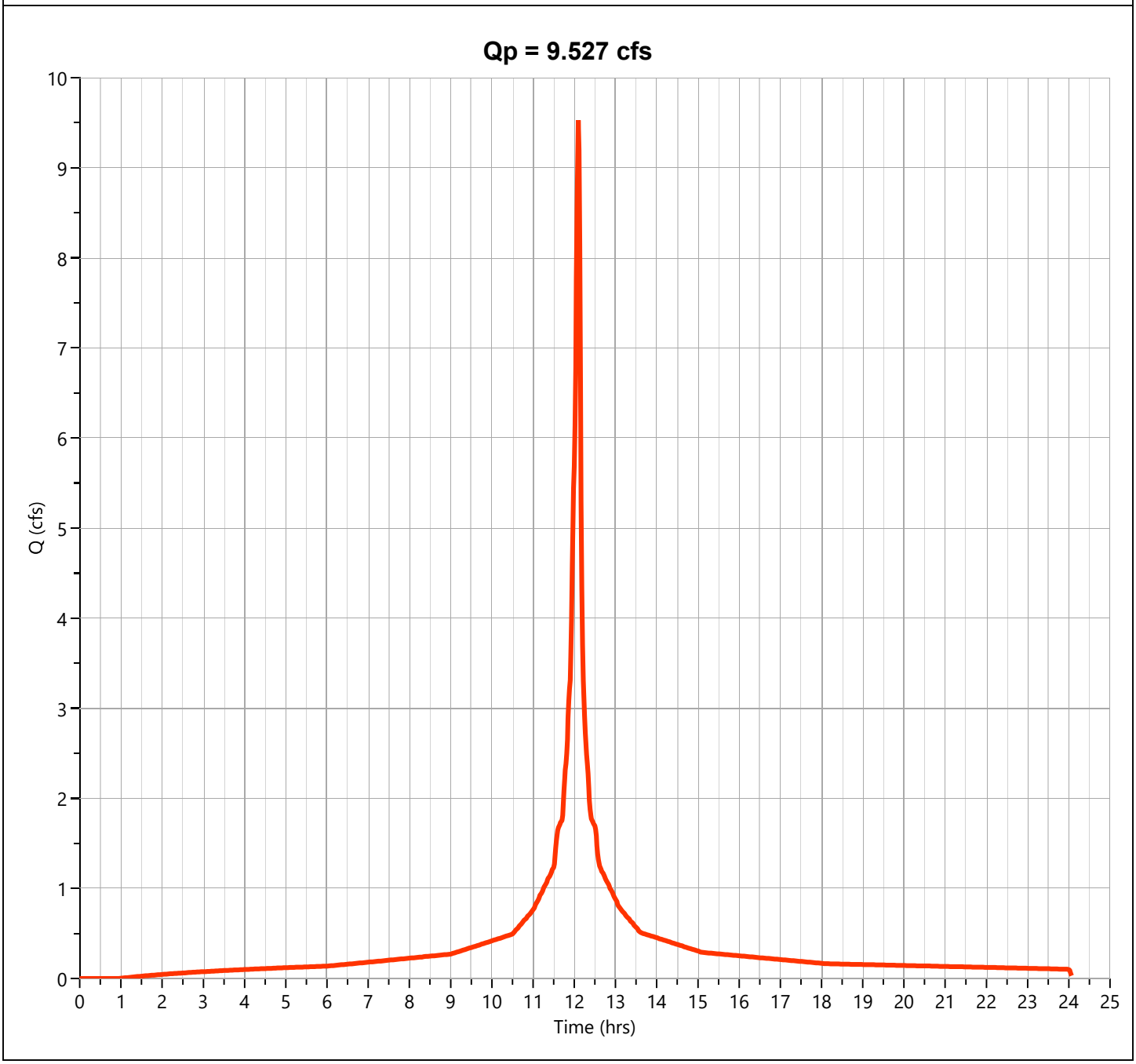
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

03-19-2026

PR WSHD A3 Imperv

Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 9.527 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 31,812 cuft
Drainage Area	= 2.34 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 4.0 min
Total Rainfall	= 3.98 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

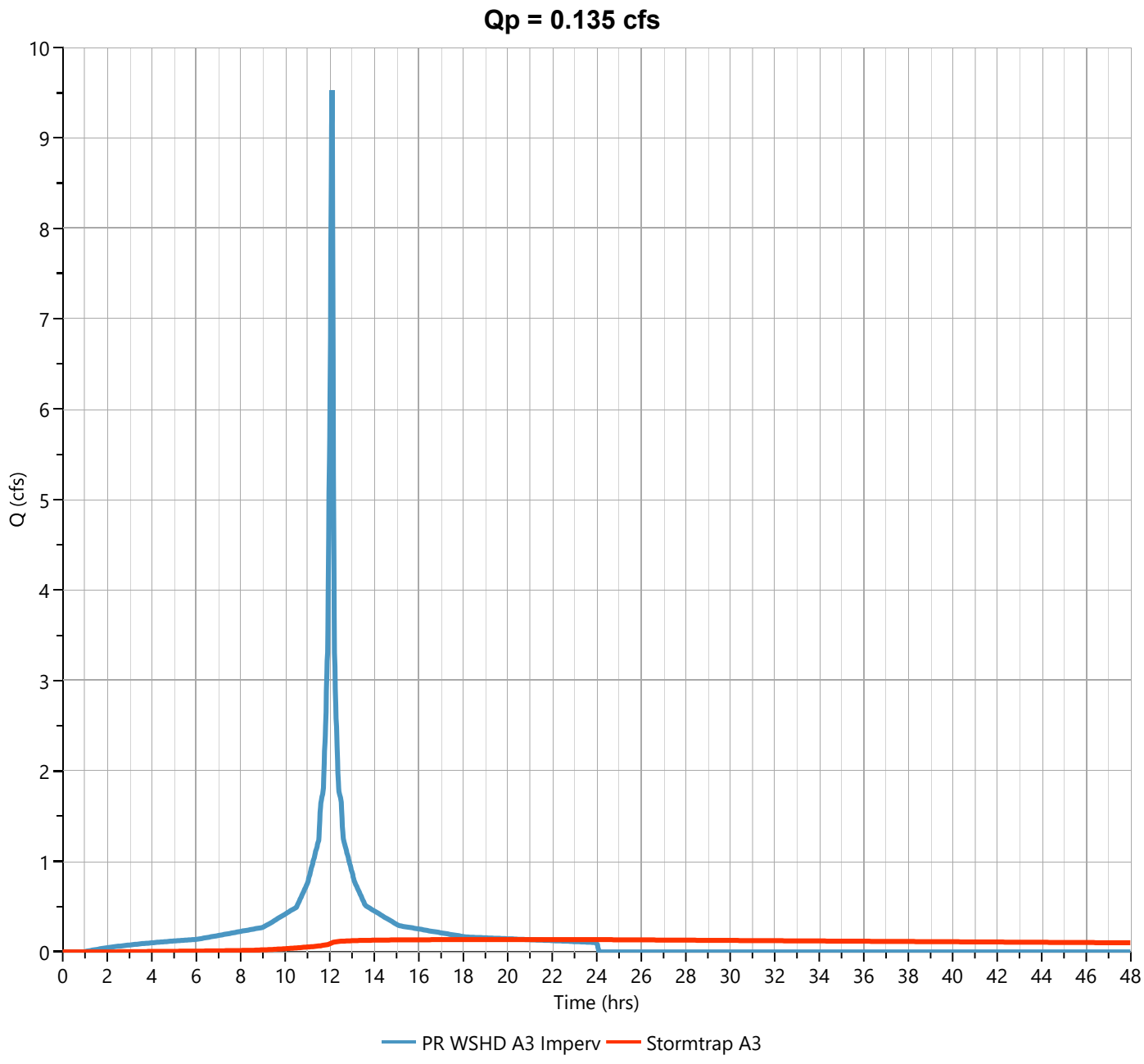
Stormtrap A3

Hyd. No. 5

Hydrograph Type	= Pond Route	Peak Flow	= 0.135 cfs
Storm Frequency	= 2-yr	Time to Peak	= 20.90 hrs
Time Interval	= 1 min	Hydrograph Volume	= 16,425 cuft
Inflow Hydrograph	= 4 - PR WSHD A3 Imperv	Max. Elevation	= 2.43 ft
Pond Name	= Stormtrap A3	Max. Storage	= 25,615 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 0.00 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 15.85 hrs



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

03-19-2026

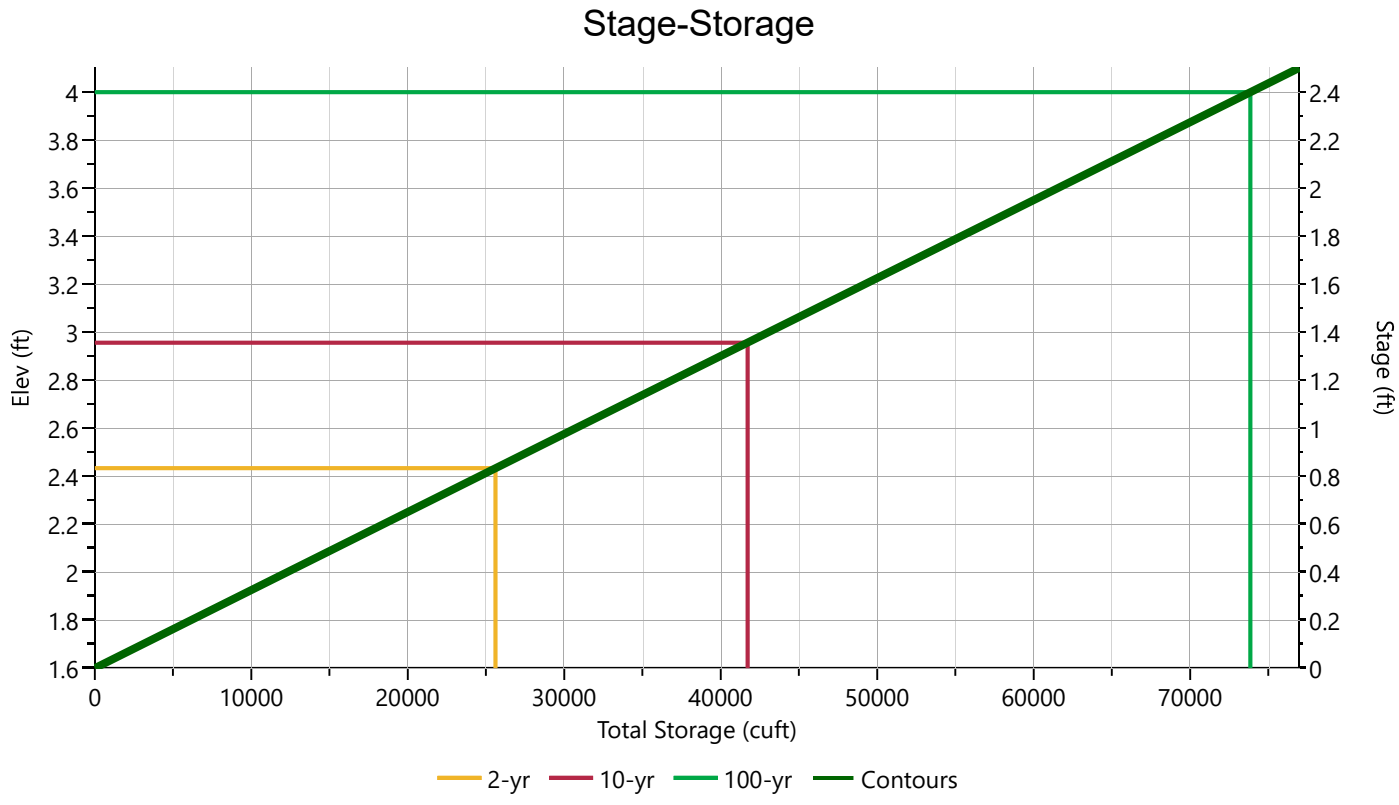
Stormtrap A3

Stage-Storage

User Defined Contours	
Description	**Input**
Bottom Elevation, ft	1.60
Voids (%)	100.00
Volume Calc	Conic

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)	Total Storage (cuft)
0.00	1.60	30,793	0.000	0.000
1.00	2.60	30,793	30,790	30,790
2.00	3.60	30,793	30,790	61,580
2.50	4.10	30,793	15,395	76,975



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

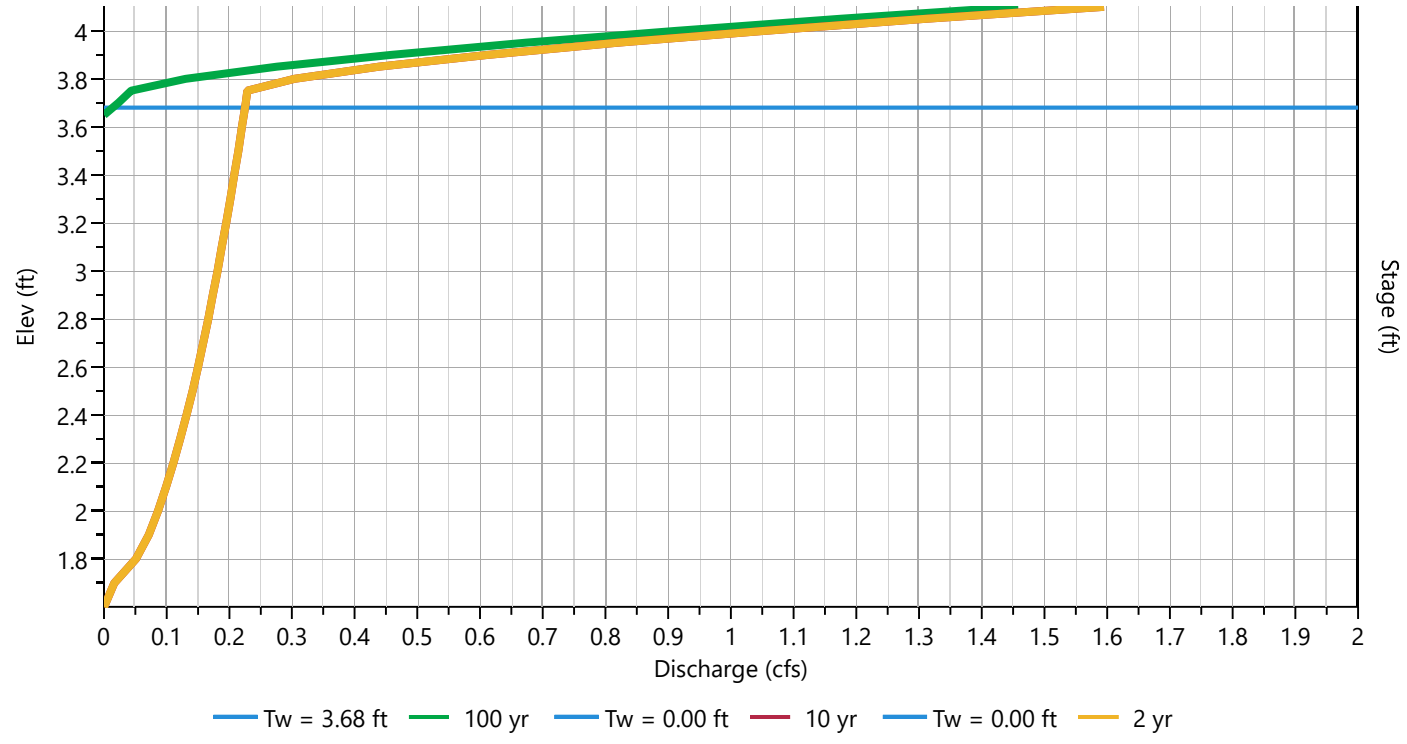
03-19-2026

Stormtrap A3

Stage-Discharge

Culvert / Orifices	Cir Culvert	Orifice			Perforated Riser	
		1 (m)	2	3		
Rise, in	18	2.5			Hole Diameter, in	
Span, in	18	2.5			No. holes	
No. Barrels	1	1			Invert Elevation, ft	
Invert Elevation, ft	1.60	1.60			Height, ft	
Orifice Coefficient, Co	0.60	0.60			Orifice Coefficient, Co	
Length, ft	410					
Barrel Slope, %	.3					
N-Value, n	0.012					
Weirs	**Riser**	**Weir**			**Ancillary**	
		1 (m)	2	3		
Shape / Type		Rectangular			Exfiltration, in/hr	
Crest Elevation, ft		3.75				
Crest Length, ft		2				
Angle, deg						
Weir Coefficient, Cw		3.3				

m = Flows through Culvert, i = Independent



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

03-19-2026

Stormtrap A3

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total* (cfs)
				1	2	3		1	2	3				
0.00	1.60	0.000	0.000	0.000				0.000						0.000
1.00	2.60	30,790	0.150 ic	0.150				0.000						0.150
2.00	3.60	61,580	0.220 ic	0.220				0.000						0.220
2.50	4.10	76,975	1.595 ic	0.228				1.367						1.595

** No Tailwater. See Stage-Discharge plot for Tw effects. Suffix key: ic = inlet control, oc = outlet control, s = submerged weir*

Pond Report

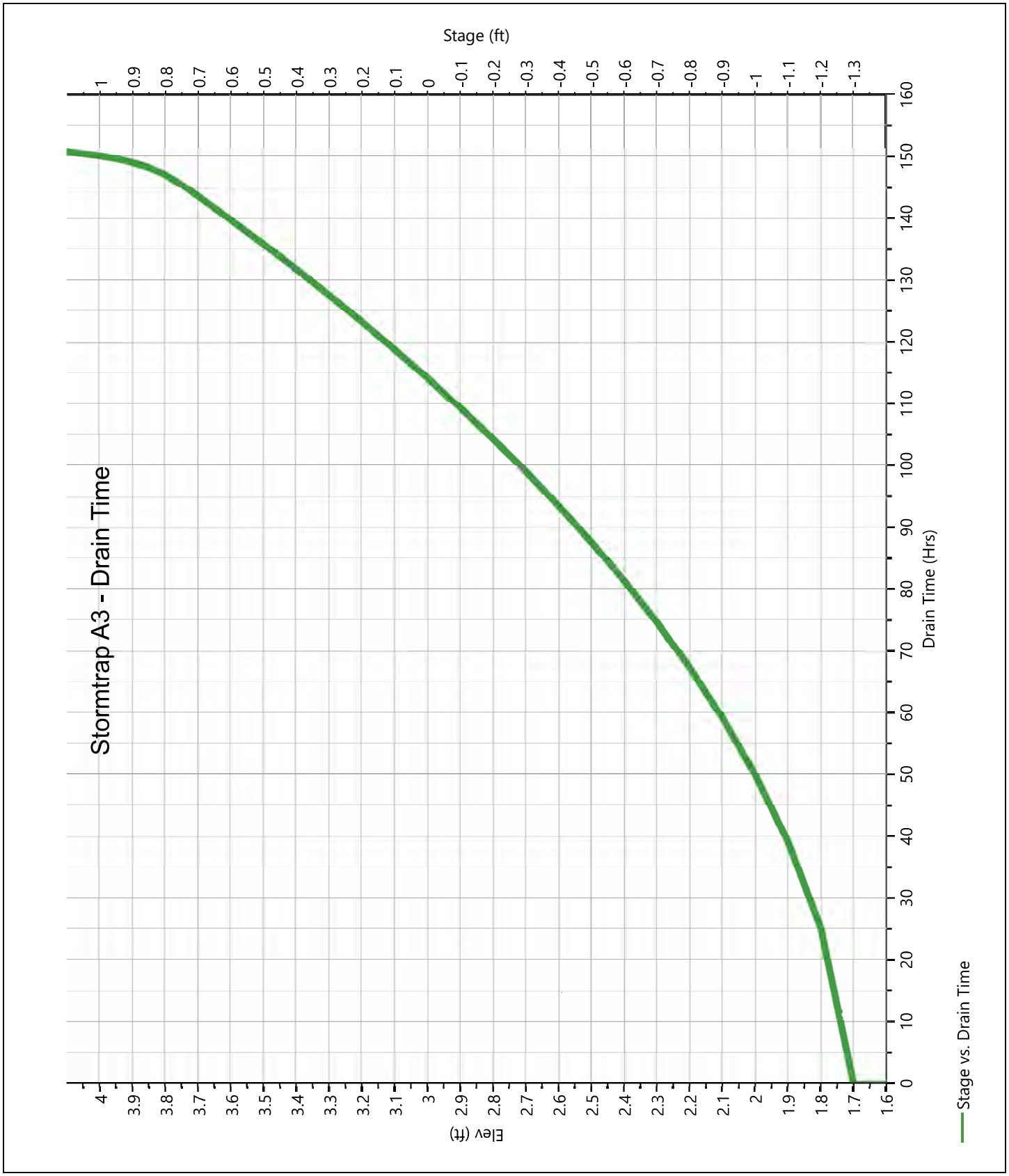
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Hydrology Studio v 3.0.0.43

03-19-2026

Stormtrap A3

Pond Drawdown



Hydrograph Report

Hydrology Studio v 3.0.0.43

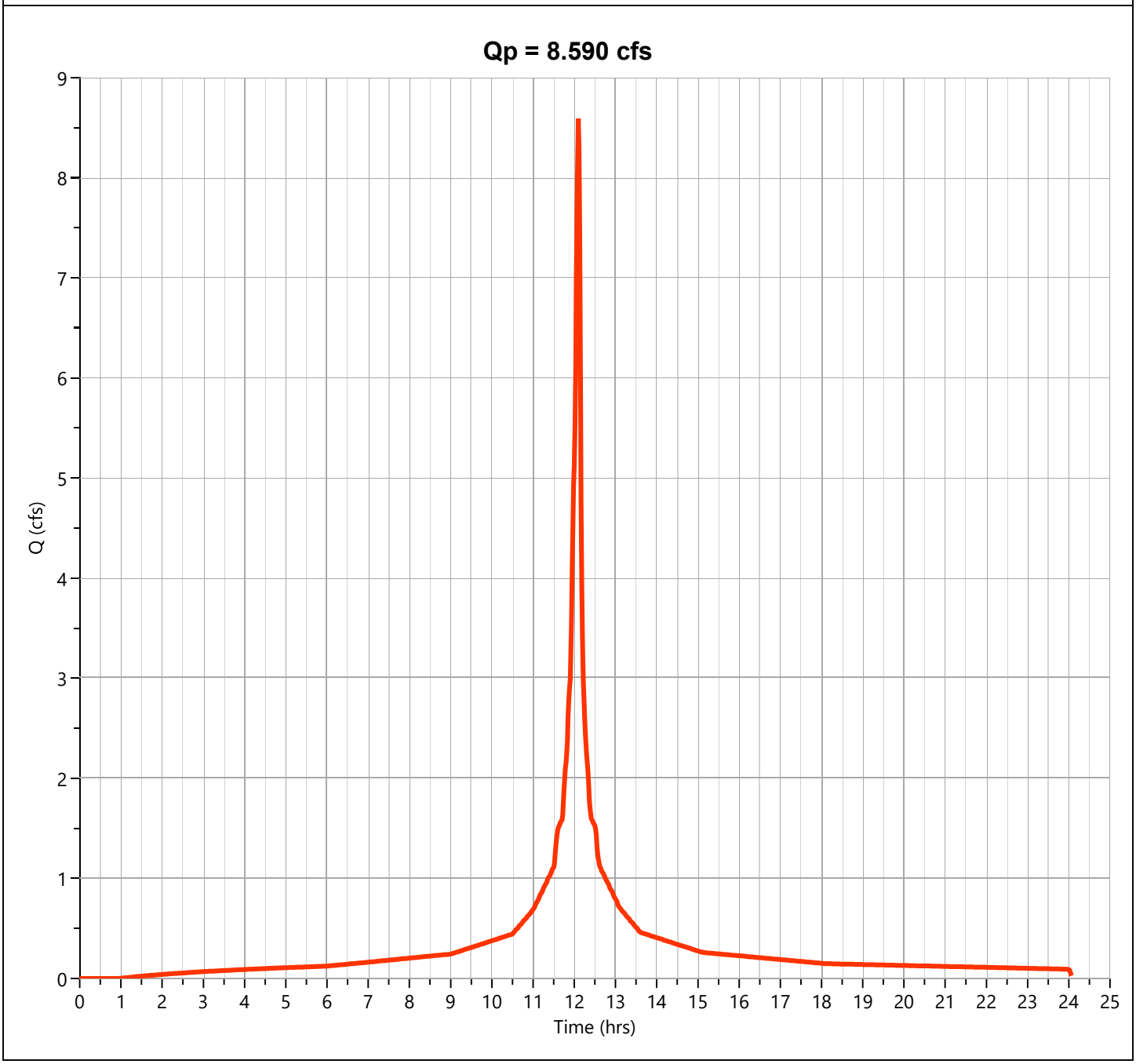
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

03-19-2026

PR WSHD A4 Imperv

Hyd. No. 7

Hydrograph Type	= NRCS Runoff	Peak Flow	= 8.590 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 28,685 cuft
Drainage Area	= 2.11 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 4.0 min
Total Rainfall	= 3.98 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

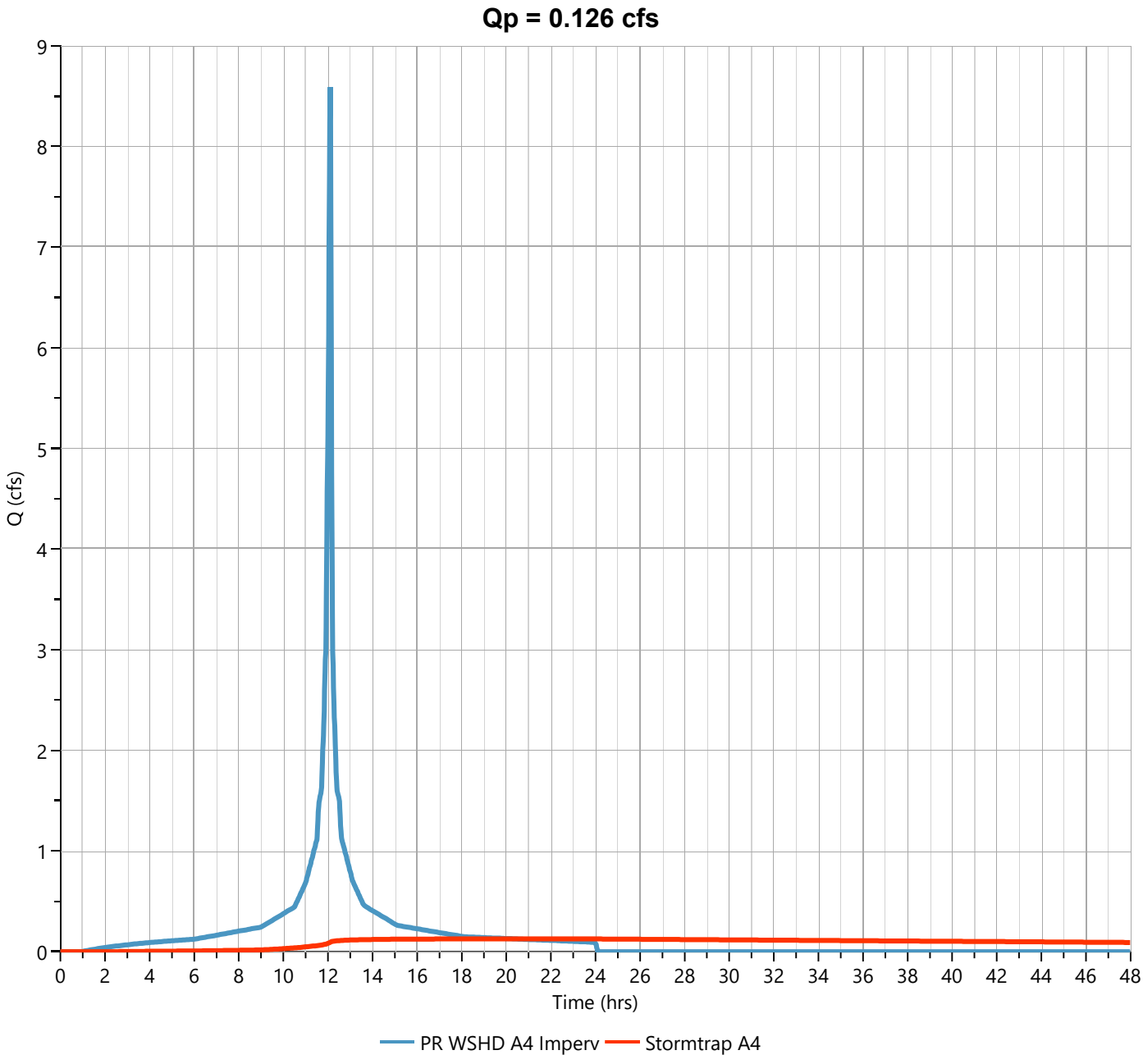
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

Stormtrap A4 — Hyd. No. 8

Hydrograph Type	= Pond Route	Peak Flow	= 0.126 cfs
Storm Frequency	= 2-yr	Time to Peak	= 20.43 hrs
Time Interval	= 1 min	Hydrograph Volume	= 15,268 cuft
Inflow Hydrograph	= 7 - PR WSHD A4 Imperv	Max. Elevation	= 2.35 ft
Pond Name	= Stormtrap A4	Max. Storage	= 22,960 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 0.00 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 15.83 hrs



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

03-19-2026

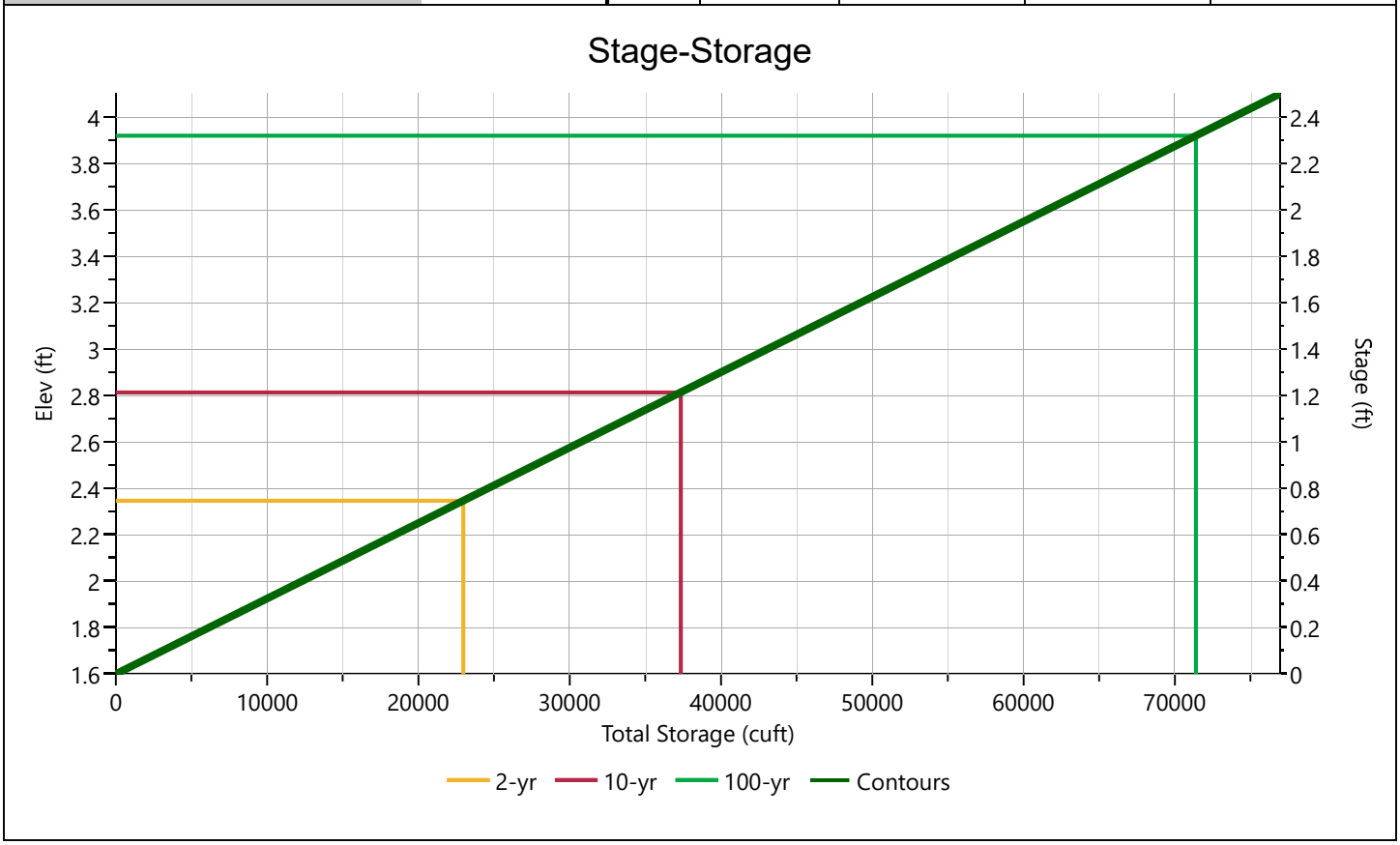
Stormtrap A4

Stage-Storage

User Defined Contours	
Description	**Input**
Bottom Elevation, ft	1.60
Voids (%)	100.00
Volume Calc	Ave End Area

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)	Total Storage (cuft)
0.00	1.60	30,793	0.000	0.000
1.00	2.60	30,793	30,793	30,793
2.00	3.60	30,793	30,793	61,586
2.50	4.10	30,793	15,397	76,983



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

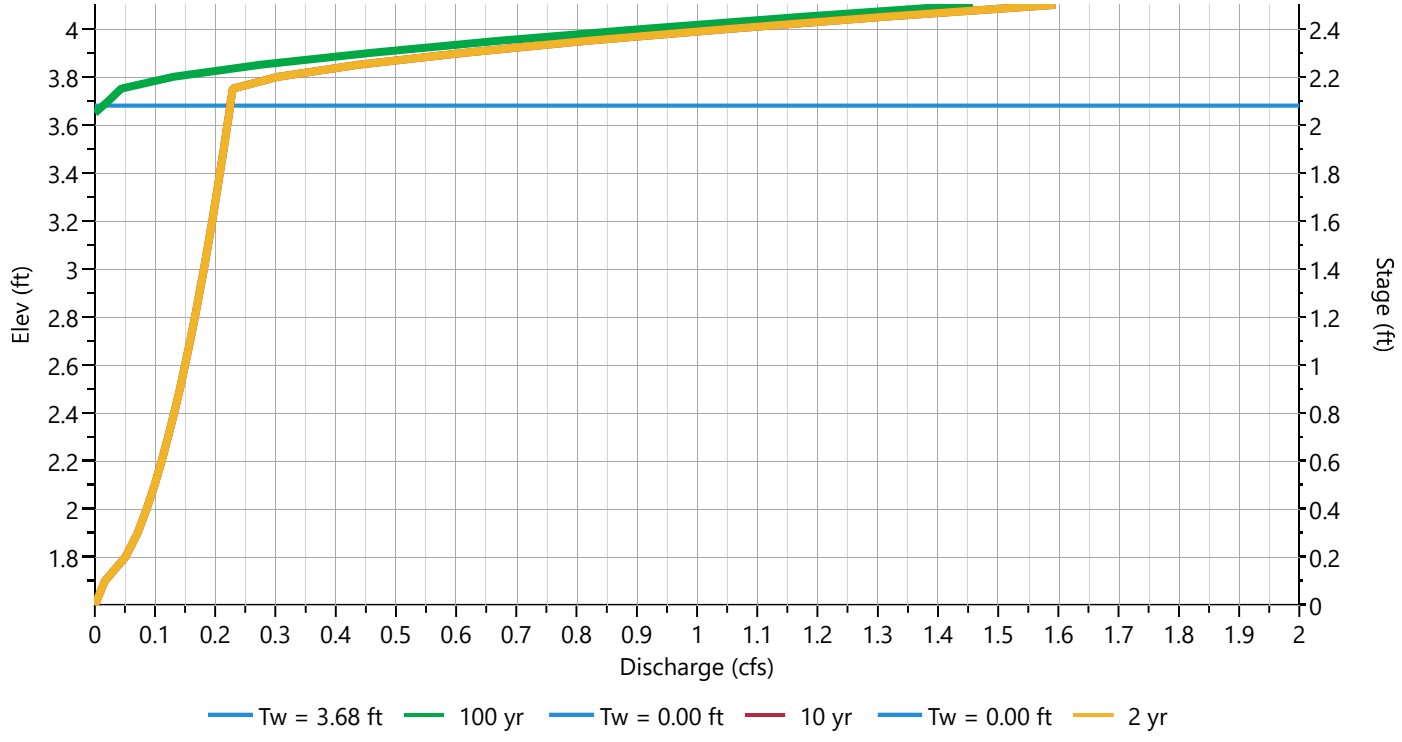
03-19-2026

Stormtrap A4

Stage-Discharge

Culvert / Orifices	Cir Culvert	Orifice			Perforated Riser	
		1 (m)	2	3		
Rise, in	18	2.5			Hole Diameter, in	
Span, in	18	2.5			No. holes	
No. Barrels	1	1			Invert Elevation, ft	
Invert Elevation, ft	1.60	1.60			Height, ft	
Orifice Coefficient, Co	0.60	0.60			Orifice Coefficient, Co	
Length, ft	410					
Barrel Slope, %	.3					
N-Value, n	0.012					
Weirs	**Riser**	**Weir**			**Ancillary**	
		1 (m)	2	3		
Shape / Type		Rectangular			Exfiltration, in/hr	
Crest Elevation, ft		3.75				
Crest Length, ft		2				
Angle, deg						
Weir Coefficient, Cw		3.3				

m = Flows through Culvert, i = Independent



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

03-19-2026

Stormtrap A4

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total* (cfs)
				1	2	3		1	2	3				
0.00	1.60	0.000	0.000	0.000				0.000						0.000
1.00	2.60	30,793	0.150 ic	0.150				0.000						0.150
2.00	3.60	61,586	0.220 ic	0.220				0.000						0.220
2.50	4.10	76,983	1.595 ic	0.228				1.367						1.595

** No Tailwater. See Stage-Discharge plot for Tw effects. Suffix key: ic = inlet control, oc = outlet control, s = submerged weir*

Pond Report

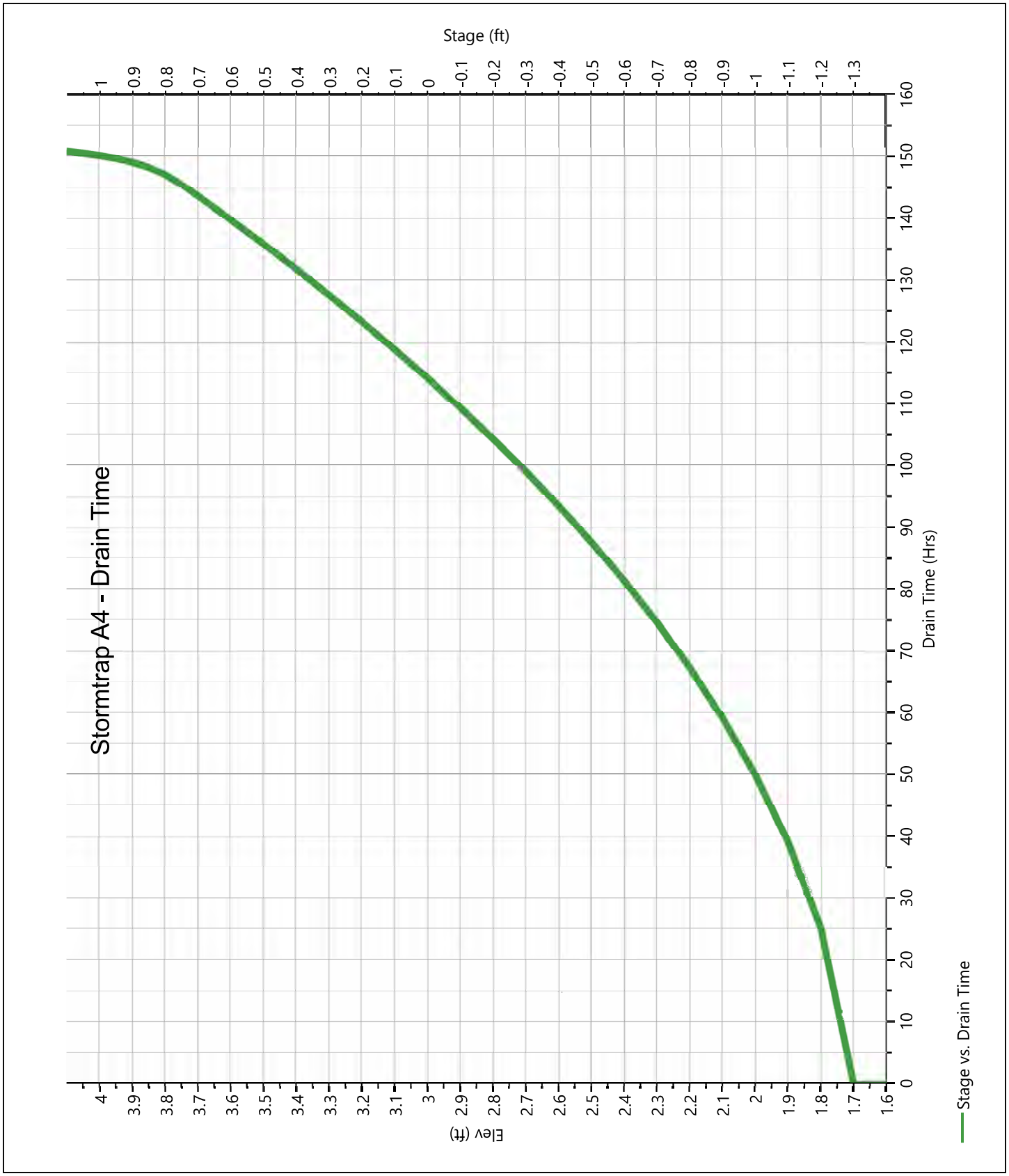
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Hydrology Studio v 3.0.0.43

03-19-2026

Stormtrap A4

Pond Drawdown



Hydrograph Report

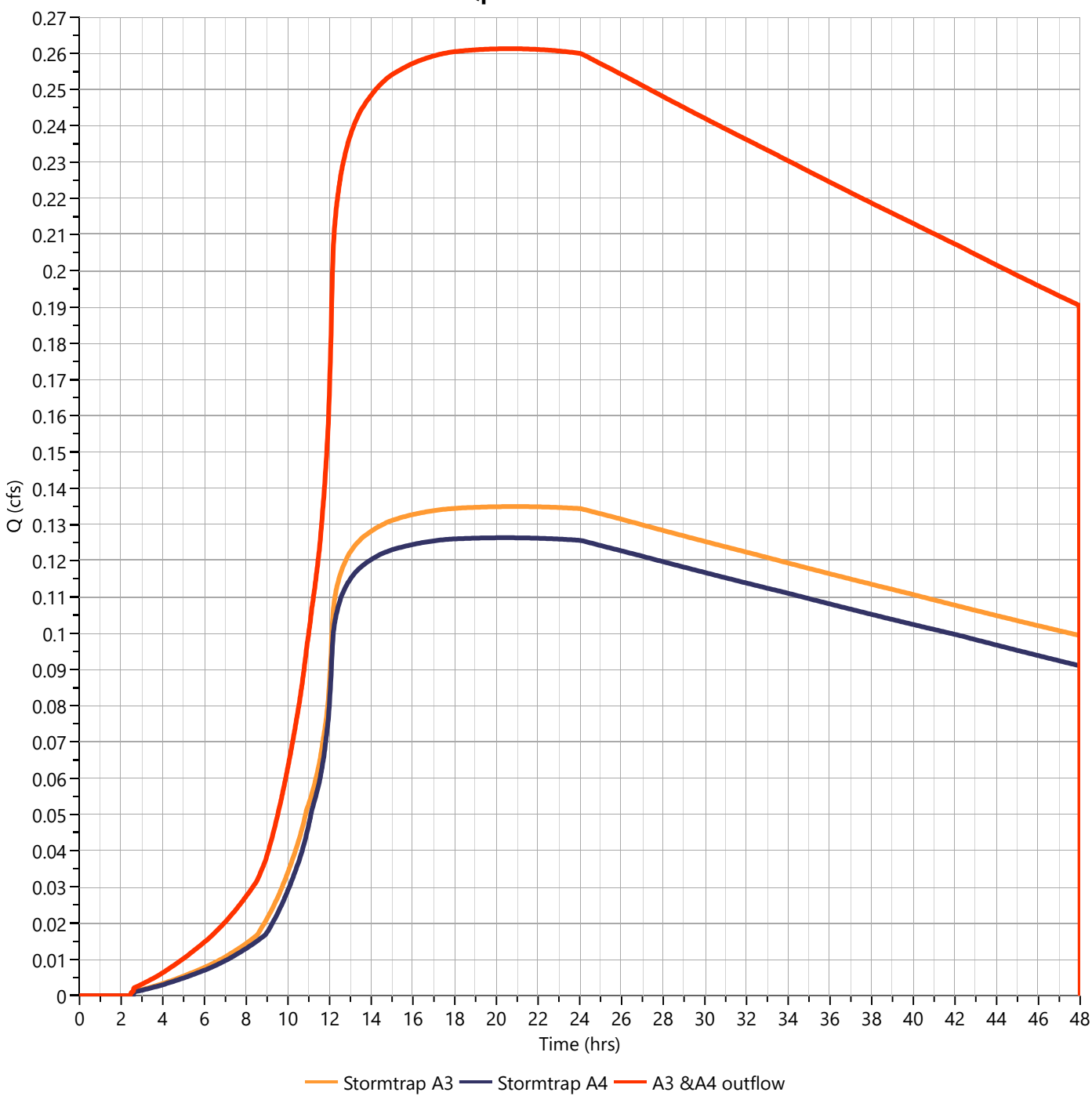
Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

A3 &A4 outflow

Hyd. No. 9

Hydrograph Type	= Junction	Peak Flow	= 0.261 cfs
Storm Frequency	= 2-yr	Time to Peak	= 20.67 hrs
Time Interval	= 1 min	Hydrograph Volume	= 31,693 cuft
Inflow Hydrographs		Total Contrib. Area	= 0.0 ac



Hydrograph Report

Hydrology Studio v 3.0.0.43

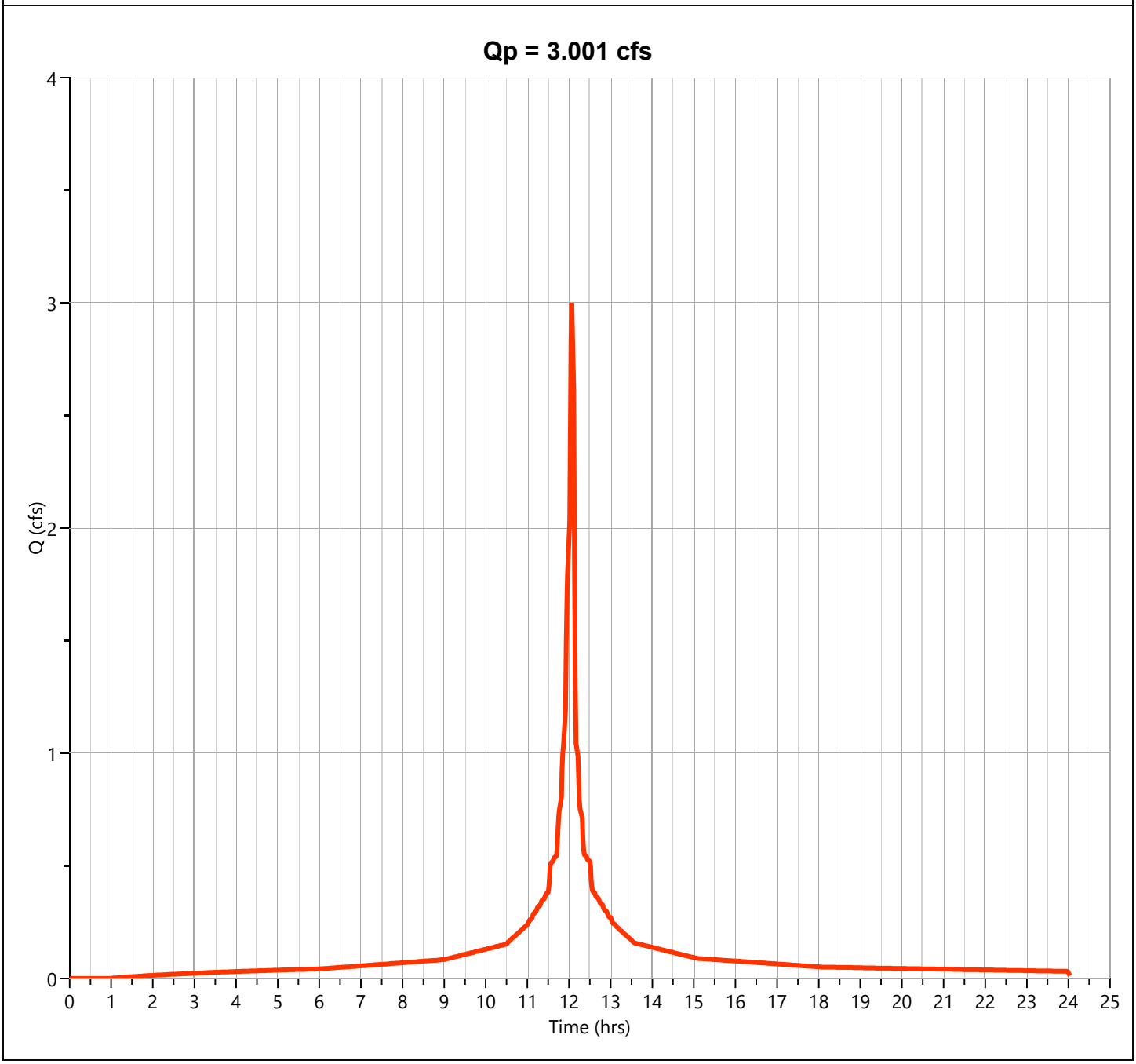
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03-19-2026

PR WSHD A5 Imperv

Hyd. No. 11

Hydrograph Type	= NRCS Runoff	Peak Flow	= 3.001 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 9,814 cuft
Drainage Area	= 0.77 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 3.98 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

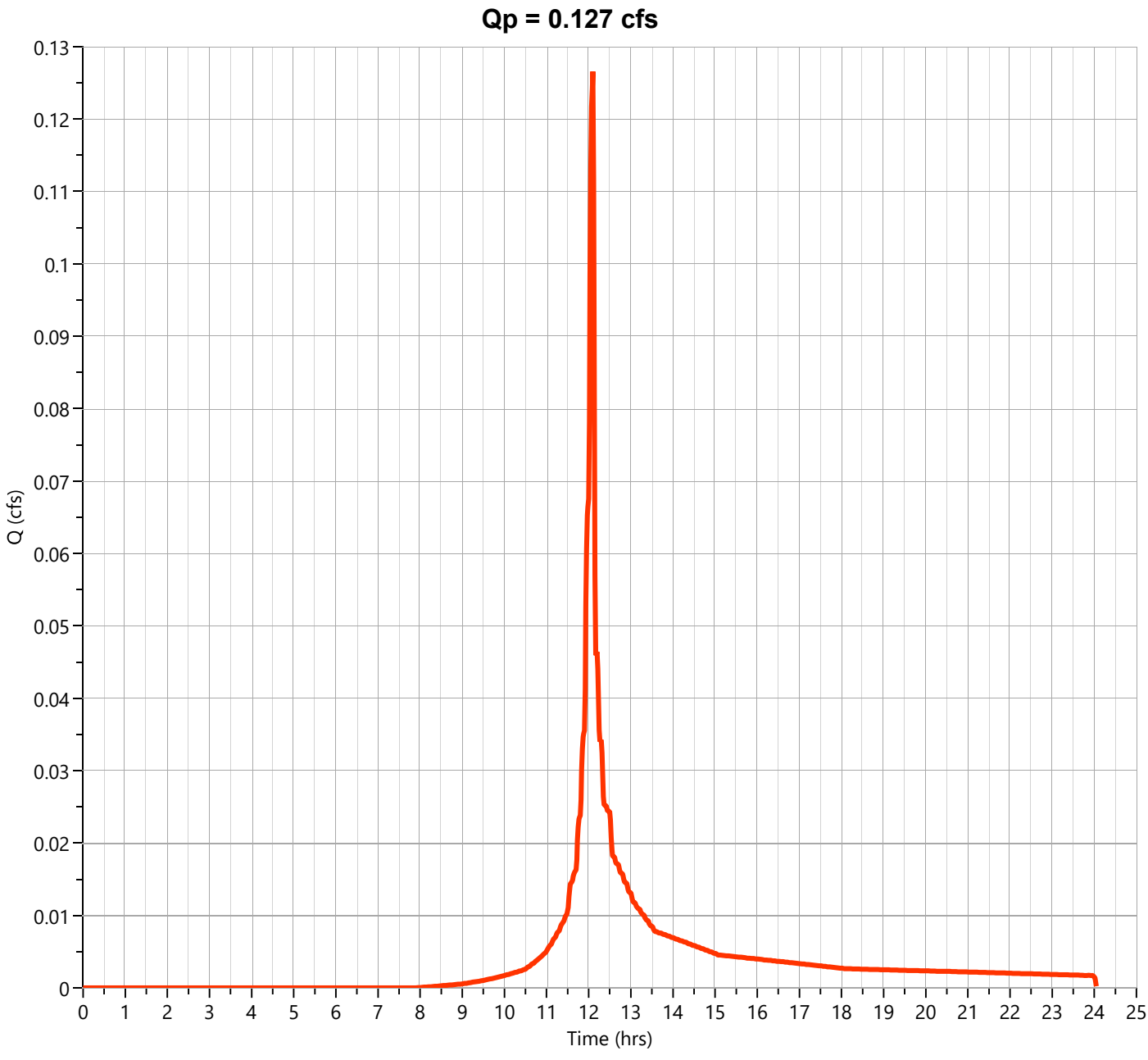
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03-19-2026

PR WSHD A5 Perv

Hyd. No. 12

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.127 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 345 cuft
Drainage Area	= 0.05 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 3.98 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

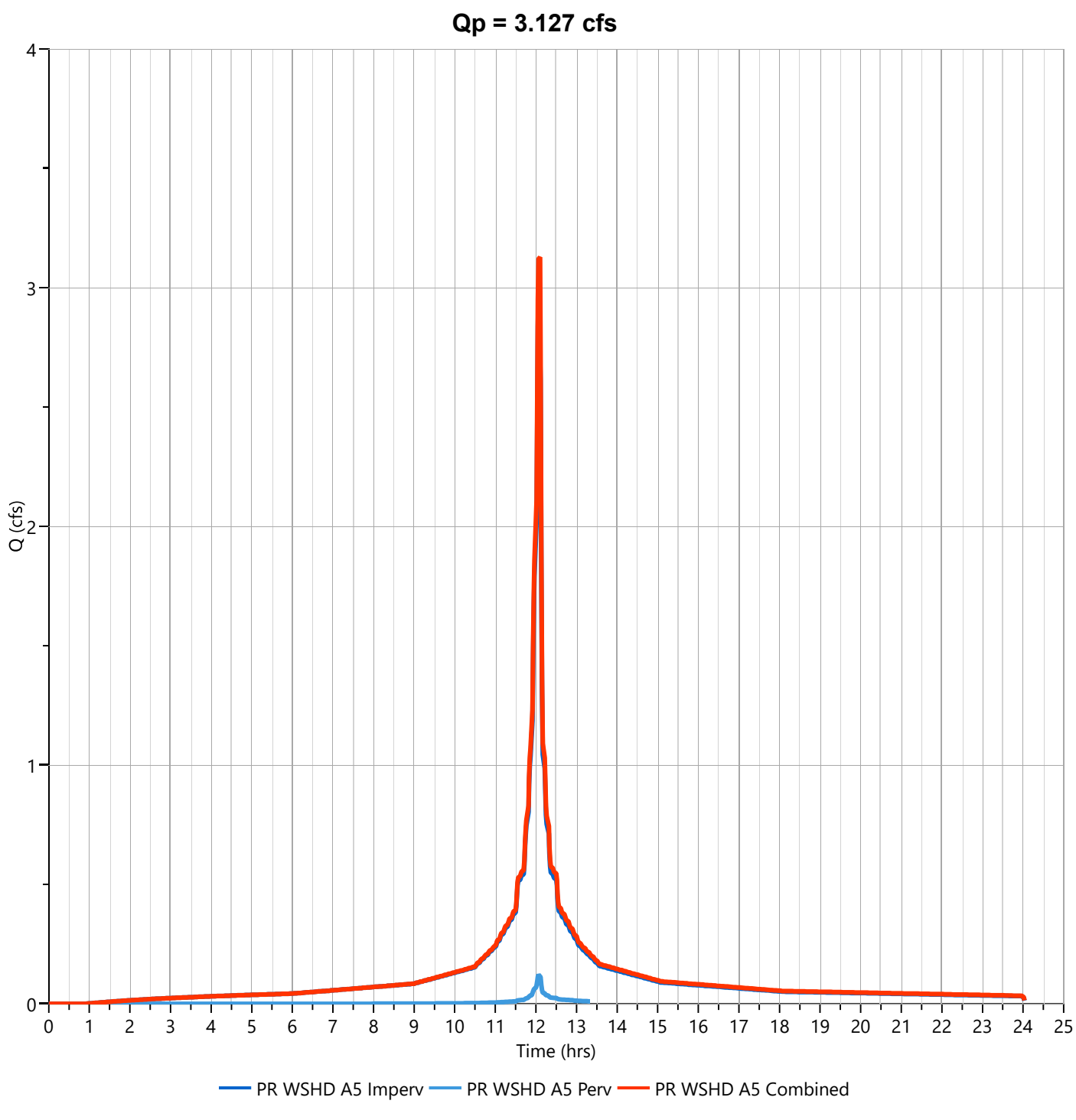
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03-19-2026

PR WSHD A5 Combined

Hyd. No. 13

Hydrograph Type	= Junction	Peak Flow	= 3.127 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Hydrograph Volume	= 10,158 cuft
Inflow Hydrographs	= 11, 12	Total Contrib. Area	= 0.82 ac



Hydrograph Report

Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

03-19-2026

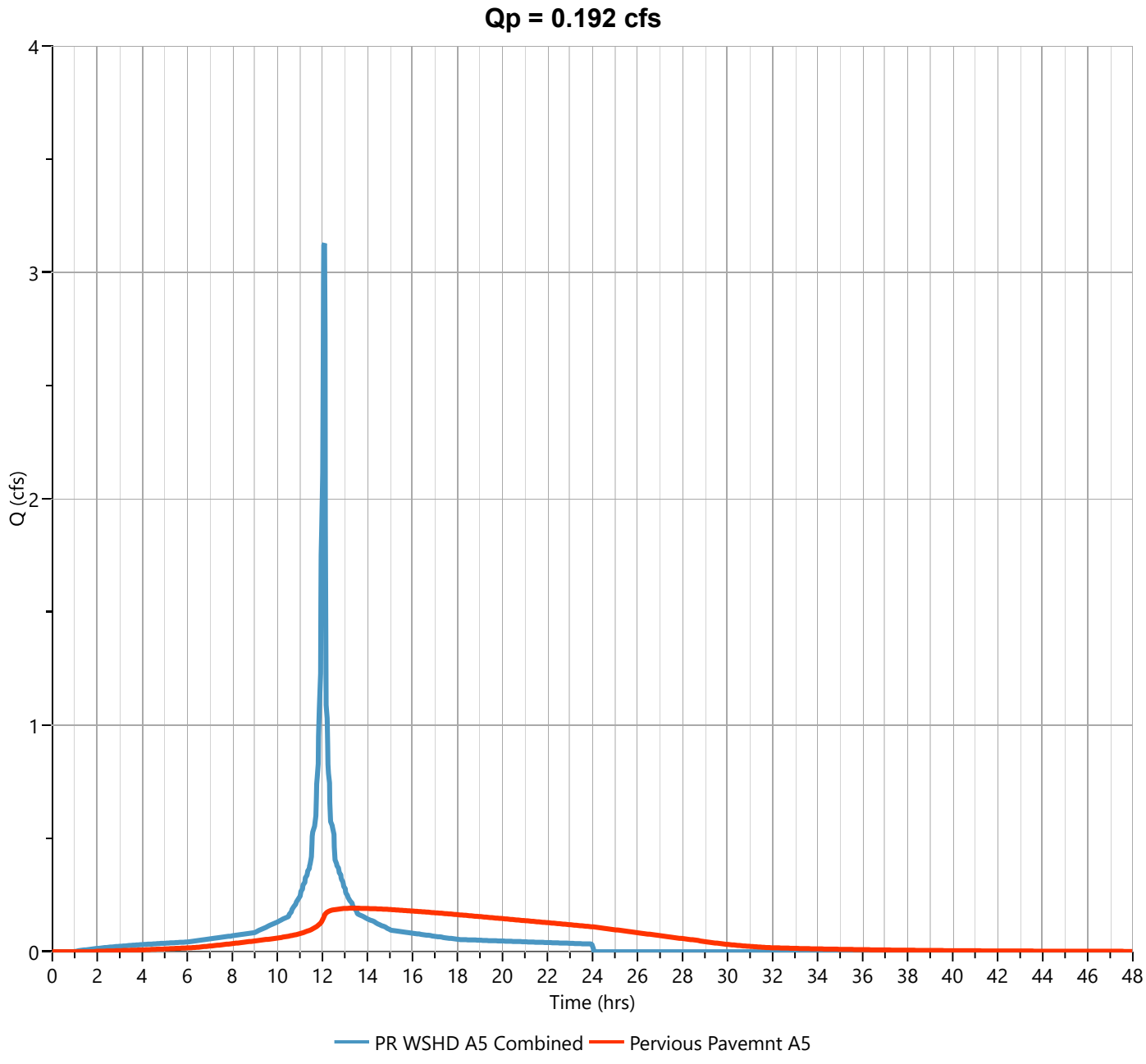
Pervious Pavemnt A5

Hyd. No. 14

Hydrograph Type	= Pond Route	Peak Flow	= 0.192 cfs
Storm Frequency	= 2-yr	Time to Peak	= 13.43 hrs
Time Interval	= 1 min	Hydrograph Volume	= 10,135 cuft
Inflow Hydrograph	= 13 - PR WSHD A5 Combined	Max. Elevation	= 4.87 ft
Pond Name	= Pervious Pavement A5	Max. Storage	= 5,453 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 0.00 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 6.10 hrs



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

03-19-2026

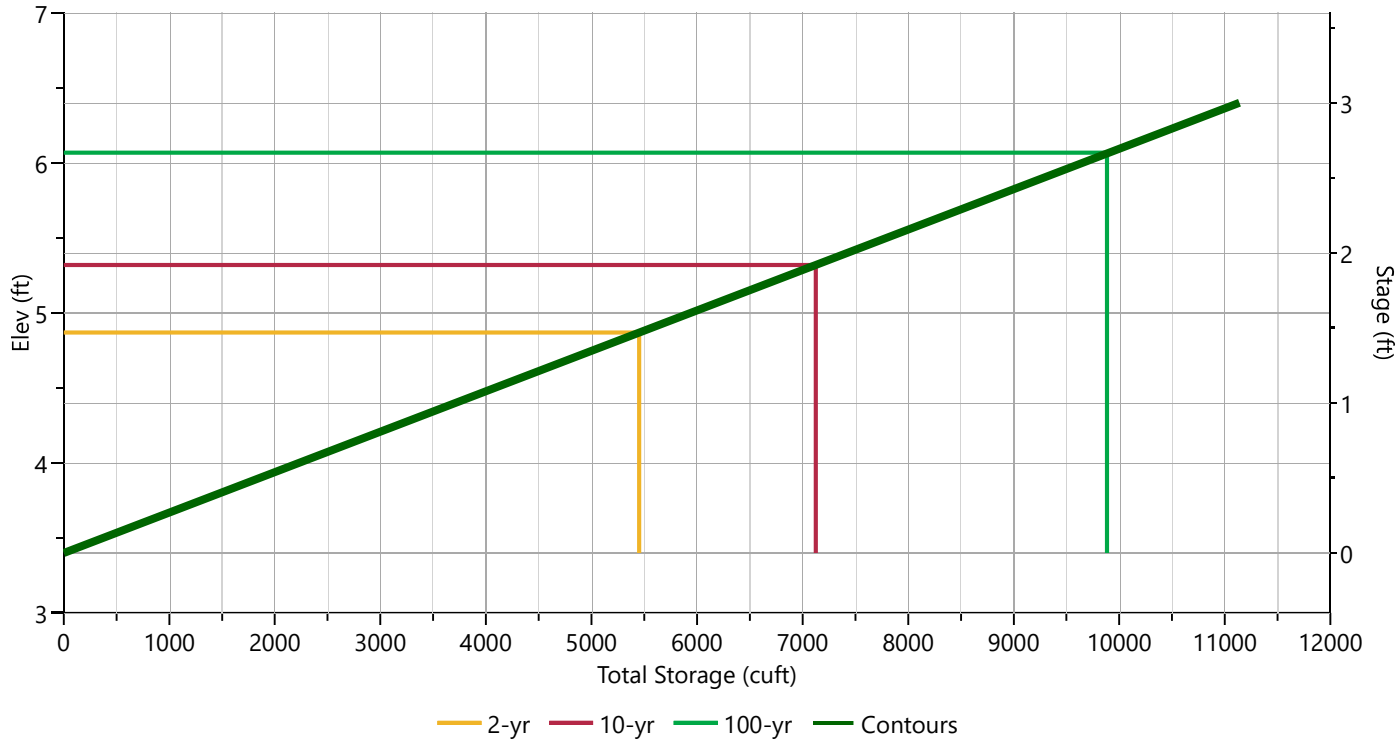
Pervious Pavement A5

Stage-Storage

User Defined Contours	
Description	**Input**
Bottom Elevation, ft	3.40
Voids (%)	40.00
Volume Calc	Ave End Area

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)	Total Storage (cuft)
0.00	3.40	9,282	0.000	0.000
1.00	4.40	9,282	3,713	3,713
2.00	5.40	9,282	3,713	7,426
3.00	6.40	9,282	3,713	11,138



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

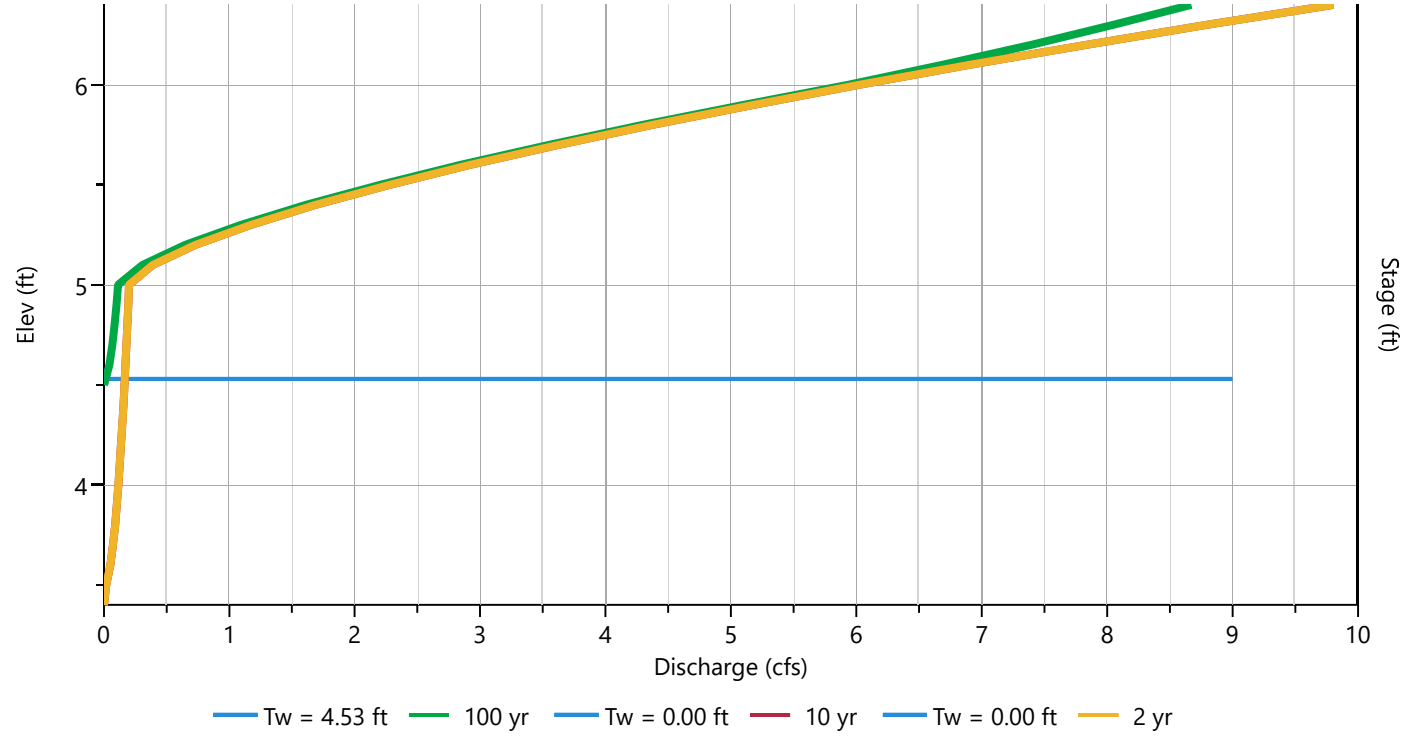
03-19-2026

Pervious Pavement A5

Stage-Discharge

Culvert / Orifices	Cir Culvert	Orifice			Perforated Riser	
		1 (m)	2	3		
Rise, in	18	2.5			Hole Diameter, in	
Span, in	18	2.5			No. holes	
No. Barrels	1	1			Invert Elevation, ft	
Invert Elevation, ft	2.00	3.40			Height, ft	
Orifice Coefficient, Co	0.60	0.60			Orifice Coefficient, Co	
Length, ft	15					
Barrel Slope, %	.3					
N-Value, n	0.012					
Weirs	**Riser**	**Weir**			**Ancillary**	
		1 (m)	2	3		
Shape / Type		Rectangular			Exfiltration, in/hr	
Crest Elevation, ft		5				
Crest Length, ft		1.75				
Angle, deg						
Weir Coefficient, Cw		3.3				

m = Flows through Culvert, i = Independent



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

03-19-2026

Pervious Pavement A5

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total* (cfs)
				1	2	3		1	2	3				
0.00	3.40	0.000	0.000	0.000				0.000						0.000
1.00	4.40	3,713	0.155 oc	0.155				0.000						0.155
2.00	5.40	7,426	1.687 oc	0.226				1.461						1.687
3.00	6.40	11,138	9.805 oc	0.239				9.566						9.805

** No Tailwater. See Stage-Discharge plot for Tw effects. Suffix key: ic = inlet control, oc = outlet control, s = submerged weir*

Pond Report

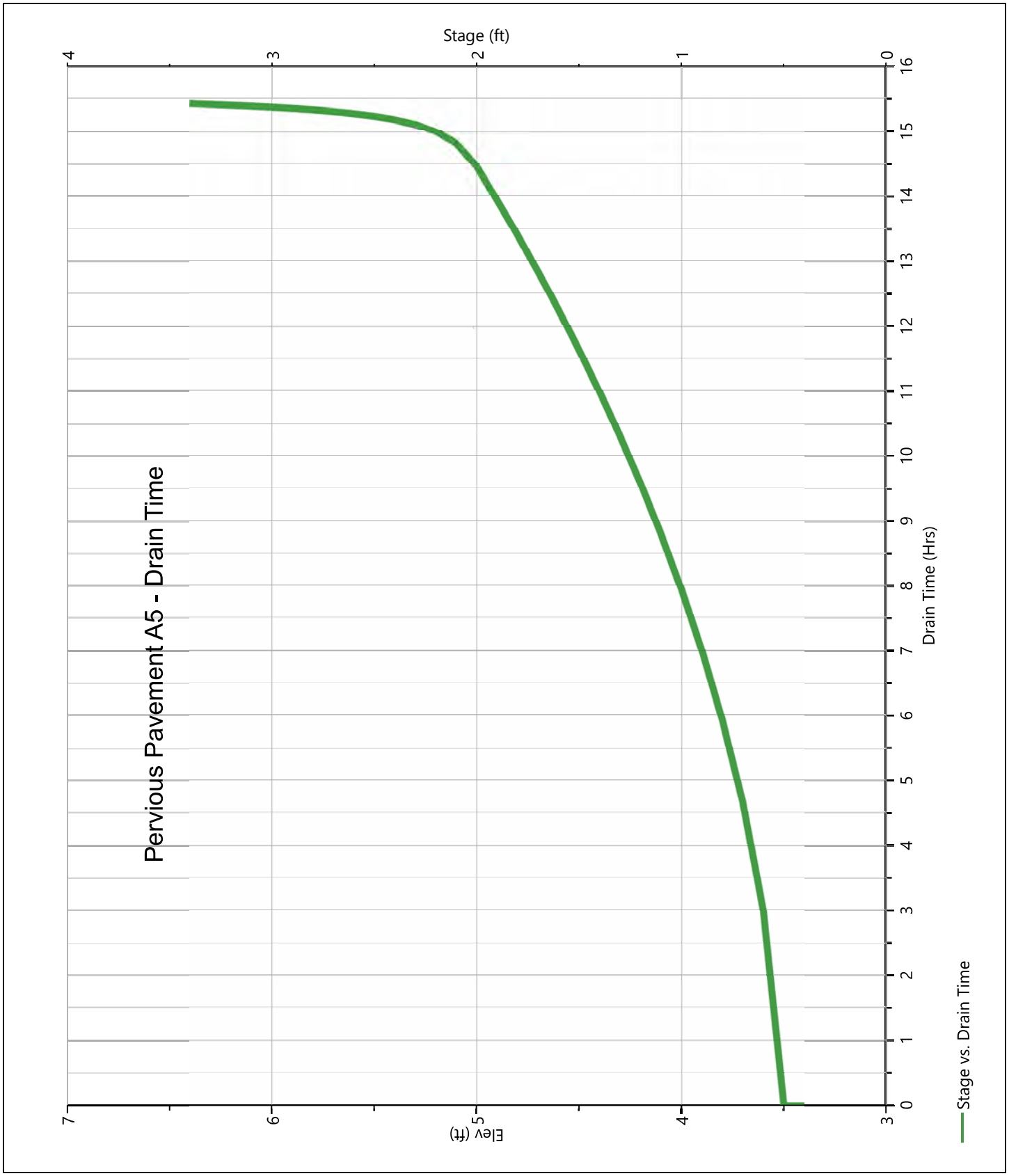
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Hydrology Studio v 3.0.0.43

03-19-2026

Pervious Pavement A5

Pond Drawdown



Hydrograph Report

Hydrology Studio v 3.0.0.43

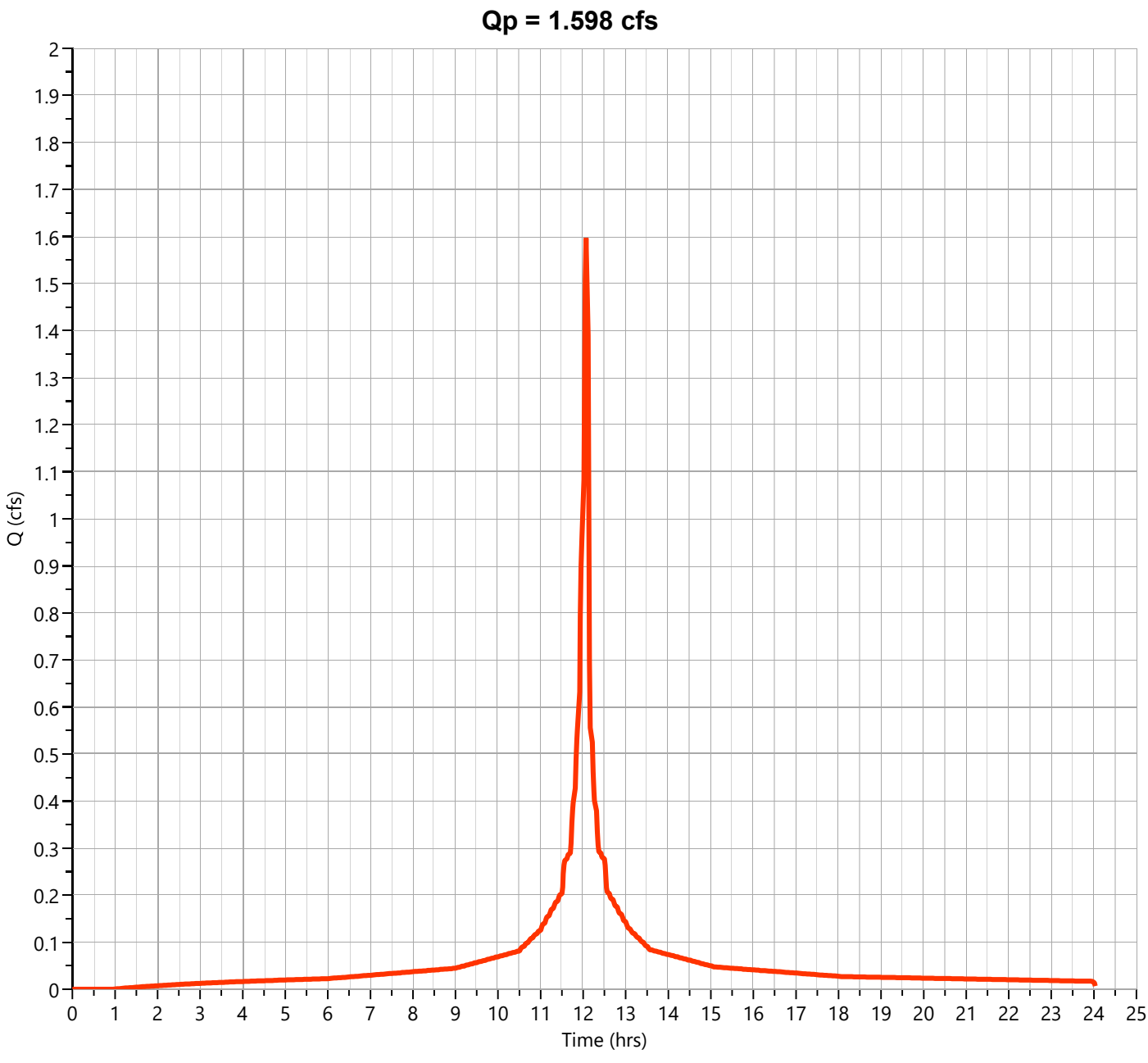
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03-19-2026

PR WSHD A6 Imperv

Hyd. No. 16

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.598 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 5,226 cuft
Drainage Area	= 0.41 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 3.98 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

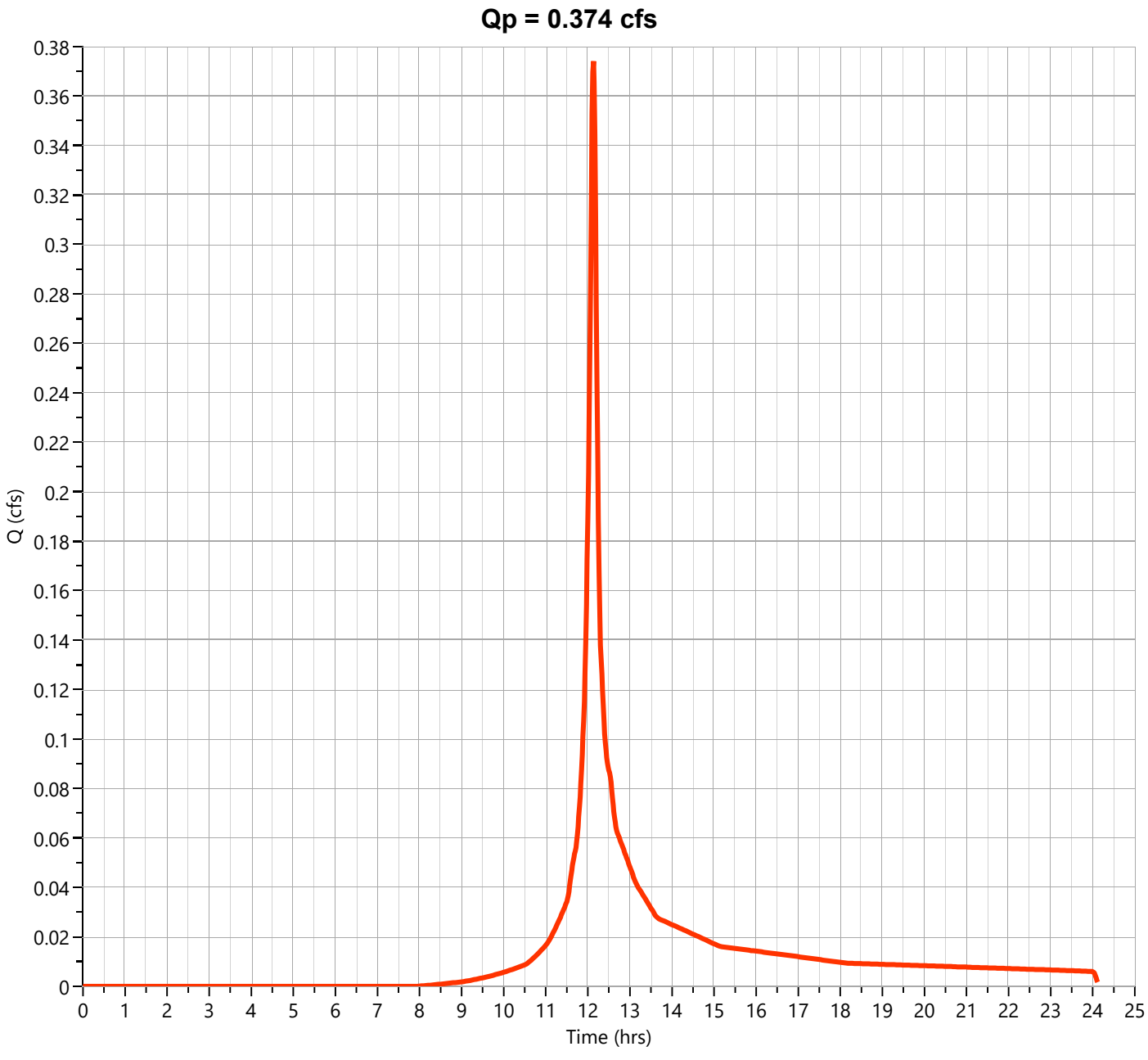
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

03-19-2026

PR WSHD A6 Perv

Hyd. No. 17

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.374 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.13 hrs
Time Interval	= 1 min	Runoff Volume	= 1,218 cuft
Drainage Area	= 0.17 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 8.0 min
Total Rainfall	= 3.98 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

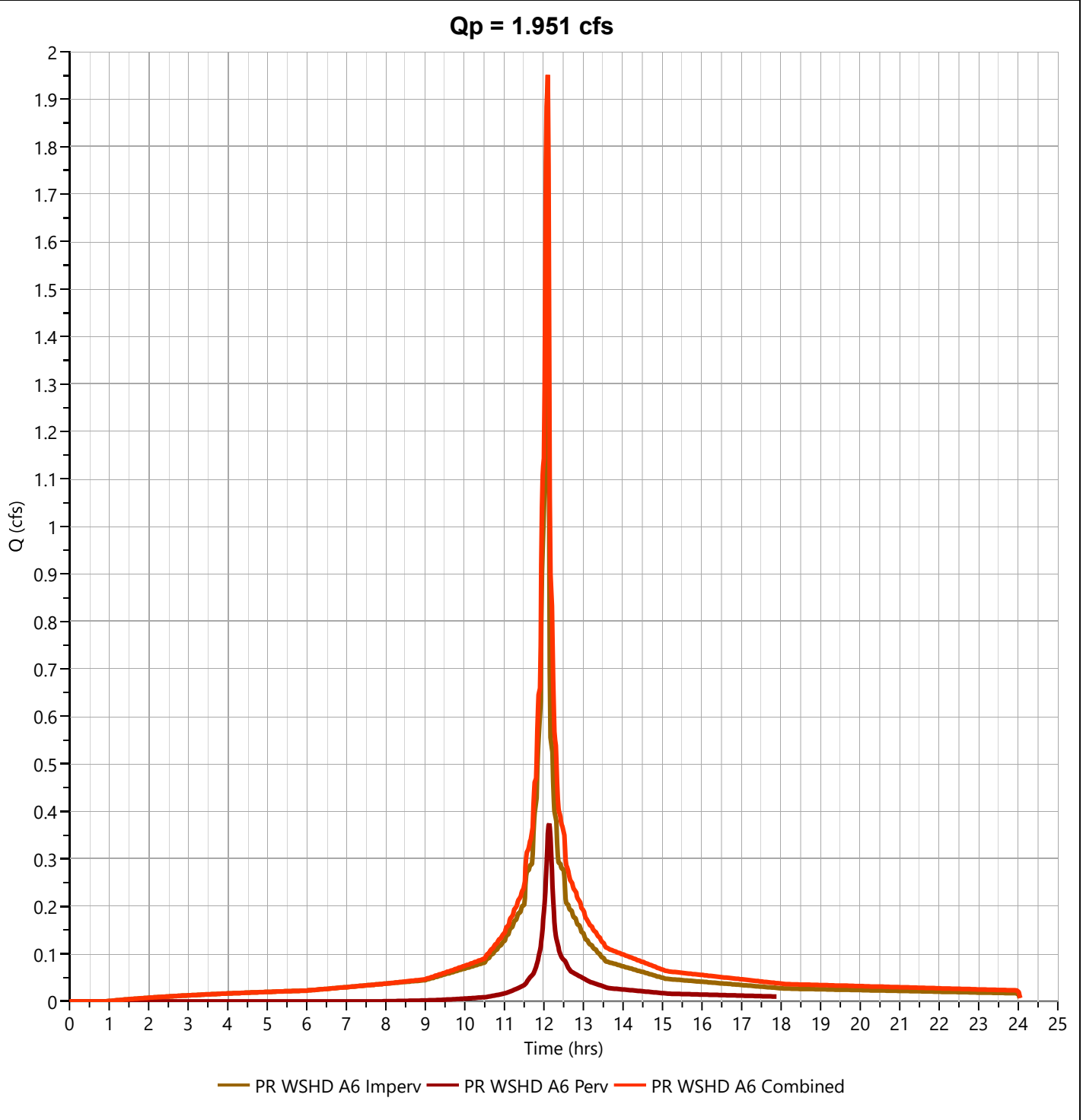
Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

PR WSHD A6 Combined

Hyd. No. 18

Hydrograph Type	= Junction	Peak Flow	= 1.951 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Hydrograph Volume	= 6,444 cuft
Inflow Hydrographs	= 16, 17	Total Contrib. Area	= 0.58 ac



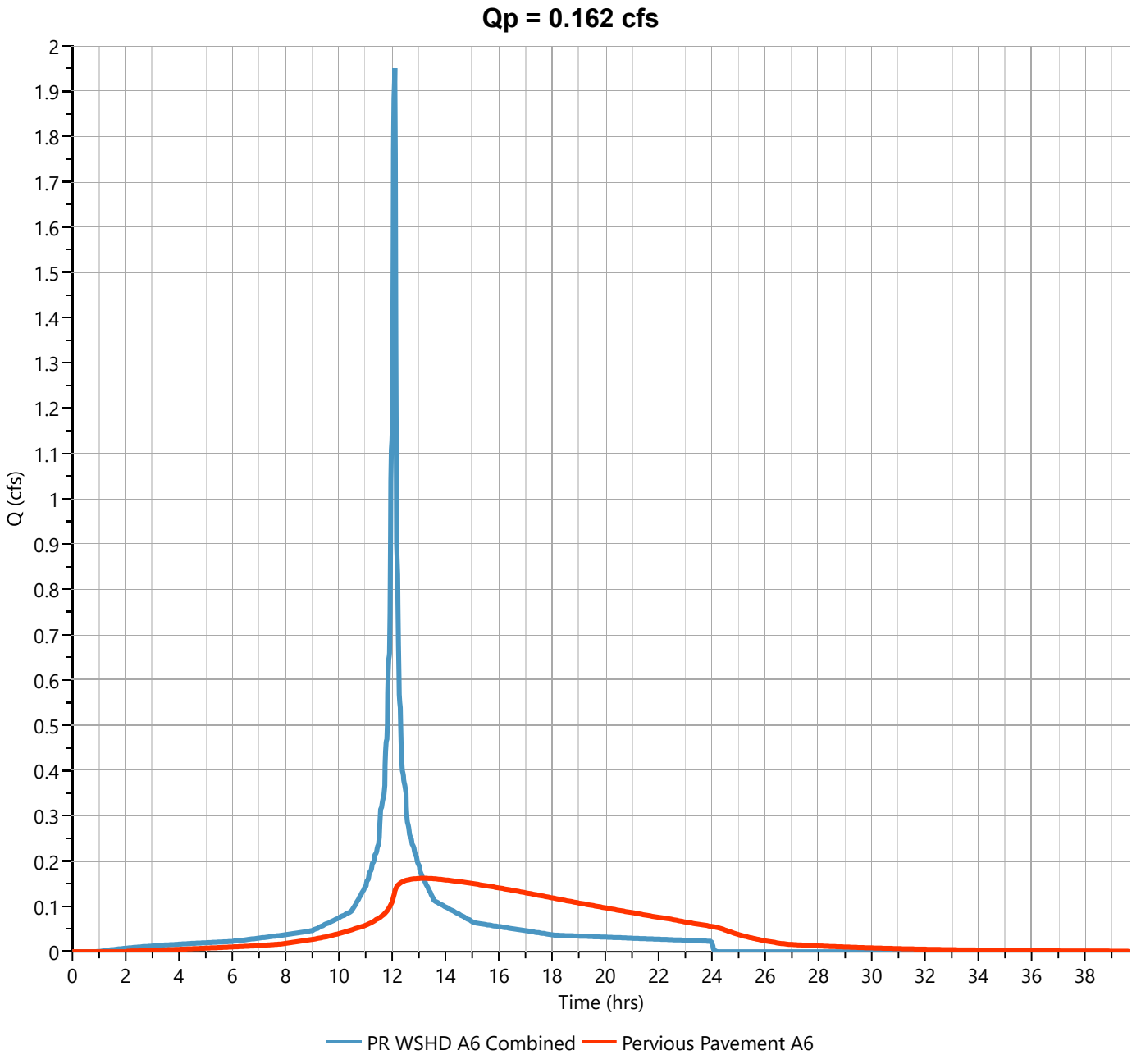
Pervious Pavement A6

Hyd. No. 19

Hydrograph Type	= Pond Route	Peak Flow	= 0.162 cfs
Storm Frequency	= 2-yr	Time to Peak	= 13.15 hrs
Time Interval	= 1 min	Hydrograph Volume	= 6,428 cuft
Inflow Hydrograph	= 18 - PR WSHD A6 Combined	Max. Elevation	= 3.64 ft
Pond Name	= Pervious Pavement A6	Max. Storage	= 3,117 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 0.00 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 4.21 hrs



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

03-19-2026

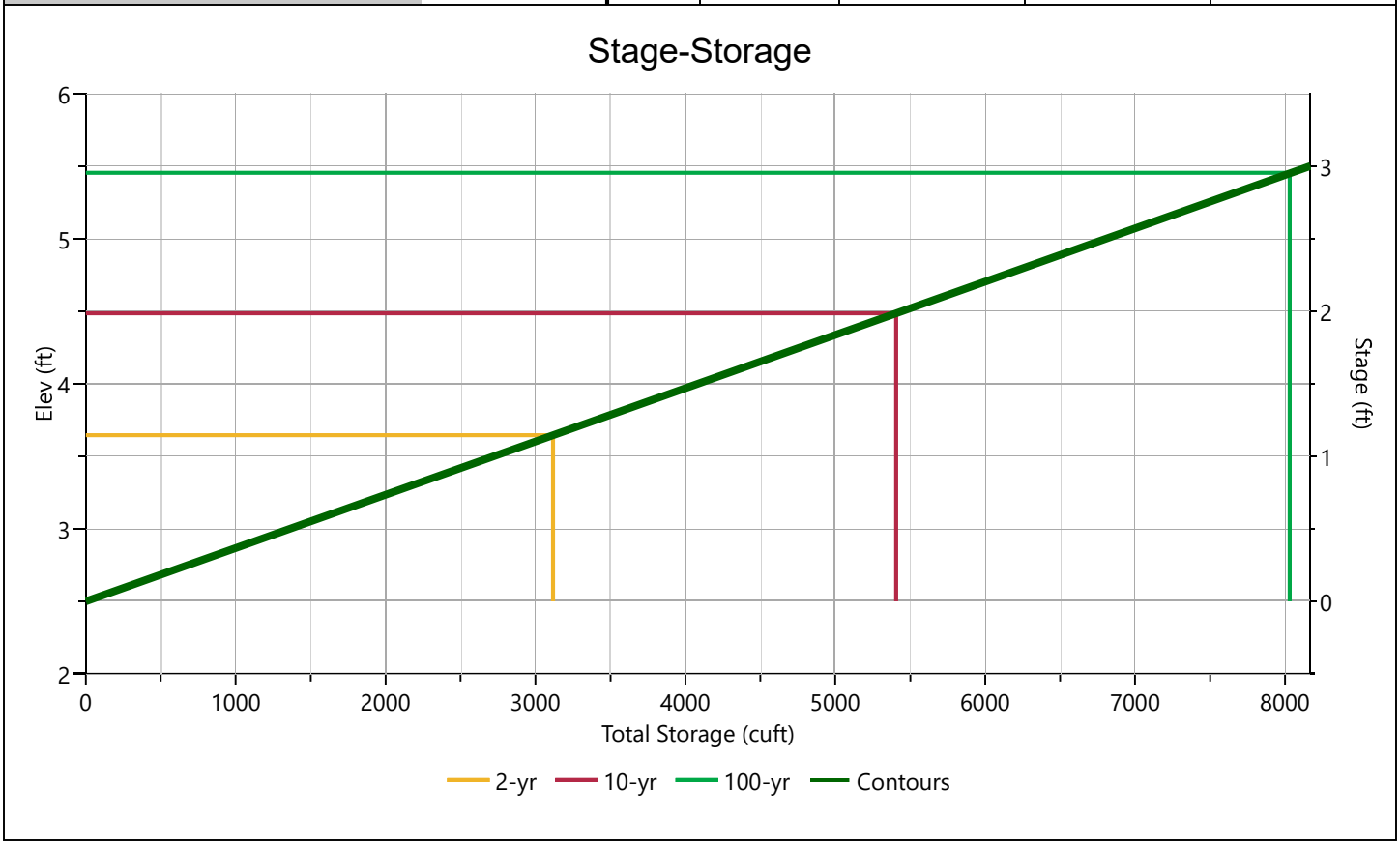
Pervious Pavement A6

Stage-Storage

User Defined Contours	
Description	**Input**
Bottom Elevation, ft	2.50
Voids (%)	40.00
Volume Calc	Ave End Area

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)	Total Storage (cuft)
0.00	2.50	6,808	0.000	0.000
1.00	3.50	6,808	2,723	2,723
2.00	4.50	6,808	2,723	5,446
3.00	5.50	6,808	2,723	8,170



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

03-19-2026

Pervious Pavement A6

Stage-Discharge

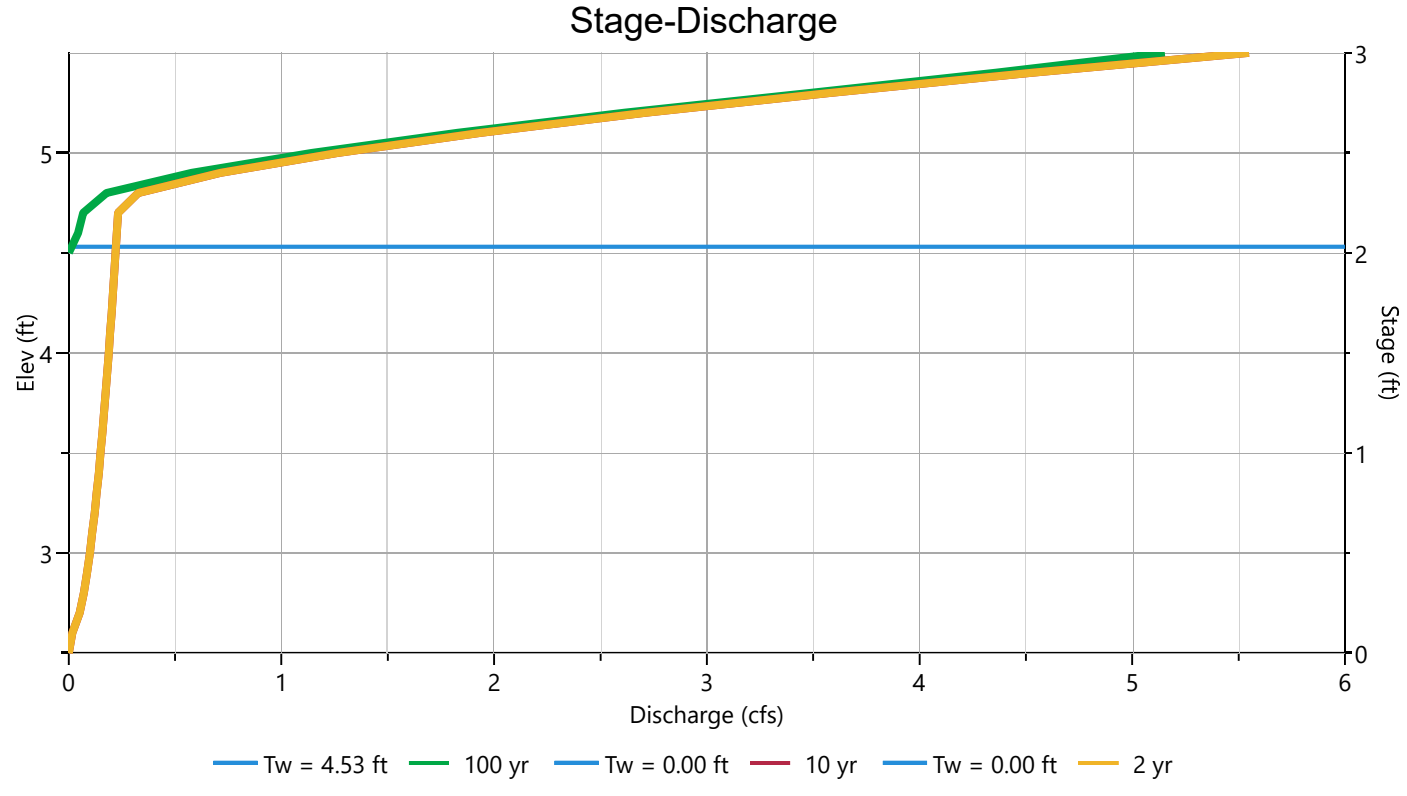
Culvert / Orifices	Cir Culvert	Orifice		
		1 (m)	2	3
Rise, in	18	2.5		
Span, in	18	2.5		
No. Barrels	1	1		
Invert Elevation, ft	2.50	2.50		
Orifice Coefficient, Co	0.60	0.60		
Length, ft	115			
Barrel Slope, %	.3			
N-Value, n	0.012			

Perforated Riser	
Hole Diameter, in	
No. holes	
Invert Elevation, ft	
Height, ft	
Orifice Coefficient, Co	

Weirs	Riser	Weir		
		1 (m)	2	3
Shape / Type		Rectangular		
Crest Elevation, ft		4.75		
Crest Length, ft		2.5		
Angle, deg				
Weir Coefficient, Cw		3.3		

Ancillary	
Exfiltration, in/hr	

m = Flows through Culvert, i = Independent



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

03-19-2026

Pervious Pavement A6

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total* (cfs)
				1	2	3		1	2	3				
0.00	2.50	0.000	0.000	0.000				0.000						0.000
1.00	3.50	2,723	0.150 ic	0.150				0.000						0.150
2.00	4.50	5,446	0.220 ic	0.220				0.000						0.220
3.00	5.50	8,170	5.549 oc	0.190				5.359						5.549

** No Tailwater. See Stage-Discharge plot for Tw effects. Suffix key: ic = inlet control, oc = outlet control, s = submerged weir*

Pond Report

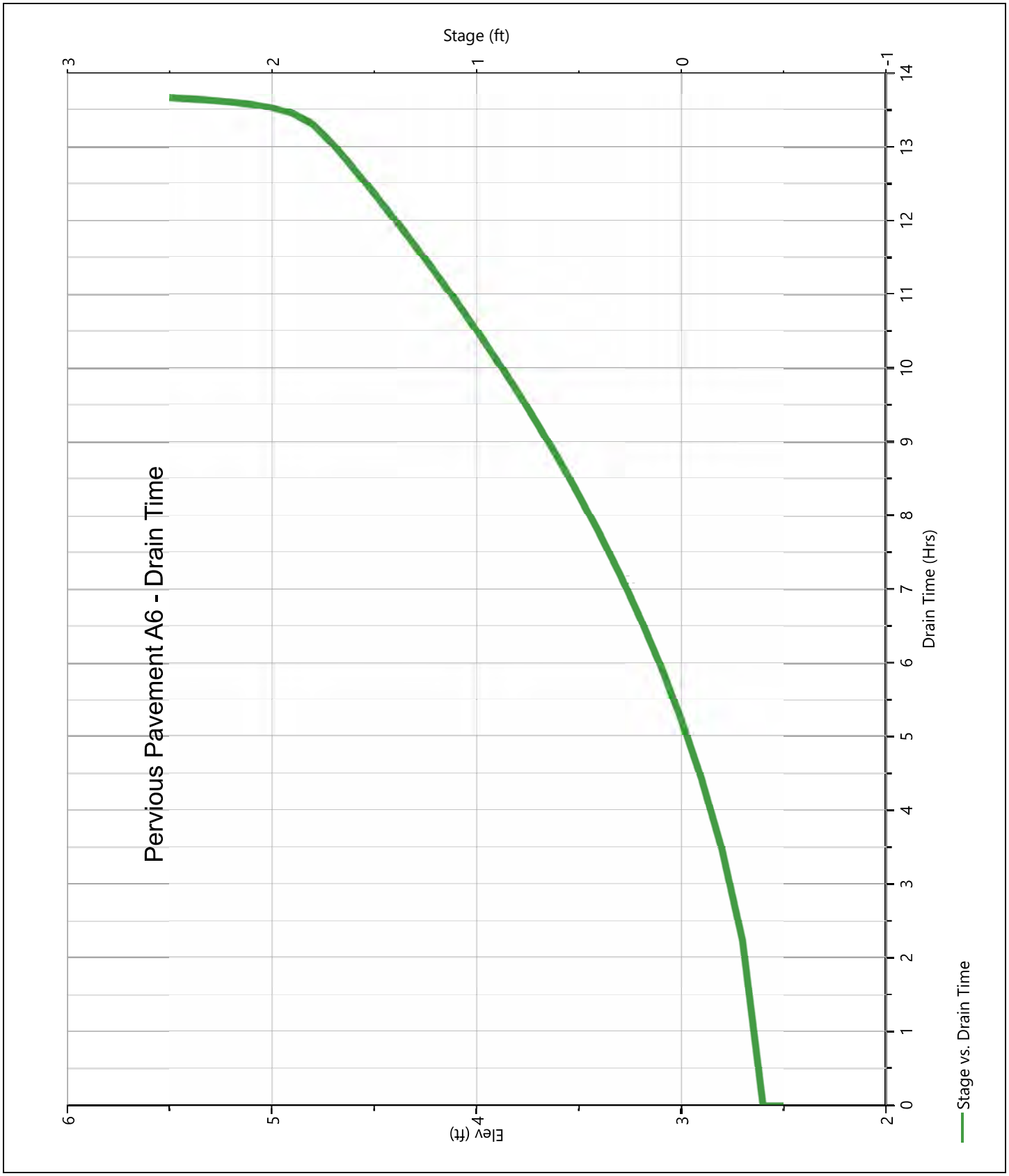
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Hydrology Studio v 3.0.0.43

03-19-2026

Pervious Pavement A6

Pond Drawdown



Hydrograph Report

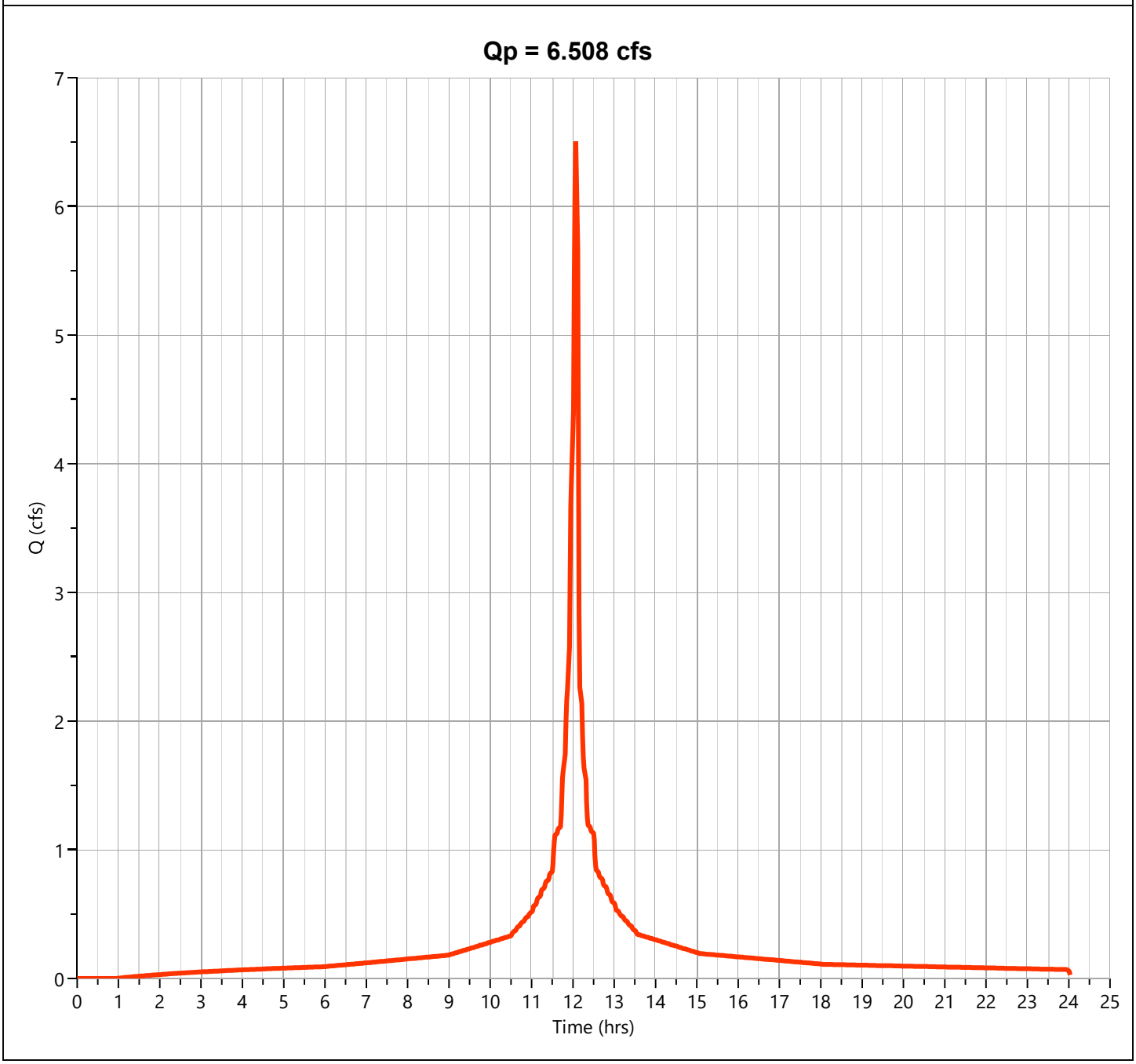
Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

PR WSHD A7 Imperv

Hyd. No. 21

Hydrograph Type	= NRCS Runoff	Peak Flow	= 6.508 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 21,285 cuft
Drainage Area	= 1.67 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 3.98 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

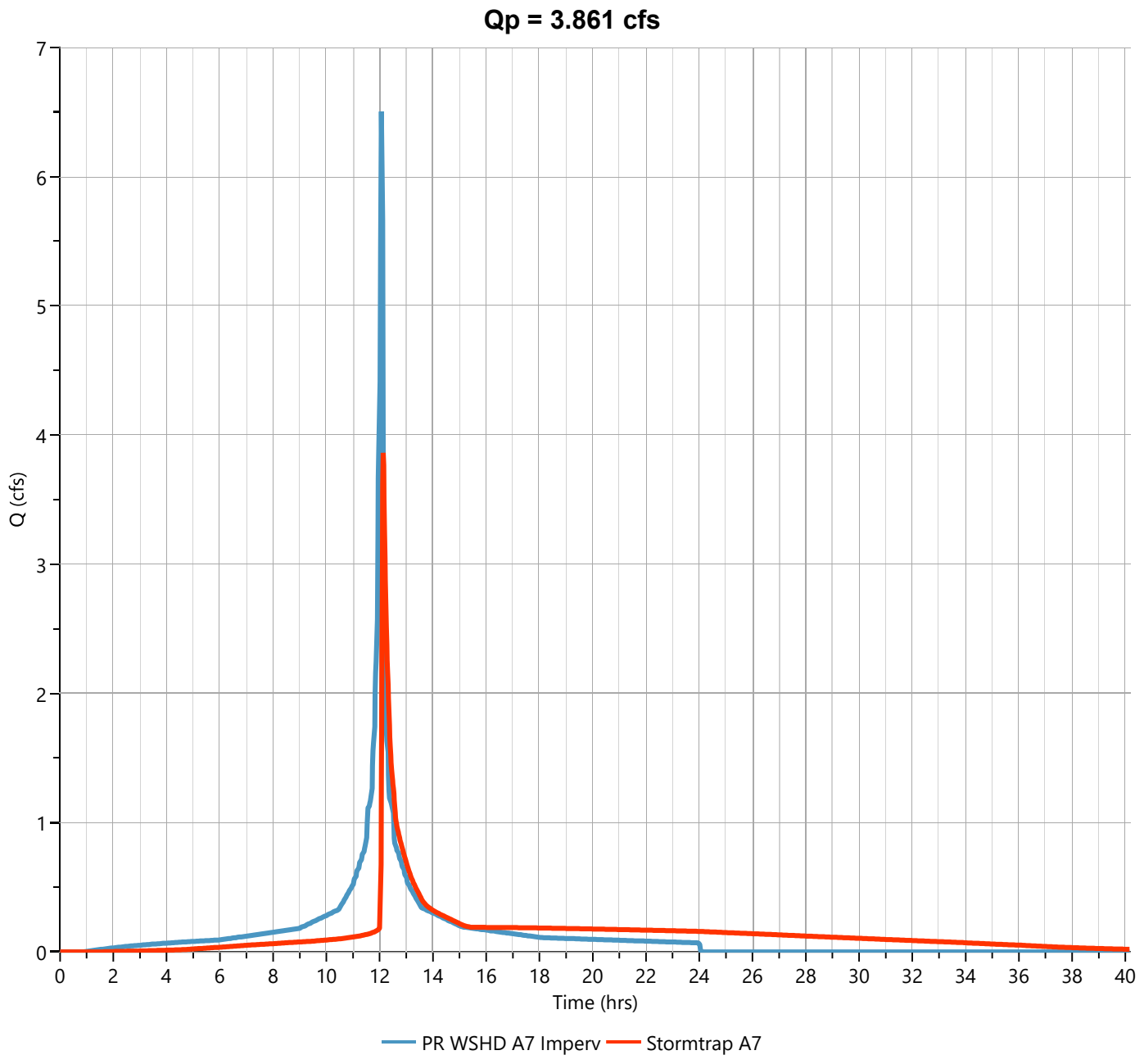
Stormtrap A7

Hyd. No. 22

Hydrograph Type	= Pond Route	Peak Flow	= 3.861 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.13 hrs
Time Interval	= 1 min	Hydrograph Volume	= 21,073 cuft
Inflow Hydrograph	= 21 - PR WSHD A7 Imperv	Max. Elevation	= 4.77 ft
Pond Name	= Stormtrap A7	Max. Storage	= 9,069 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 0.00 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 5.63 hrs



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

03-19-2026

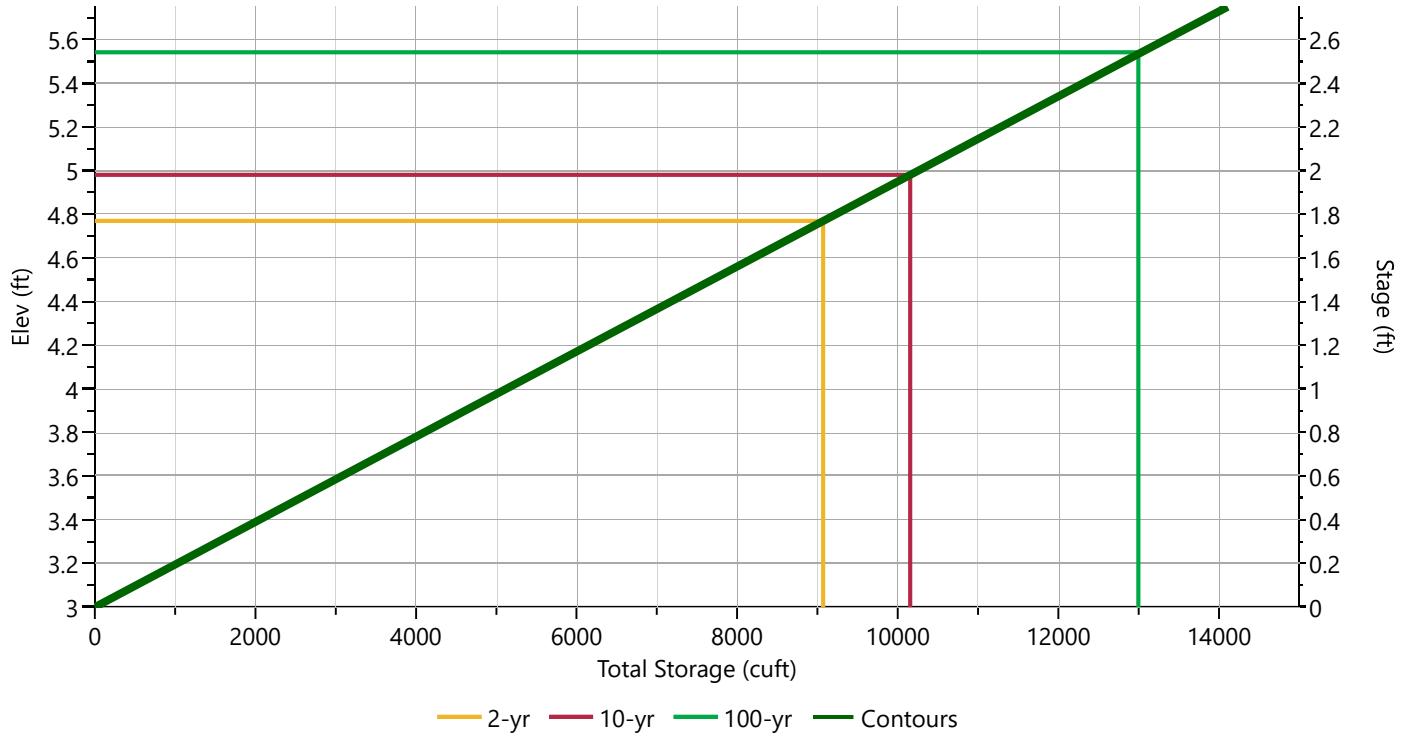
Stormtrap A7

Stage-Storage

User Defined Contours	
Description	**Input**
Bottom Elevation, ft	3.00
Voids (%)	100.00
Volume Calc	None

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)	Total Storage (cuft)
0.00	3.00	5,130	0.000	0.000
1.00	4.00	5,130	5,130	5,130
2.00	5.00	5,130	5,130	10,260
2.50	5.50	5,130	2,565	12,825
2.75	5.75	5,130	1,283	14,108



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

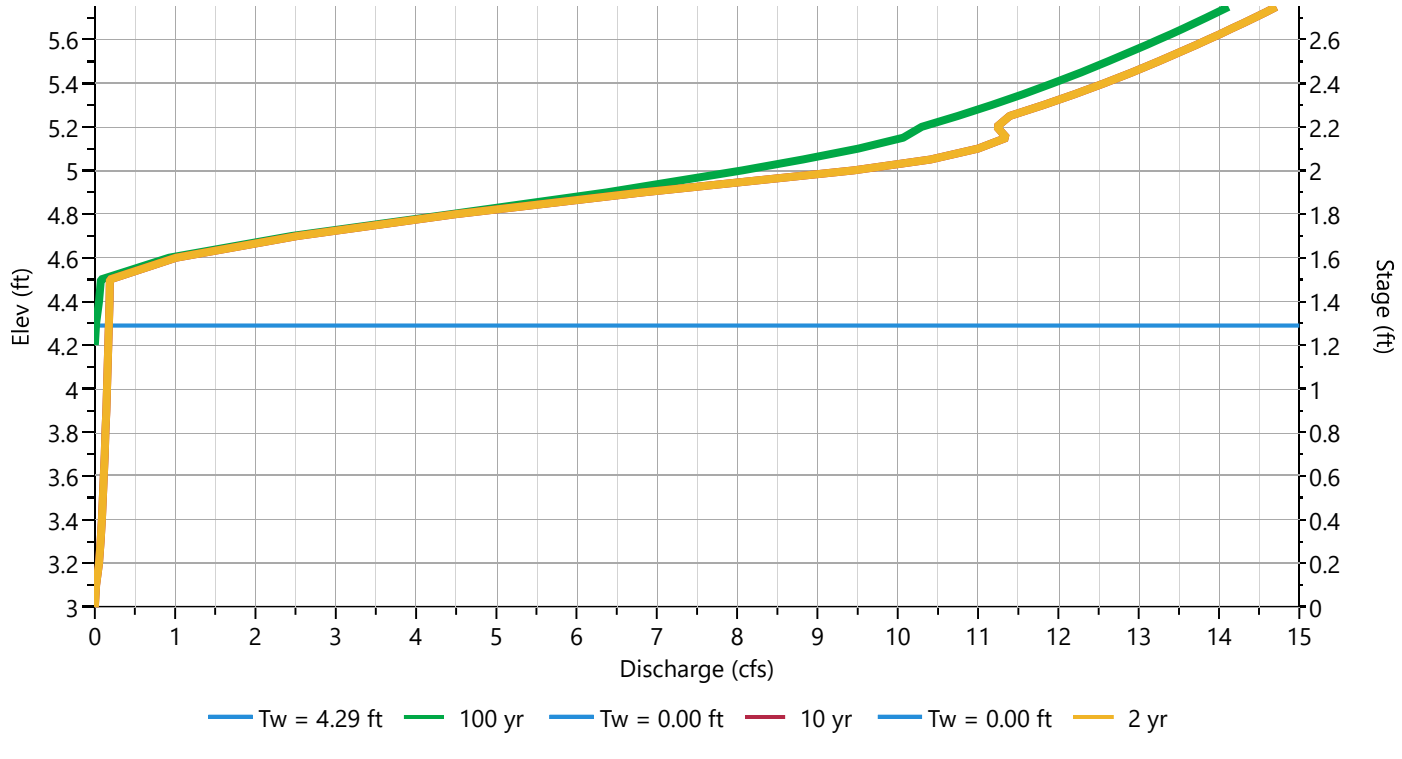
03-19-2026

Stormtrap A7

Stage-Discharge

Culvert / Orifices	Cir Culvert	Orifice			Perforated Riser	
		1 (m)	2	3		
Rise, in	24	2.5			Hole Diameter, in	
Span, in	24	2.5			No. holes	
No. Barrels	1	1			Invert Elevation, ft	
Invert Elevation, ft	3.00	3.00			Height, ft	
Orifice Coefficient, Co	0.60	0.60			Orifice Coefficient, Co	
Length, ft	279					
Barrel Slope, %	.3					
N-Value, n	0.012					
Weirs	**Riser**	**Weir**			**Ancillary**	
		1 (m)	2	3		
Shape / Type		Rectangular			Exfiltration, in/hr	
Crest Elevation, ft		4.5				
Crest Length, ft		8				
Angle, deg						
Weir Coefficient, Cw		3.3				

m = Flows through Culvert, i = Independent



Pond Report

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

03-19-2026

Stormtrap A7

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total* (cfs)
				1	2	3		1	2	3				
0.00	3.00	0.000	0.000	0.000				0.000						0.000
1.00	4.00	5,130	0.151 ic	0.151				0.000						0.151
2.00	5.00	10,260	9.452 oc	0.118				9.334						9.452
2.50	5.50	12,825	13.25 oc	0.055				13.19 s						13.25
2.75	5.75	14,108	14.71 oc	0.046				14.67 s						14.71

** No Tailwater. See Stage-Discharge plot for Tw effects. Suffix key: ic = inlet control, oc = outlet control, s = submerged weir*

Pond Report

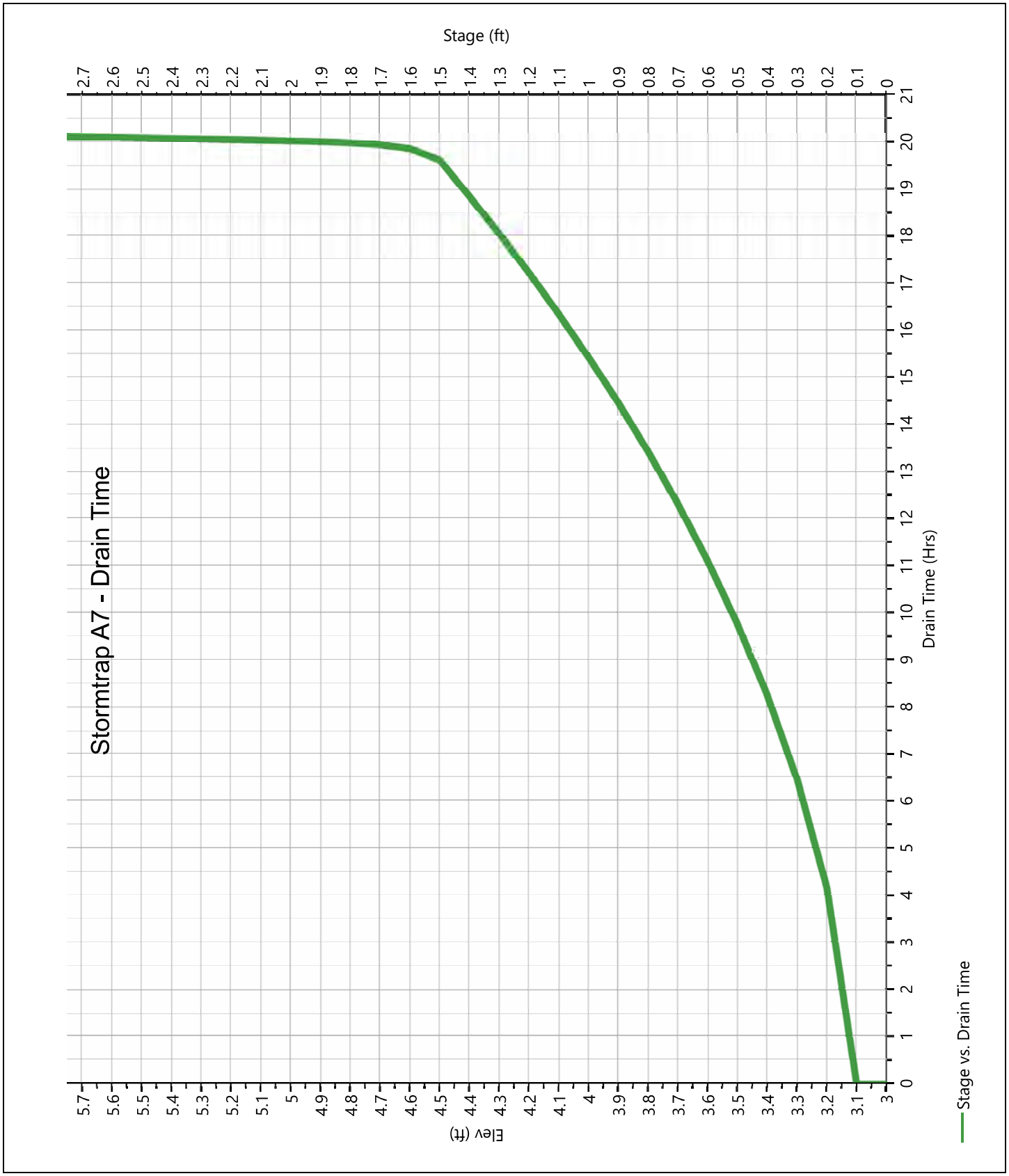
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

Hydrology Studio v 3.0.0.43

03-19-2026

Stormtrap A7

Pond Drawdown



Hydrograph Report

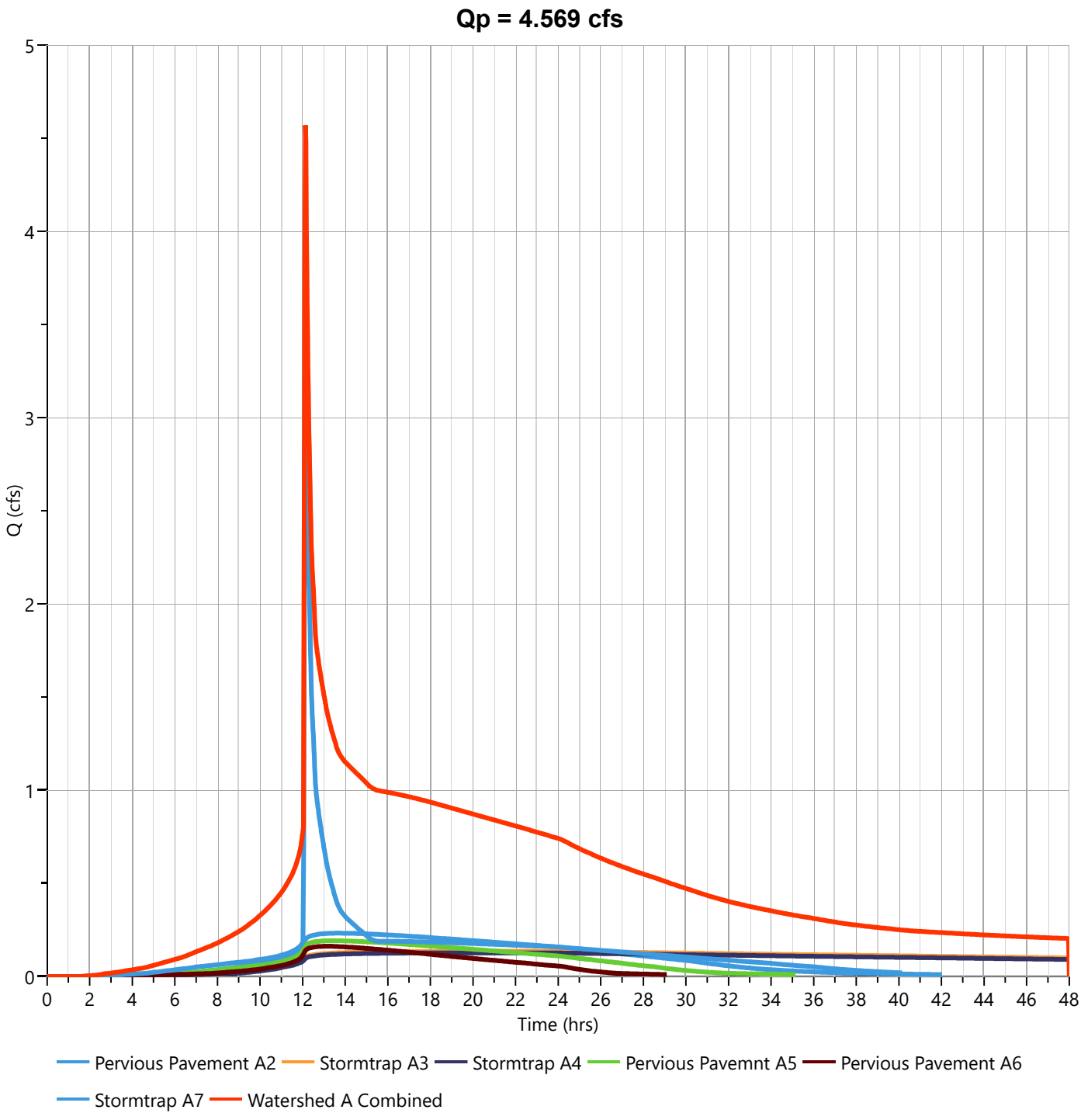
Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

Watershed A Combined

Hyd. No. 24

Hydrograph Type	= Junction	Peak Flow	= 4.569 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.13 hrs
Time Interval	= 1 min	Hydrograph Volume	= 83,632 cuft
Inflow Hydrographs		Total Contrib. Area	= 0.0 ac



Hydrograph Report

Hydrology Studio v 3.0.0.43

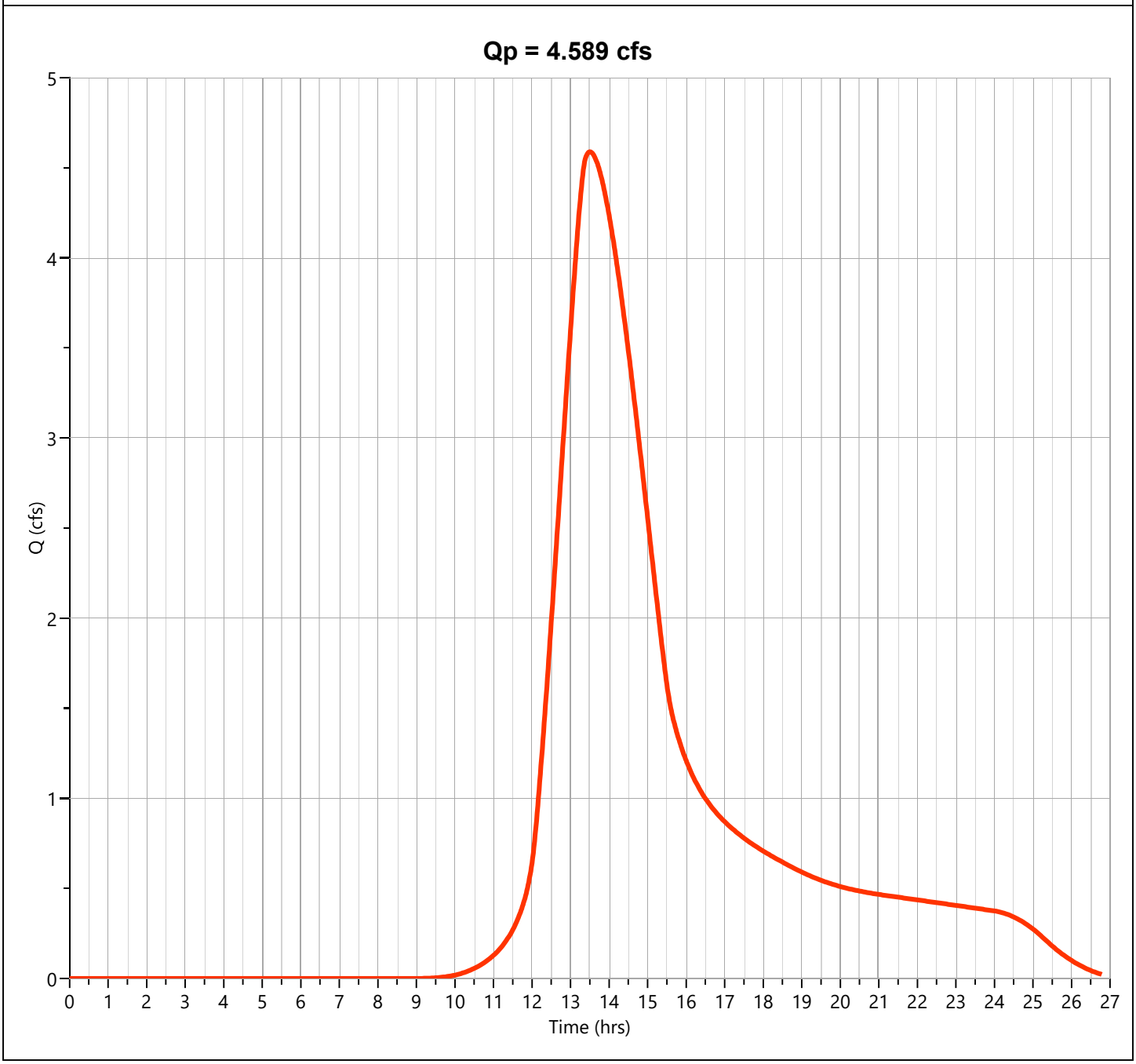
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

03-19-2026

PR WSHD B Perv

Hyd. No. 26

Hydrograph Type	= NRCS Runoff	Peak Flow	= 4.589 cfs
Storm Frequency	= 2-yr	Time to Peak	= 13.50 hrs
Time Interval	= 1 min	Runoff Volume	= 62,045 cuft
Drainage Area	= 9.5 ac	Curve Number	= 77.00
Tc Method	= User	Time of Conc. (Tc)	= 134.0 min
Total Rainfall	= 3.98 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

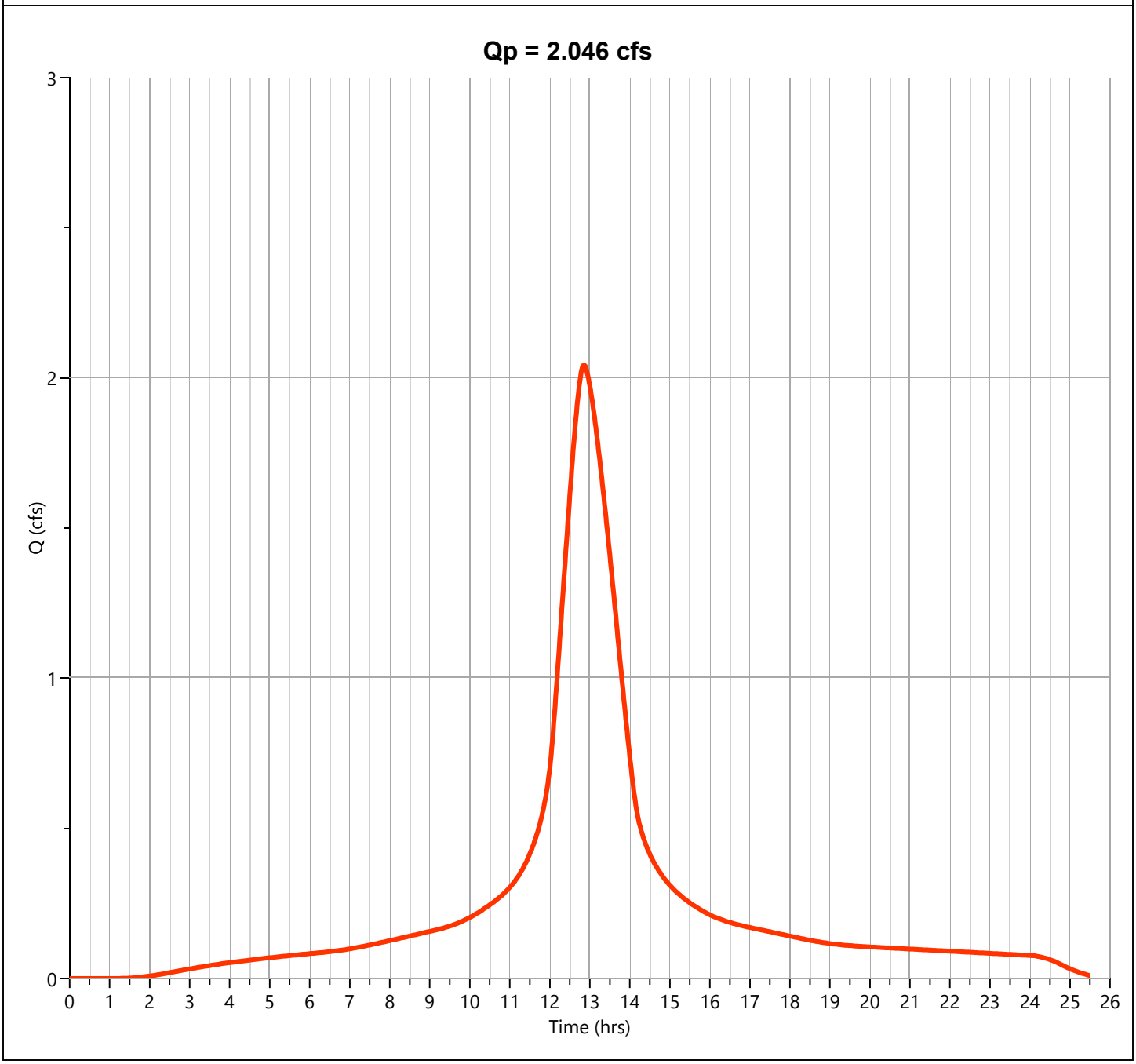
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

03-19-2026

PR WSHD B Imperv

Hyd. No. 27

Hydrograph Type	= NRCS Runoff	Peak Flow	= 2.046 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.85 hrs
Time Interval	= 1 min	Runoff Volume	= 21,752 cuft
Drainage Area	= 1.6 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 81.0 min
Total Rainfall	= 3.98 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

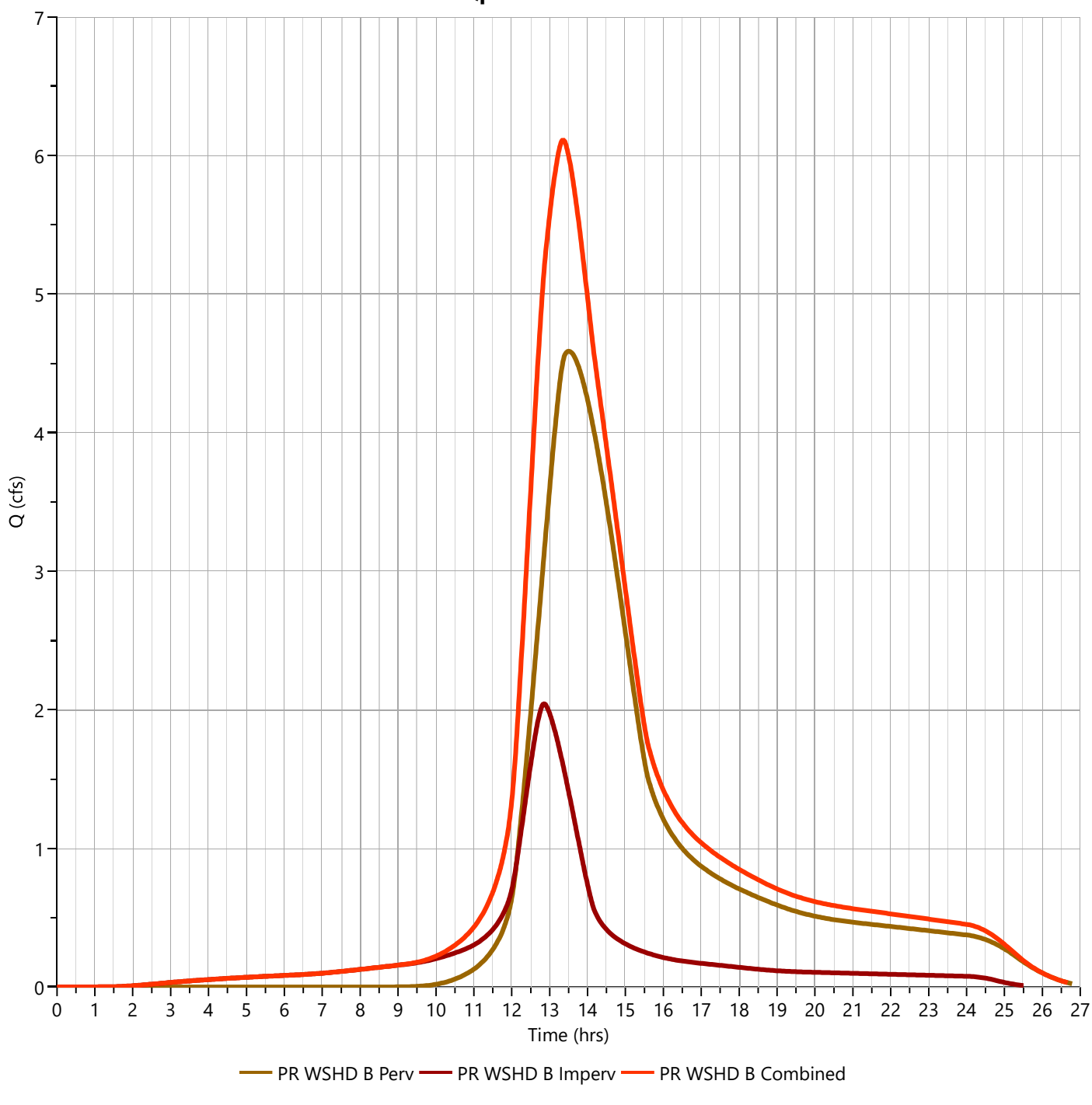
Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

PR WSHD B Combined

Hyd. No. 28

Hydrograph Type	= Junction	Peak Flow	= 6.116 cfs
Storm Frequency	= 2-yr	Time to Peak	= 13.35 hrs
Time Interval	= 1 min	Hydrograph Volume	= 83,797 cuft
Inflow Hydrographs	= 26, 27	Total Contrib. Area	= 11.1 ac



Hydrograph Report

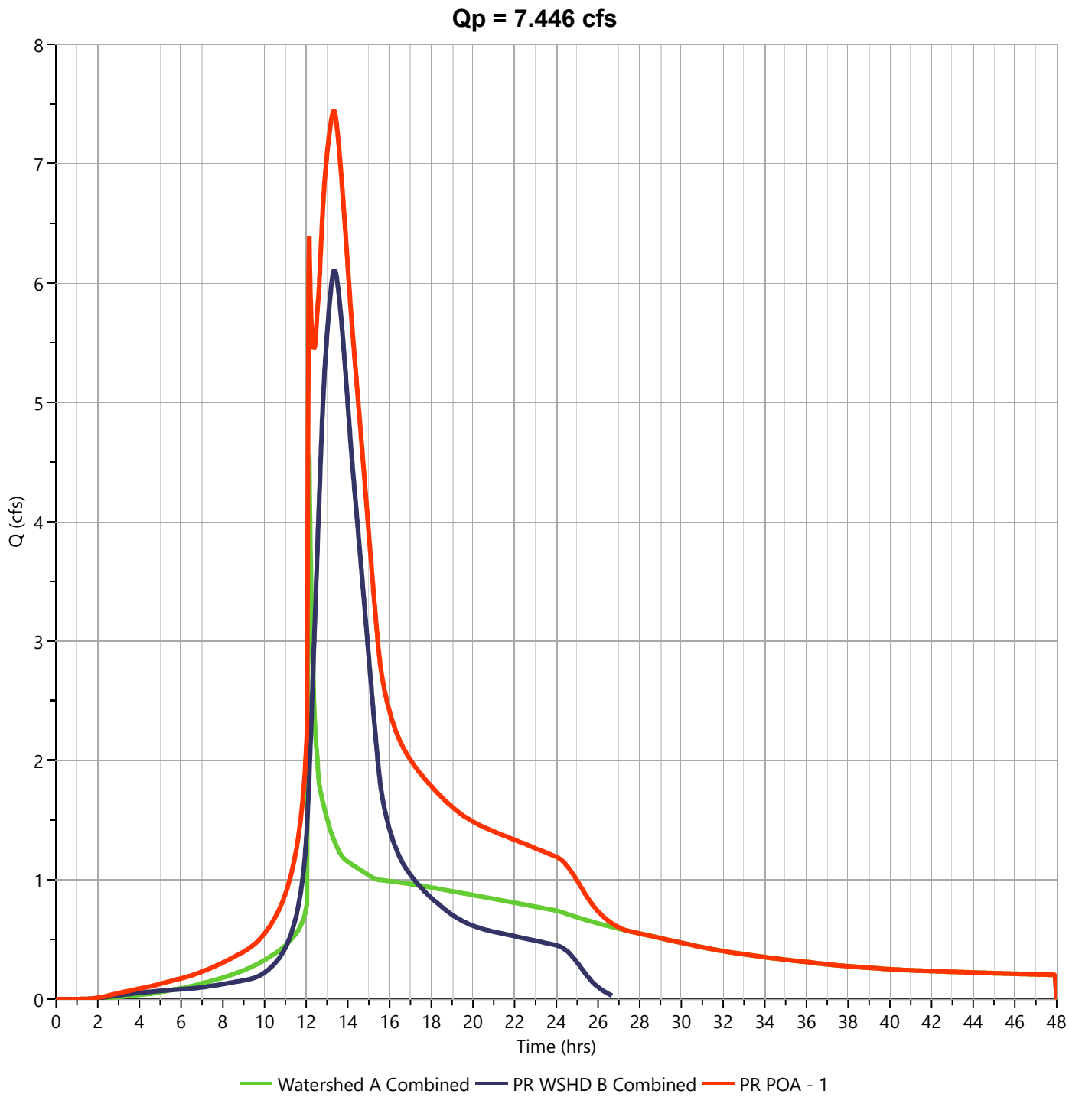
Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

PR POA - 1

Hyd. No. 30

Hydrograph Type	= Junction	Peak Flow	= 7.446 cfs
Storm Frequency	= 2-yr	Time to Peak	= 13.33 hrs
Time Interval	= 1 min	Hydrograph Volume	= 167,428 cuft
Inflow Hydrographs		Total Contrib. Area	= 0.0 ac



Hydrograph Report

Hydrology Studio v 3.0.0.43

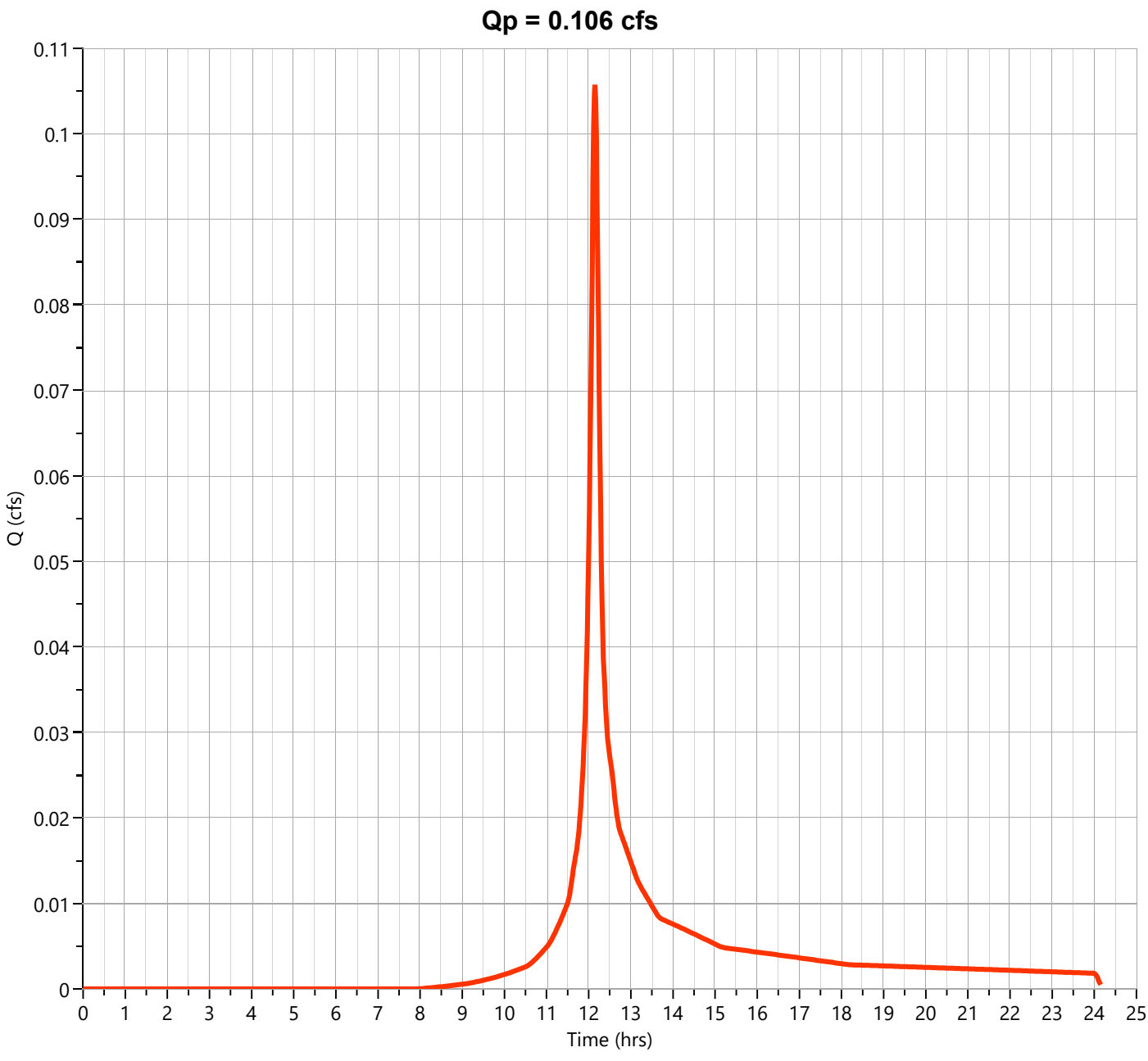
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

03-19-2026

PR WSHD C PerPOA-2

Hyd. No. 33

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.106 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.15 hrs
Time Interval	= 1 min	Runoff Volume	= 368 cuft
Drainage Area	= 0.05 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 10.0 min
Total Rainfall	= 3.98 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph 10-yr Summary

Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

03-19-2026

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	NRCS Runoff	PR WSDH A2 Imperv	6.978	12.10	23,214	----		
2	Pond Route	Pervious Pavement A2	0.969	12.55	22,777	1	2.16	12,156
4	NRCS Runoff	PR WSHD A3 Imperv	14.84	12.10	50,385	----		
5	Pond Route	Stormtrap A3	0.178	22.87	22,472	4	2.96	41,721
7	NRCS Runoff	PR WSHD A4 Imperv	13.38	12.10	45,433	----		
8	Pond Route	Stormtrap A4	0.167	22.33	21,007	7	2.81	37,350
9	Junction	A3 &A4 outflow	0.345	22.62	43,478	5, 8		
11	NRCS Runoff	PR WSHD A5 Imperv	4.672	12.10	15,543	----		
12	NRCS Runoff	PR WSHD A5 Perv	0.239	12.10	670	----		
13	Junction	PR WSHD A5 Combined	4.911	12.10	16,213	11, 12		
14	Pond Route	Pervious Pavemnt A5	1.268	12.25	16,179	13	5.32	7,125
16	NRCS Runoff	PR WSHD A6 Imperv	2.488	12.10	8,276	----		
17	NRCS Runoff	PR WSHD A6 Perv	0.716	12.13	2,368	----		
18	Junction	PR WSHD A6 Combined	3.171	12.10	10,644	16, 17		
19	Pond Route	Pervious Pavement A6	0.219	13.40	10,627	18	4.48	5,405
21	NRCS Runoff	PR WSHD A7 Imperv	10.13	12.10	33,711	----		
22	Pond Route	Stormtrap A7	8.908	12.12	33,443	21	4.98	10,154
24	Junction	Watershed A Combined	10.36	12.12	126,504	2, 5, 8, 14, 19, 22		
26	NRCS Runoff	PR WSHD B Perv	9.458	13.43	125,308	----		
27	NRCS Runoff	PR WSHD B Imperv	3.191	12.85	34,451	----		
28	Junction	PR WSHD B Combined	11.88	13.35	159,759	26, 27		
30	Junction	PR POA - 1	14.27	13.30	286,264	24, 28		
33	NRCS Runoff	PR WSHD C PerPOA-2	0.203	12.15	714	----		

Hydrograph Report

Hydrology Studio v 3.0.0.43

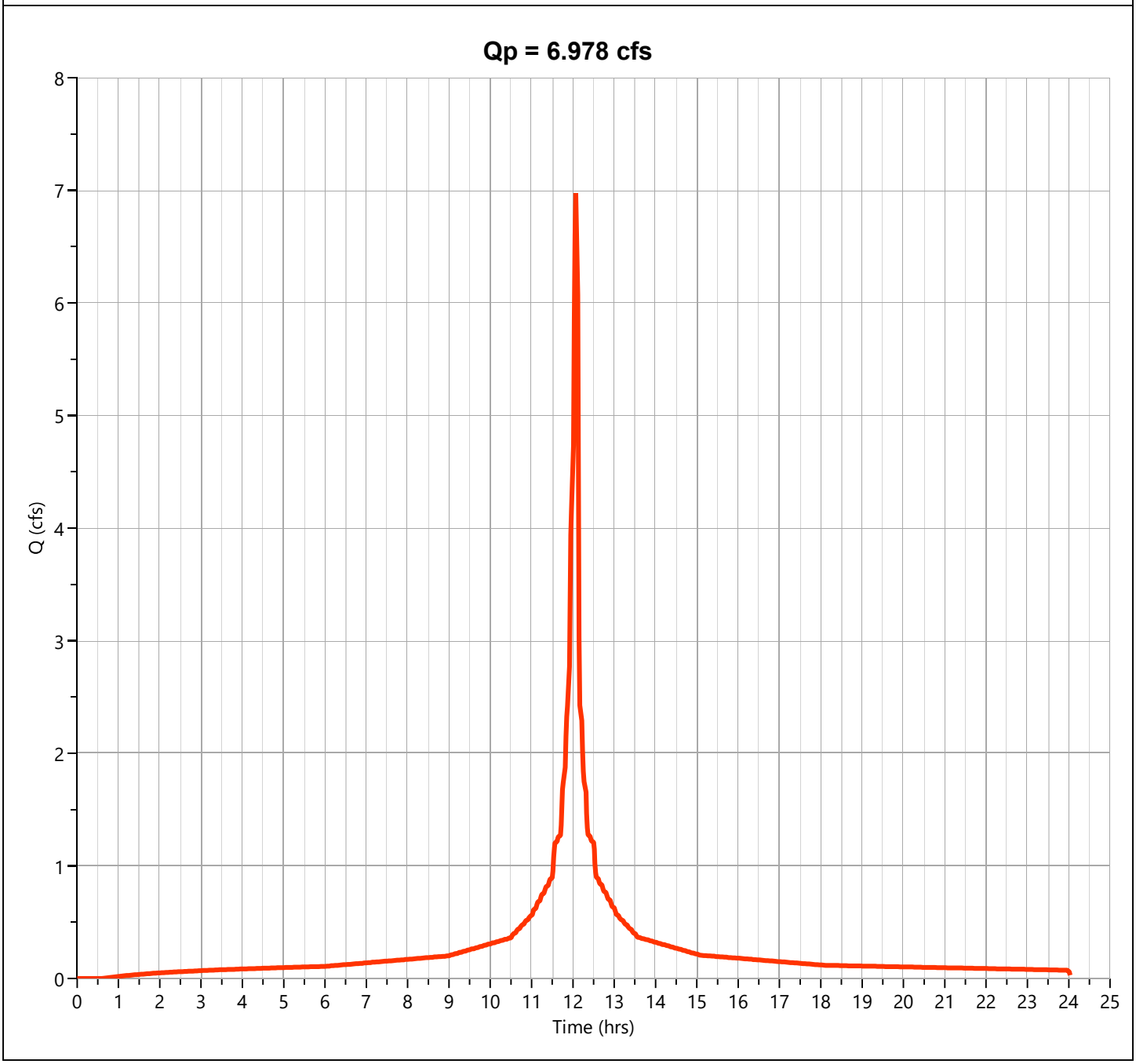
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03-19-2026

PR WSDH A2 Imperv

Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 6.978 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 23,214 cuft
Drainage Area	= 1.15 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 3.0 min
Total Rainfall	= 6.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

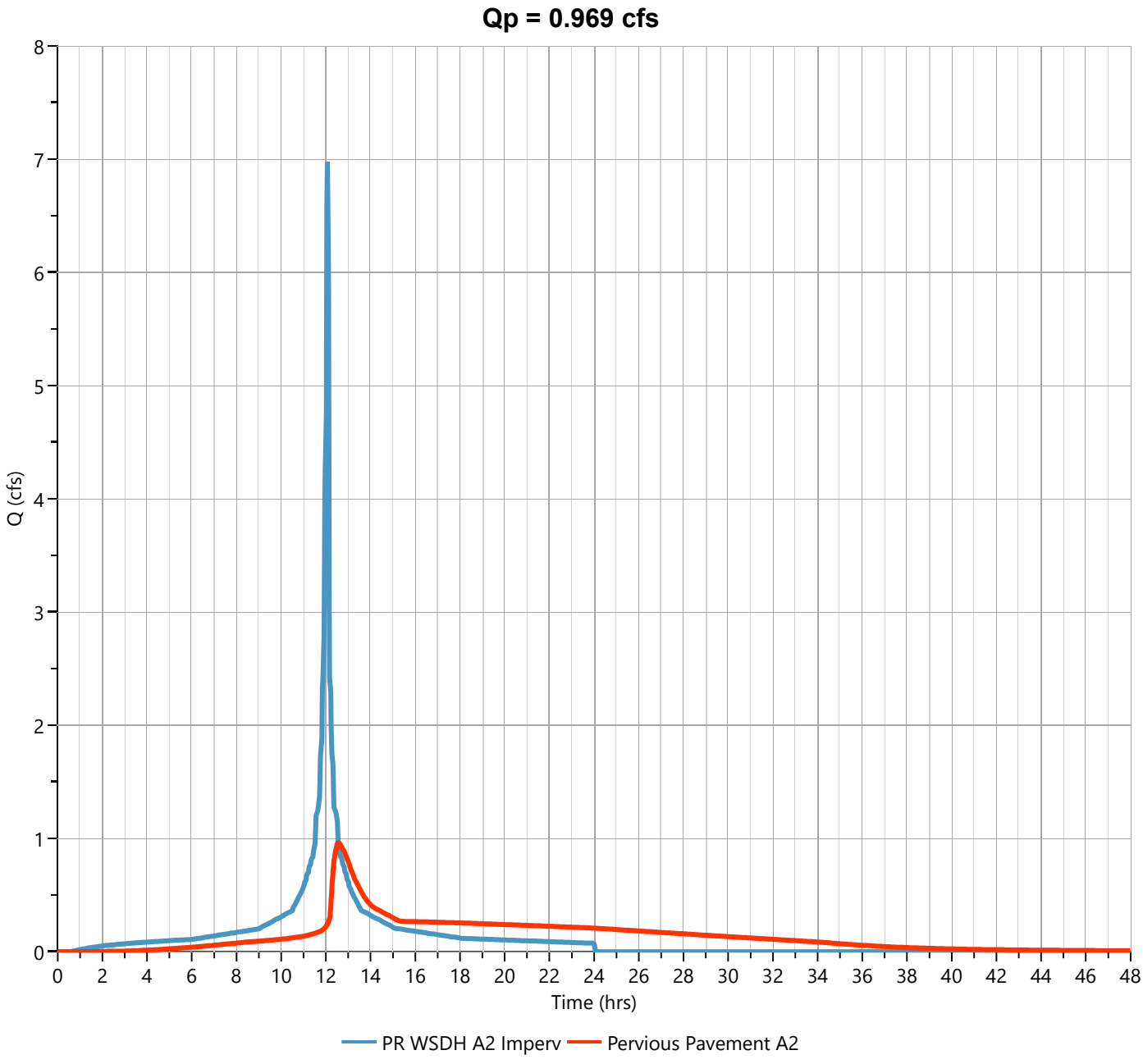
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

Pervious Pavement A2 — Hyd. No. 2

Hydrograph Type	= Pond Route	Peak Flow	= 0.969 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.55 hrs
Time Interval	= 1 min	Hydrograph Volume	= 22,777 cuft
Inflow Hydrograph	= 1 - PR WSDH A2 Imperv	Max. Elevation	= 2.16 ft
Pond Name	= Pervious Pavement A2	Max. Storage	= 12,156 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 0.00 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 7.54 hrs



Hydrograph Report

Hydrology Studio v 3.0.0.43

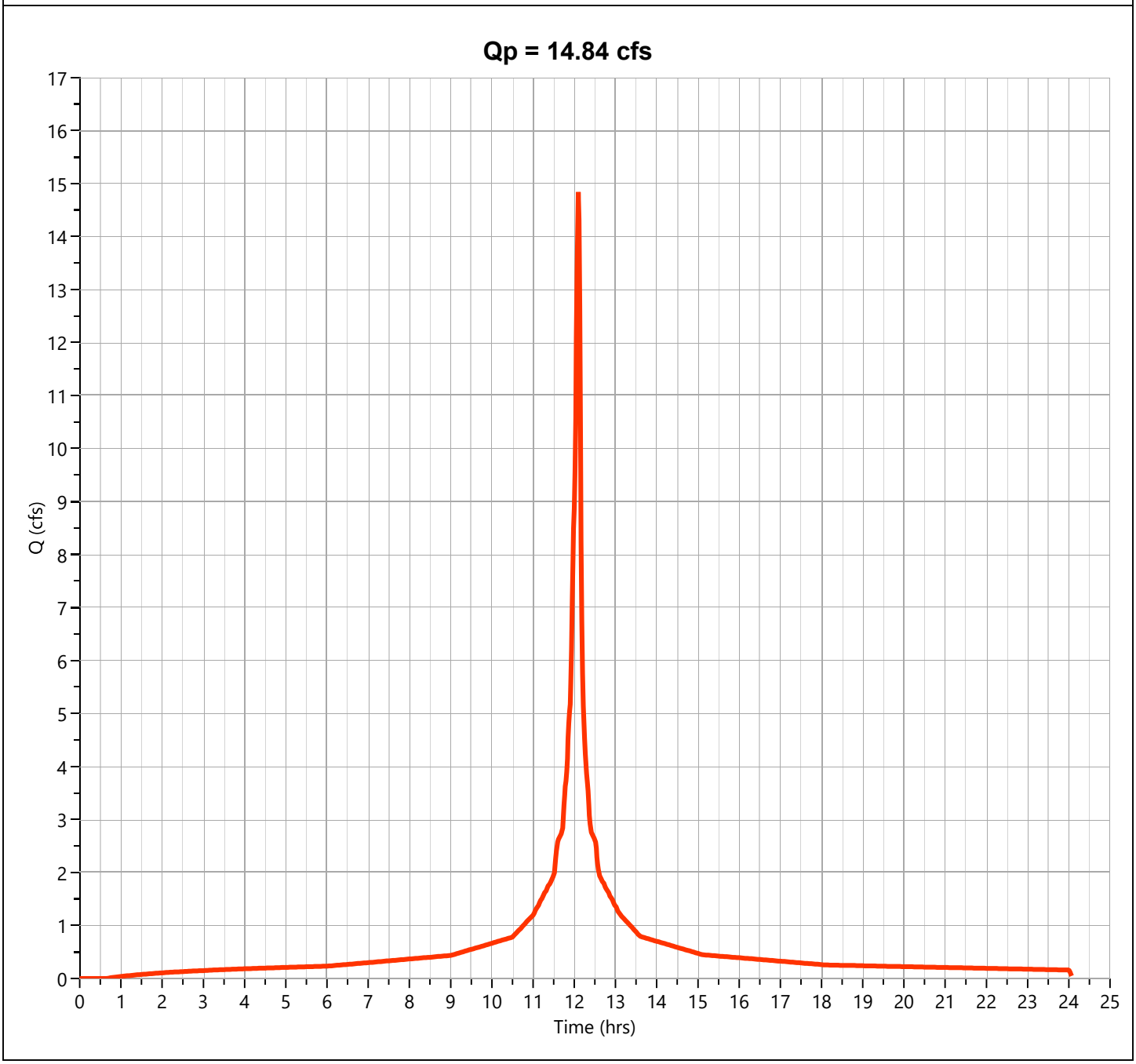
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

03-19-2026

PR WSHD A3 Imperv

Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 14.84 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 50,385 cuft
Drainage Area	= 2.34 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 4.0 min
Total Rainfall	= 6.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

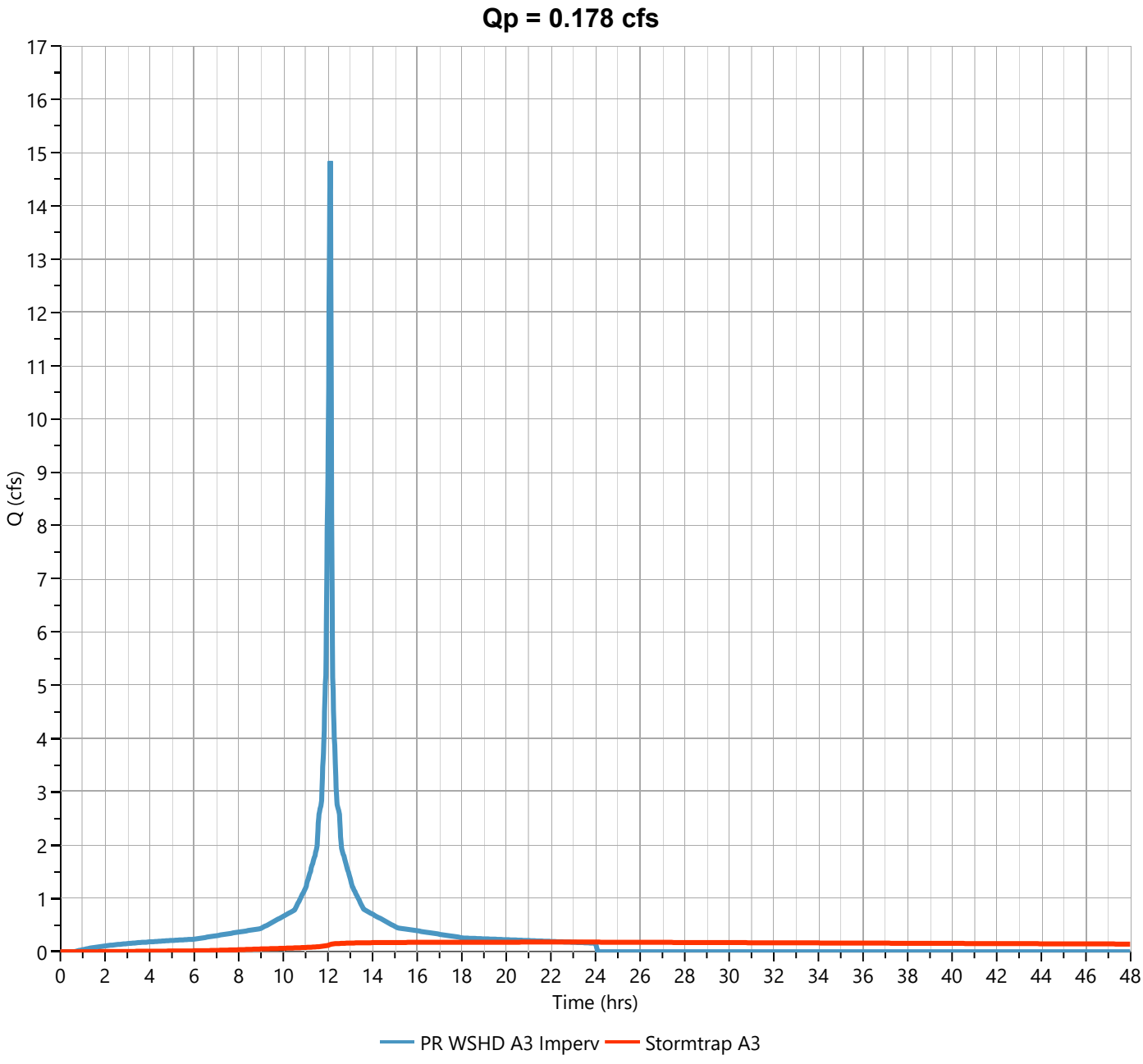
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

Stormtrap A3 — Hyd. No. 5

Hydrograph Type	= Pond Route	Peak Flow	= 0.178 cfs
Storm Frequency	= 10-yr	Time to Peak	= 22.87 hrs
Time Interval	= 1 min	Hydrograph Volume	= 22,472 cuft
Inflow Hydrograph	= 4 - PR WSHD A3 Imperv	Max. Elevation	= 2.96 ft
Pond Name	= Stormtrap A3	Max. Storage	= 41,721 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 0.00 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 15.97 hrs



Hydrograph Report

Hydrology Studio v 3.0.0.43

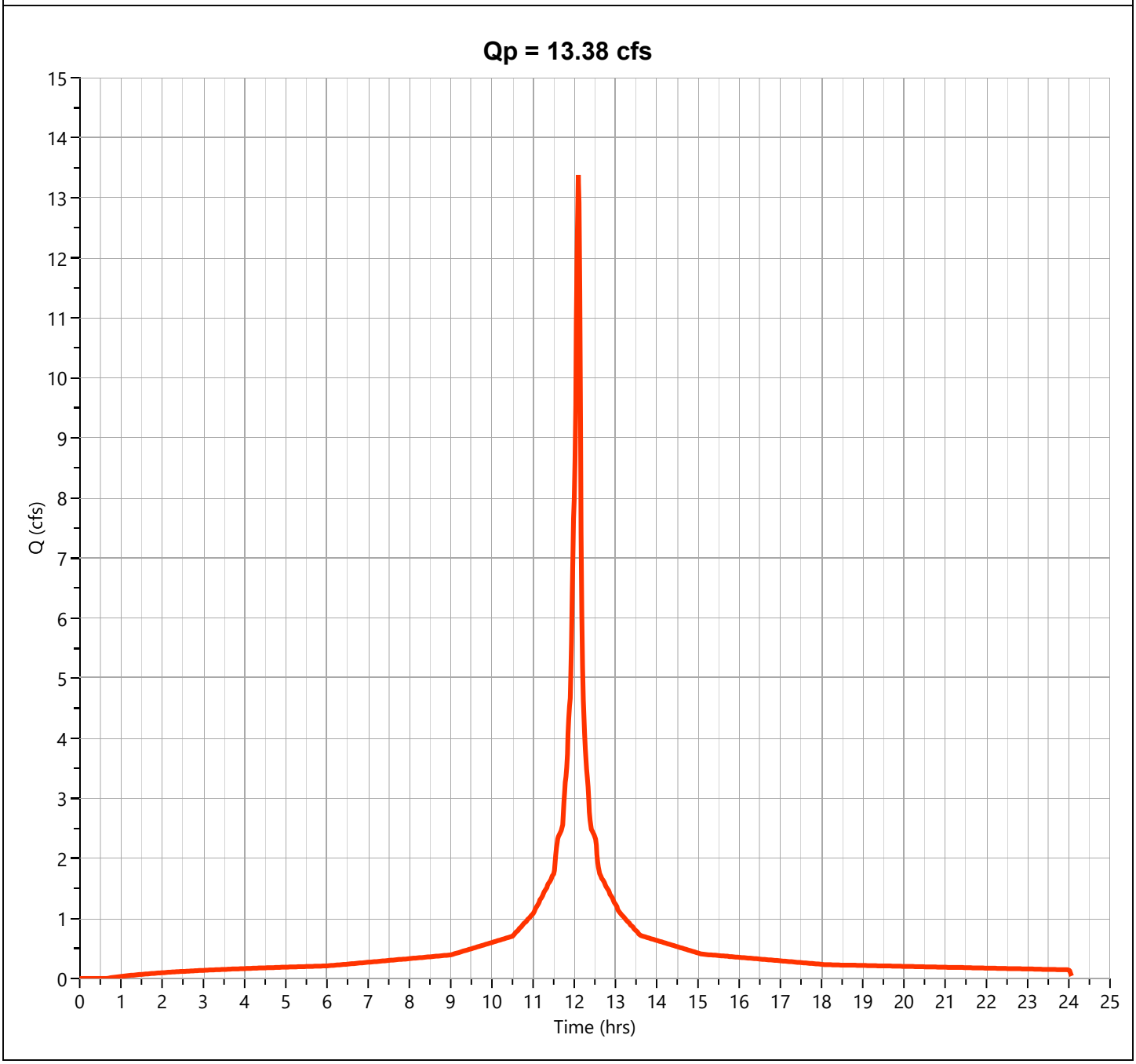
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

03-19-2026

PR WSHD A4 Imperv

Hyd. No. 7

Hydrograph Type	= NRCS Runoff	Peak Flow	= 13.38 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 45,433 cuft
Drainage Area	= 2.11 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 4.0 min
Total Rainfall	= 6.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

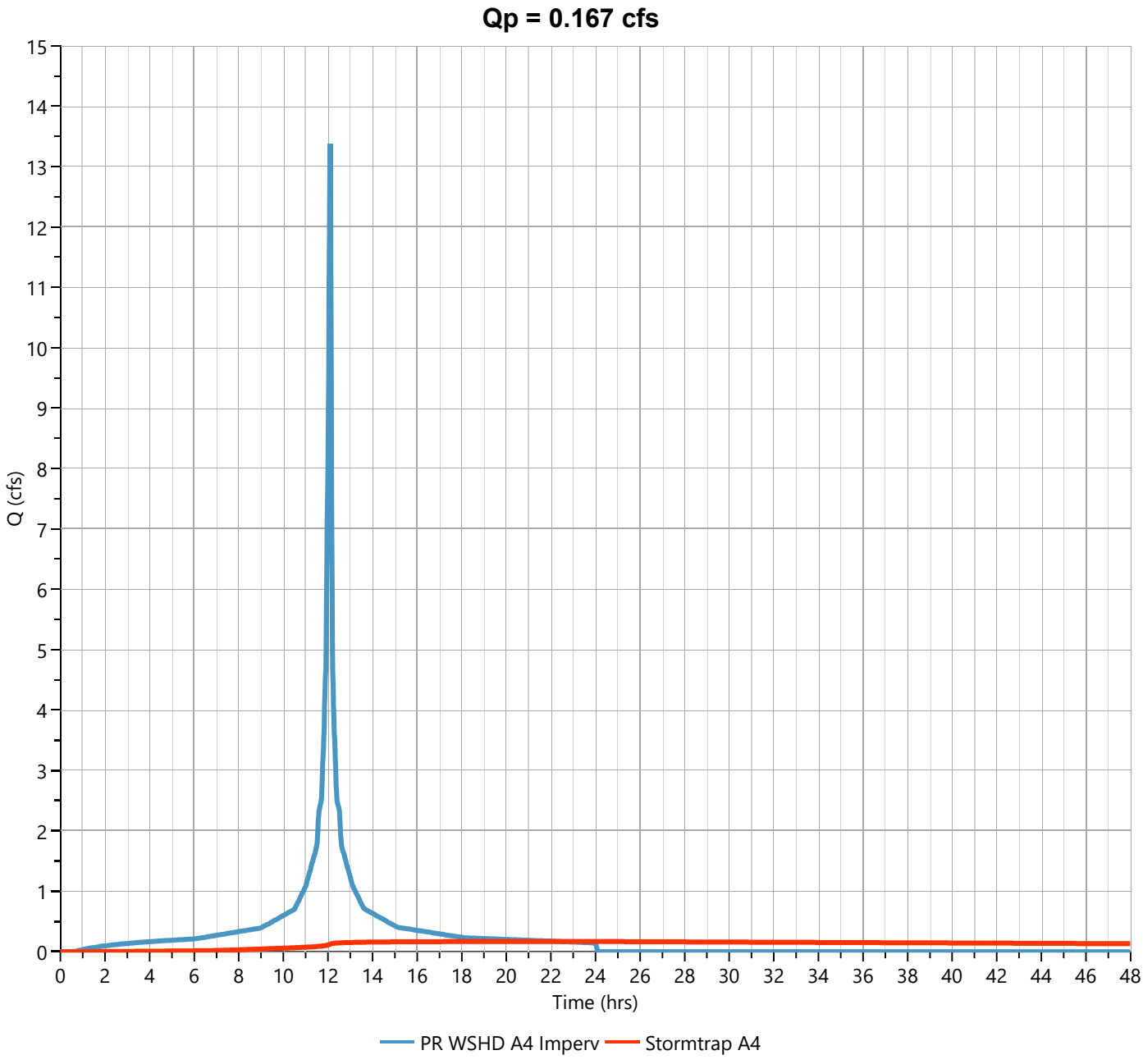
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

Stormtrap A4 — Hyd. No. 8

Hydrograph Type	= Pond Route	Peak Flow	= 0.167 cfs
Storm Frequency	= 10-yr	Time to Peak	= 22.33 hrs
Time Interval	= 1 min	Hydrograph Volume	= 21,007 cuft
Inflow Hydrograph	= 7 - PR WSHD A4 Imperv	Max. Elevation	= 2.81 ft
Pond Name	= Stormtrap A4	Max. Storage	= 37,350 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 0.00 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 15.96 hrs



Hydrograph Report

Hydrology Studio v 3.0.0.43

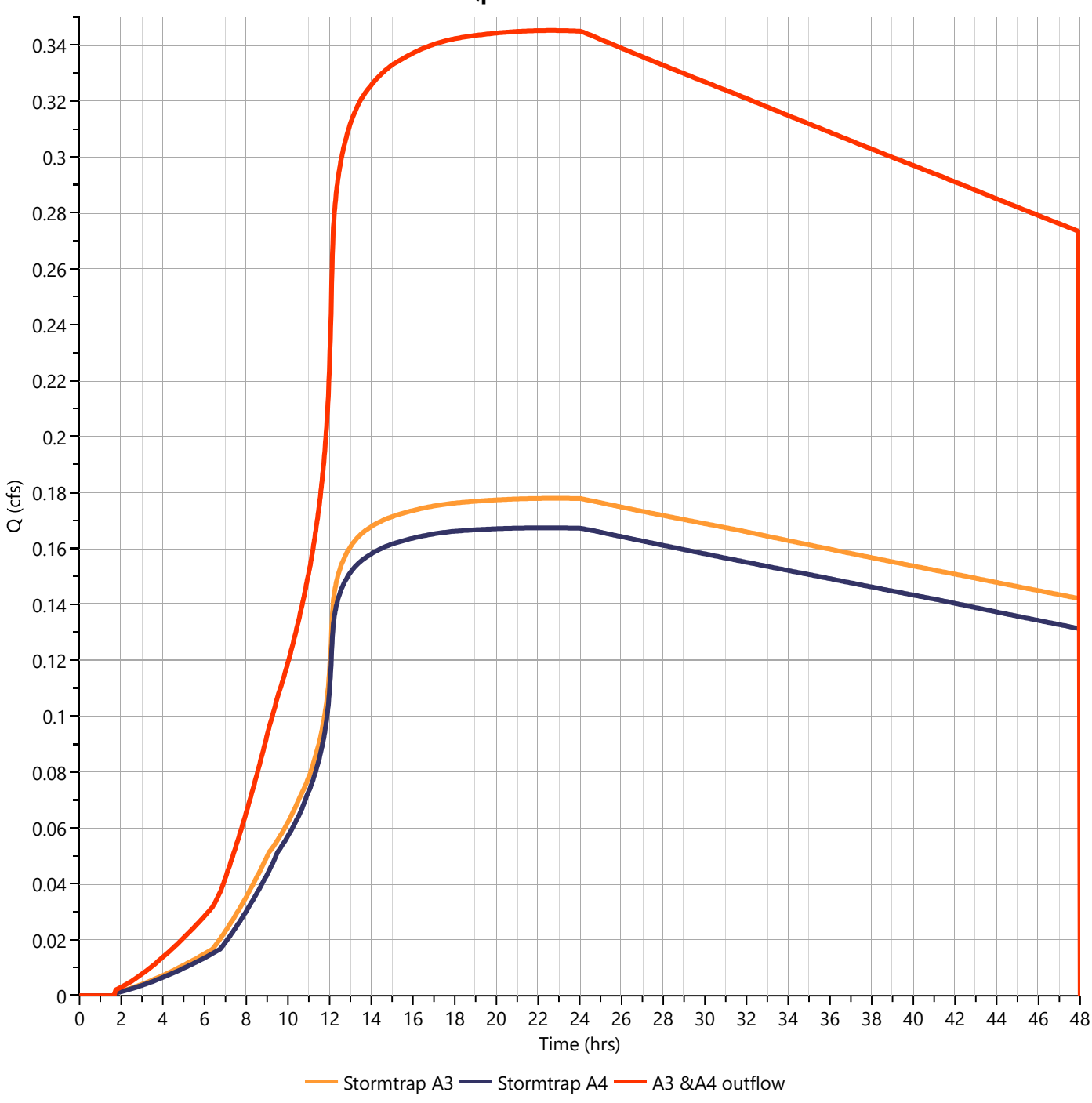
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

03-19-2026

A3 &A4 outflow

Hyd. No. 9

Hydrograph Type	= Junction	Peak Flow	= 0.345 cfs
Storm Frequency	= 10-yr	Time to Peak	= 22.62 hrs
Time Interval	= 1 min	Hydrograph Volume	= 43,478 cuft
Inflow Hydrographs		Total Contrib. Area	= 0.0 ac



Hydrograph Report

Hydrology Studio v 3.0.0.43

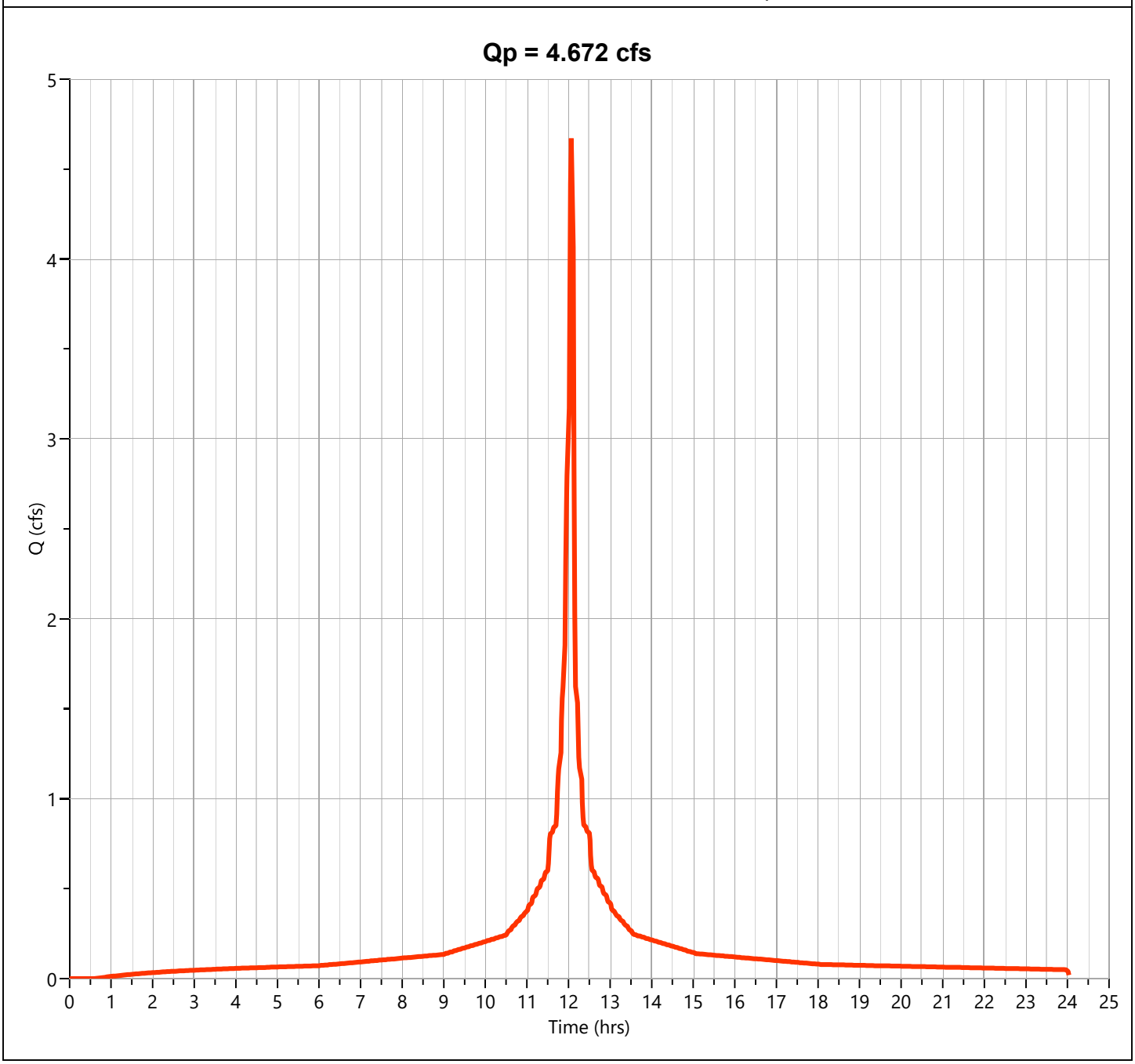
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

03-19-2026

PR WSHD A5 Imperv

Hyd. No. 11

Hydrograph Type	= NRCS Runoff	Peak Flow	= 4.672 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 15,543 cuft
Drainage Area	= 0.77 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 6.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

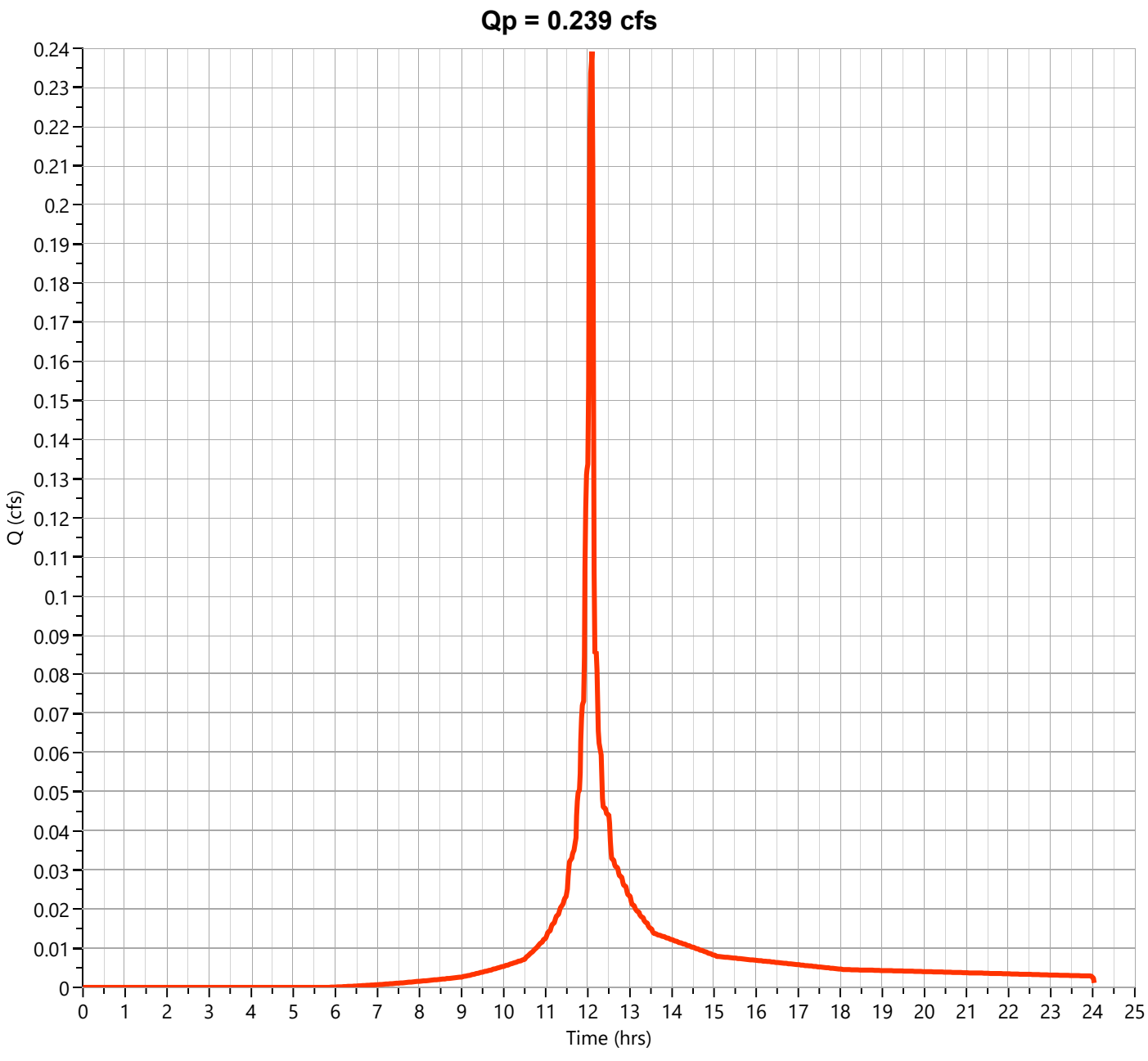
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03-19-2026

PR WSHD A5 Perv

Hyd. No. 12

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.239 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 670 cuft
Drainage Area	= 0.05 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 6.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

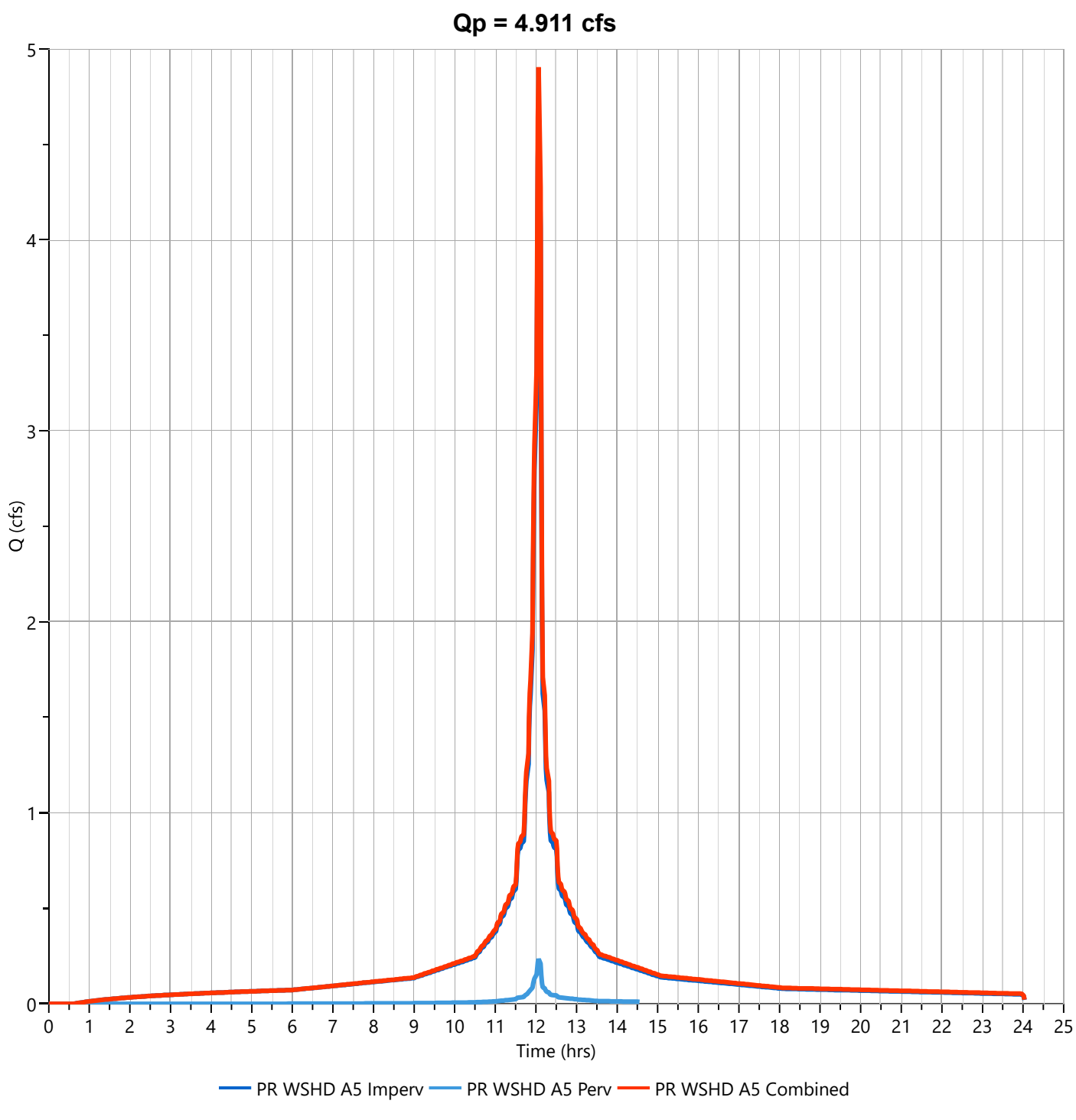
Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

PR WSHD A5 Combined

Hyd. No. 13

Hydrograph Type	= Junction	Peak Flow	= 4.911 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Hydrograph Volume	= 16,213 cuft
Inflow Hydrographs	= 11, 12	Total Contrib. Area	= 0.82 ac



Hydrograph Report

Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

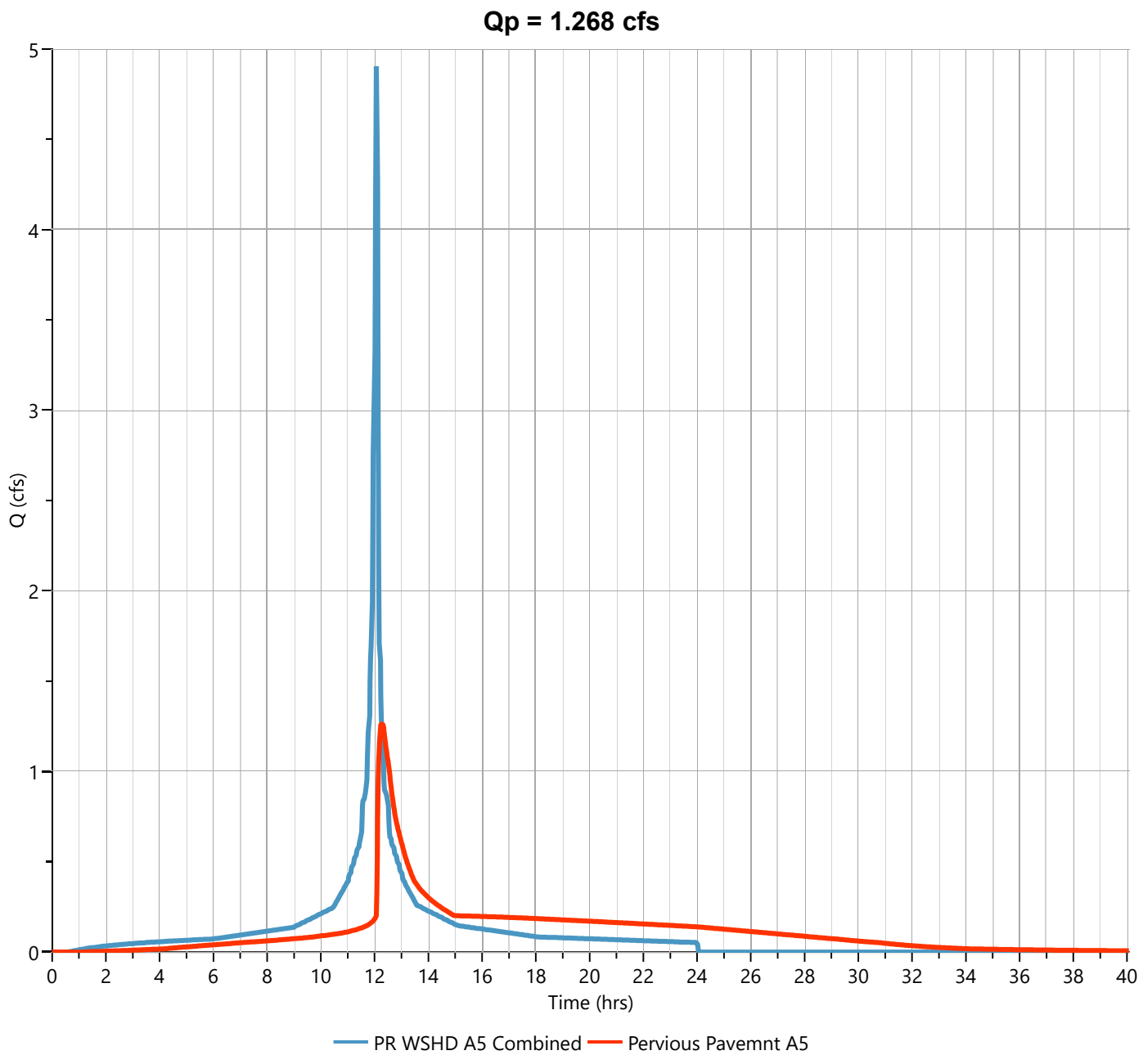
Pervious Pavemnt A5

Hyd. No. 14

Hydrograph Type	= Pond Route	Peak Flow	= 1.268 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.25 hrs
Time Interval	= 1 min	Hydrograph Volume	= 16,179 cuft
Inflow Hydrograph	= 13 - PR WSHD A5 Combined	Max. Elevation	= 5.32 ft
Pond Name	= Pervious Pavement A5	Max. Storage	= 7,125 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 0.00 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 4.88 hrs



Hydrograph Report

Hydrology Studio v 3.0.0.43

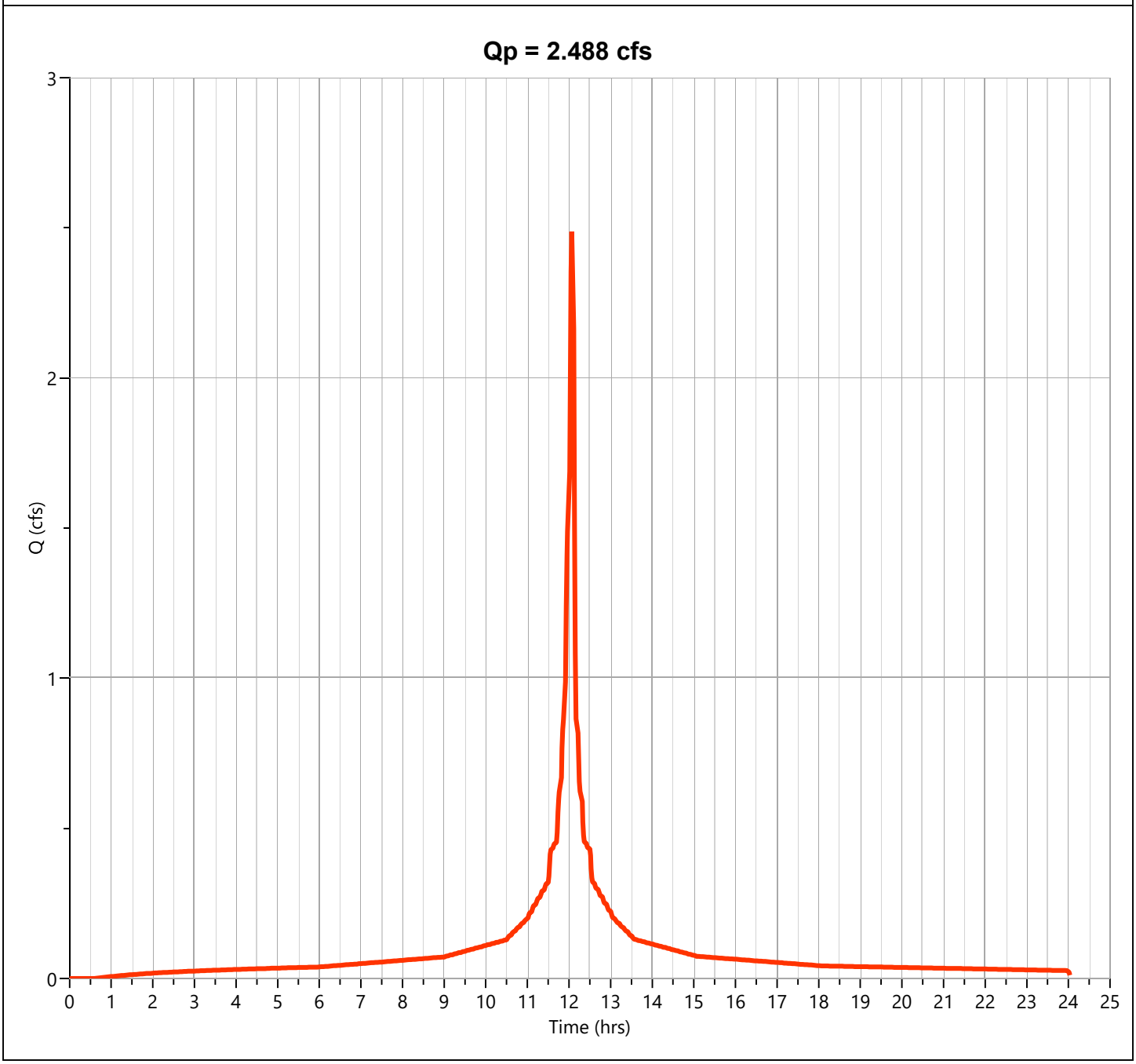
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03-19-2026

PR WSHD A6 Imperv

Hyd. No. 16

Hydrograph Type	= NRCS Runoff	Peak Flow	= 2.488 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 8,276 cuft
Drainage Area	= 0.41 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 6.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

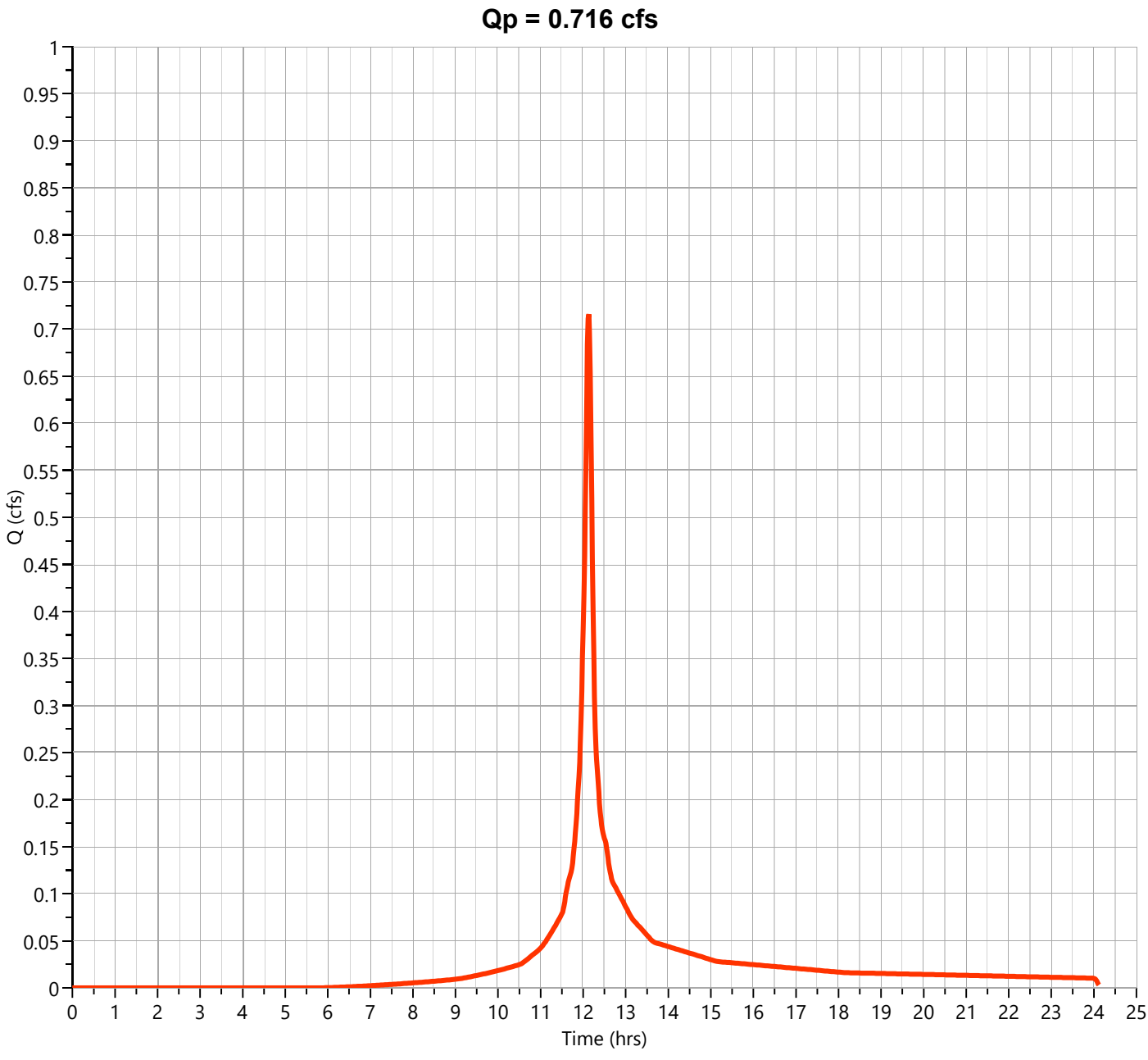
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03-19-2026

PR WSHD A6 Perv

Hyd. No. 17

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.716 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.13 hrs
Time Interval	= 1 min	Runoff Volume	= 2,368 cuft
Drainage Area	= 0.17 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 8.0 min
Total Rainfall	= 6.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

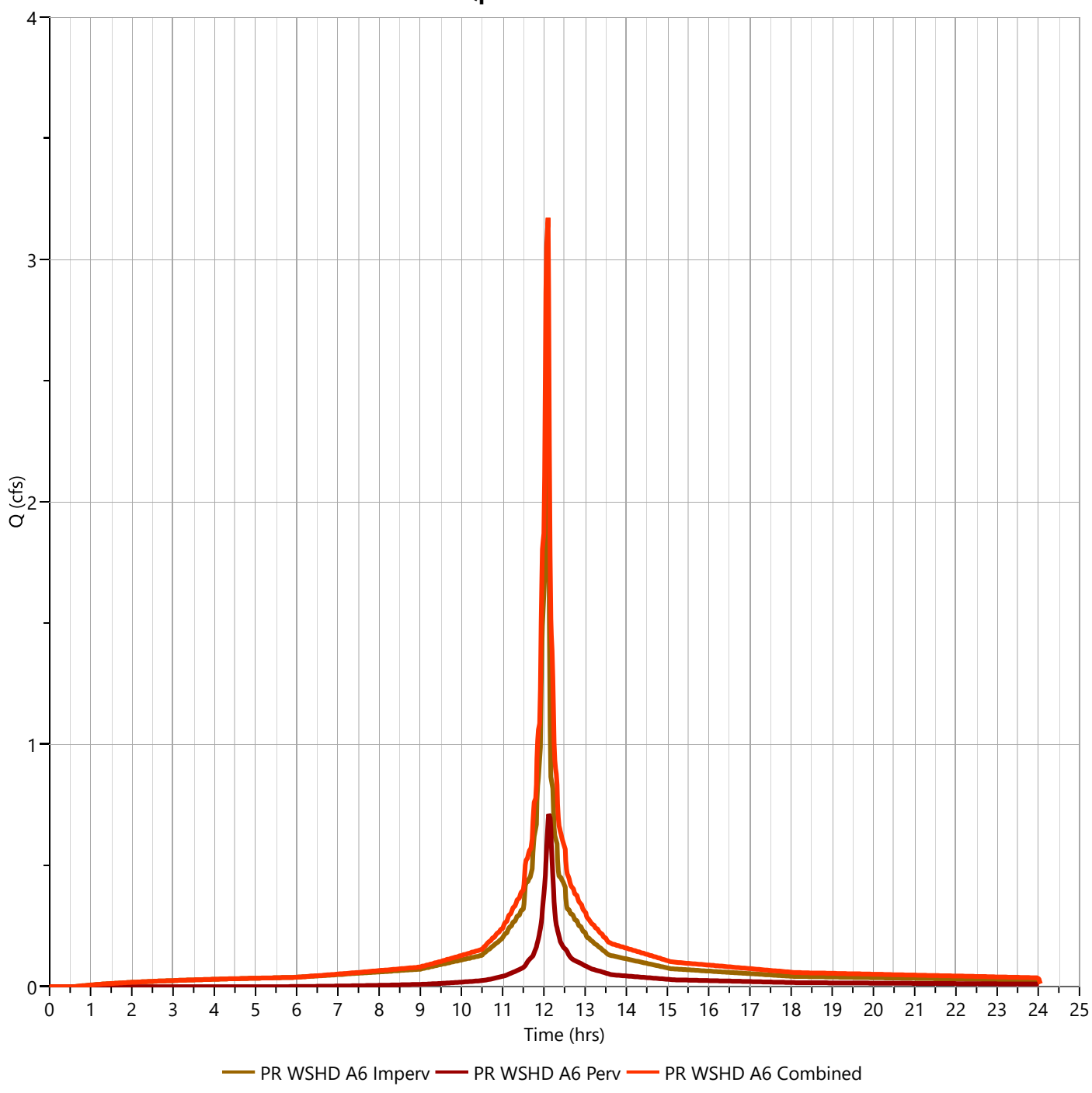
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03-19-2026

PR WSHD A6 Combined

Hyd. No. 18

Hydrograph Type	= Junction	Peak Flow	= 3.171 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Hydrograph Volume	= 10,644 cuft
Inflow Hydrographs	= 16, 17	Total Contrib. Area	= 0.58 ac



Hydrograph Report

Hydrology Studio v 3.0.0.43

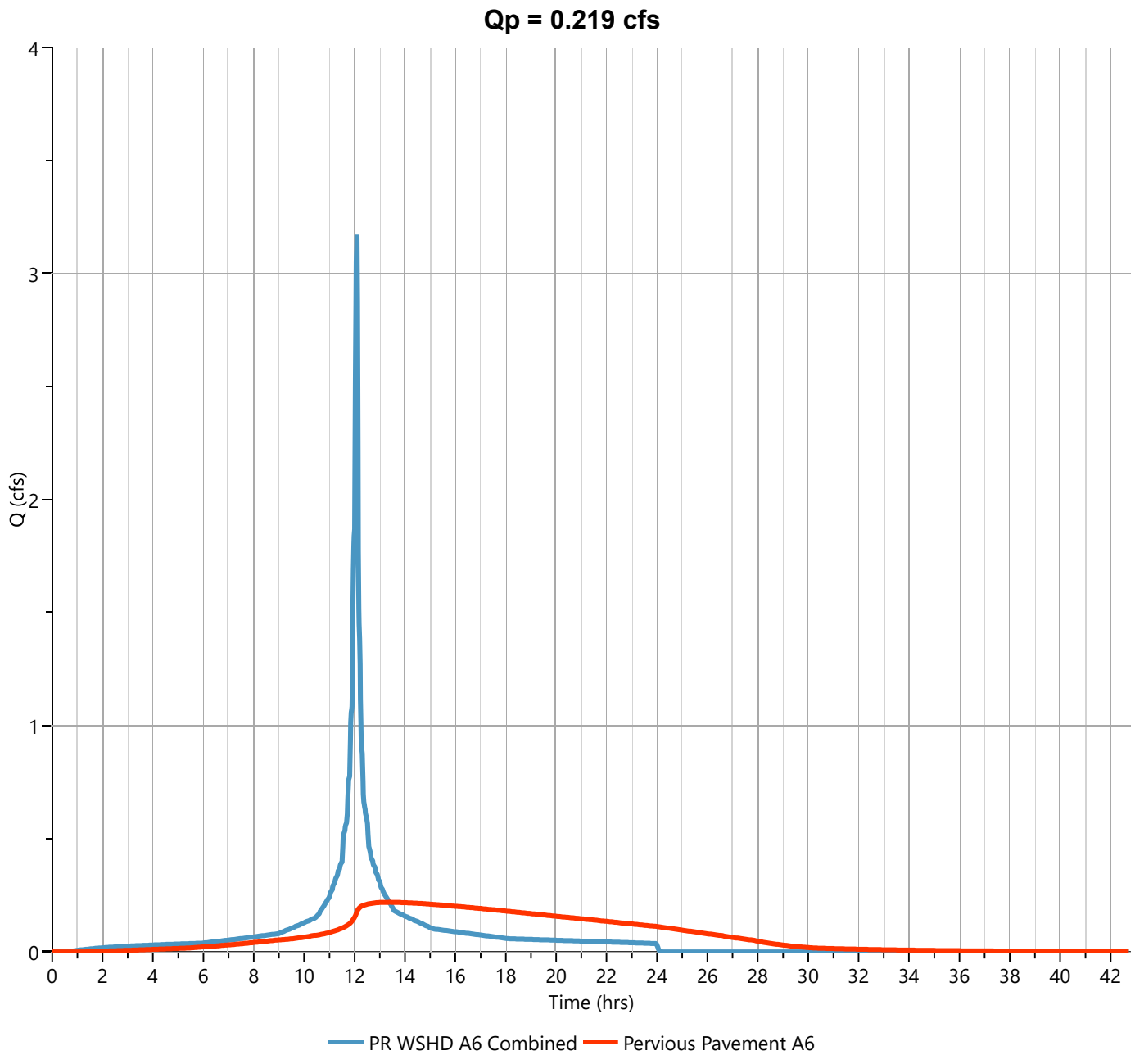
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03-19-2026

Pervious Pavement A6 — Hyd. No. 19

Hydrograph Type	= Pond Route	Peak Flow	= 0.219 cfs
Storm Frequency	= 10-yr	Time to Peak	= 13.40 hrs
Time Interval	= 1 min	Hydrograph Volume	= 10,627 cuft
Inflow Hydrograph	= 18 - PR WSHD A6 Combined	Max. Elevation	= 4.48 ft
Pond Name	= Pervious Pavement A6	Max. Storage	= 5,405 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 0.00 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 5.14 hrs



Hydrograph Report

Hydrology Studio v 3.0.0.43

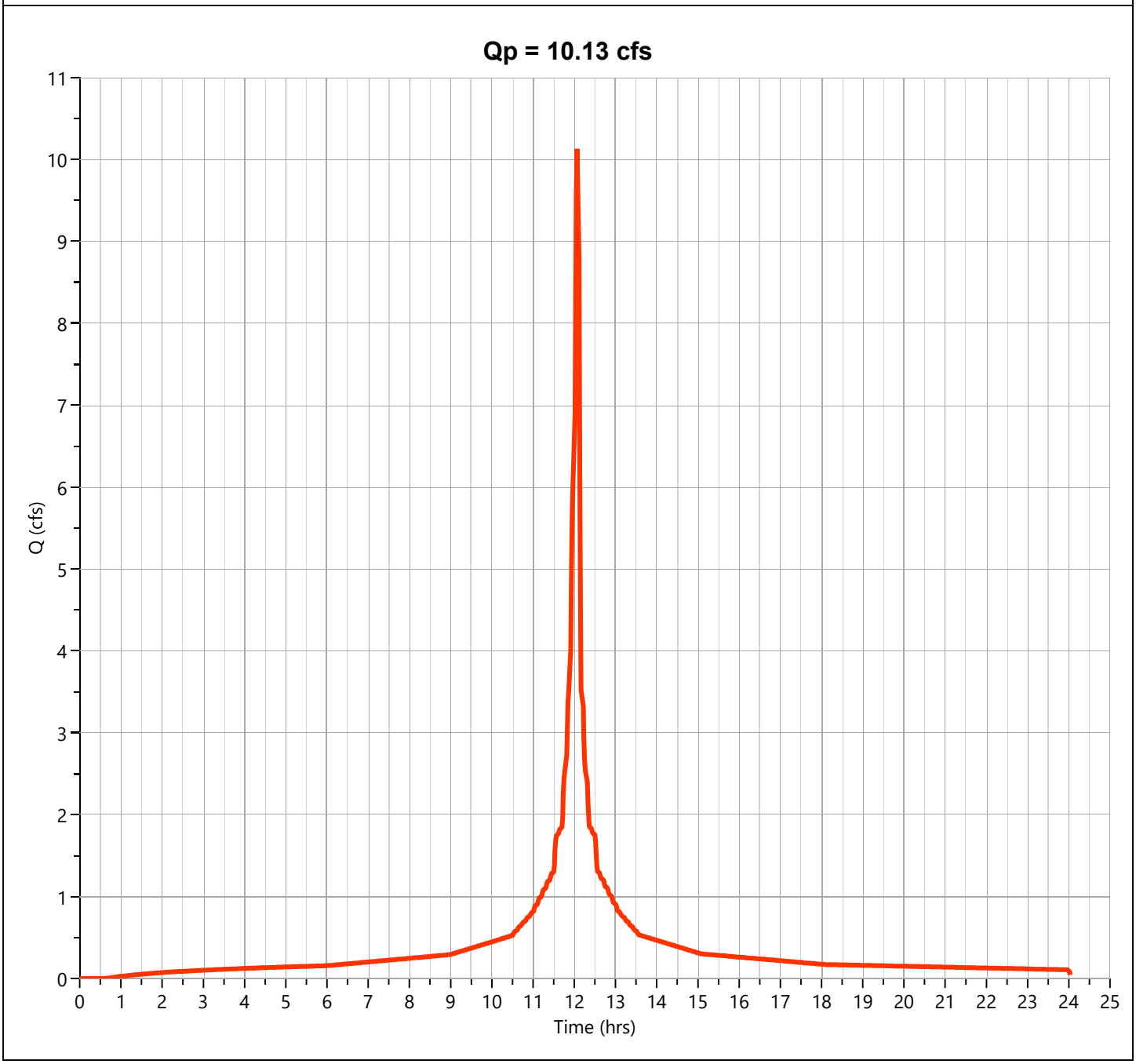
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03-19-2026

PR WSHD A7 Imperv

Hyd. No. 21

Hydrograph Type	= NRCS Runoff	Peak Flow	= 10.13 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 33,711 cuft
Drainage Area	= 1.67 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 6.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

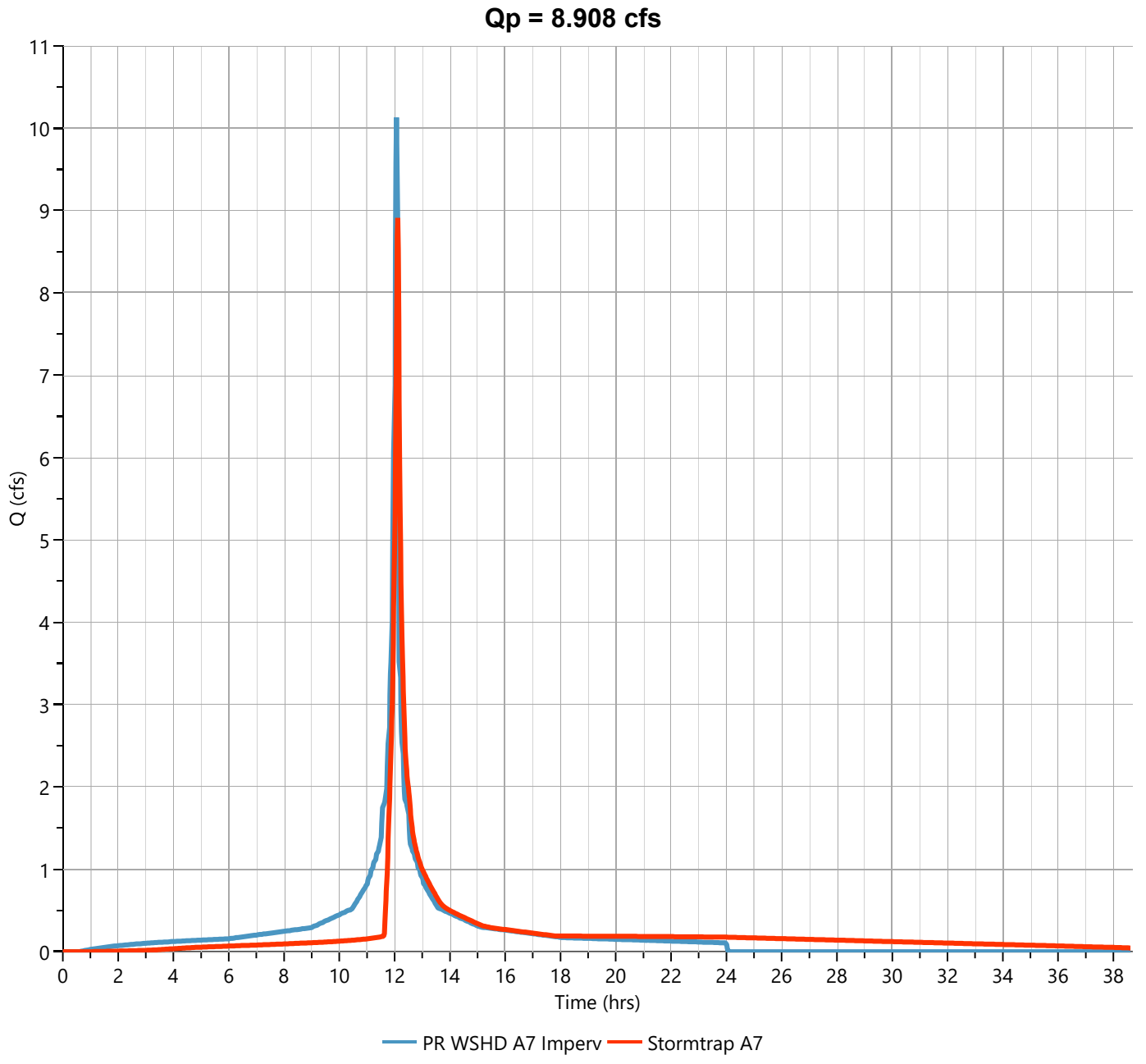
Stormtrap A7

Hyd. No. 22

Hydrograph Type	= Pond Route	Peak Flow	= 8.908 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.12 hrs
Time Interval	= 1 min	Hydrograph Volume	= 33,443 cuft
Inflow Hydrograph	= 21 - PR WSHD A7 Imperv	Max. Elevation	= 4.98 ft
Pond Name	= Stormtrap A7	Max. Storage	= 10,154 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 0.00 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 3.53 hrs



Hydrograph Report

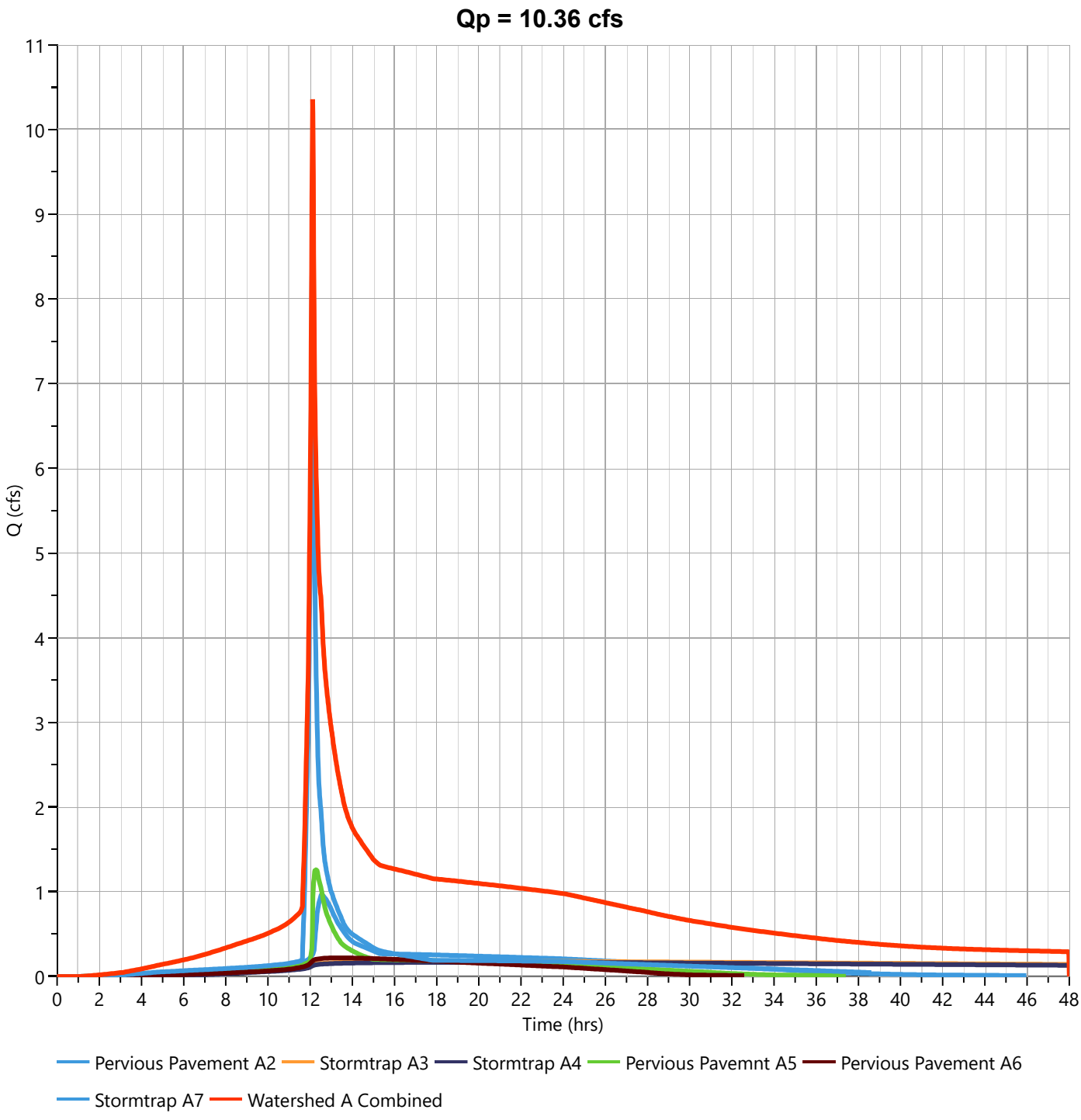
Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

Watershed A Combined

Hyd. No. 24

Hydrograph Type	= Junction	Peak Flow	= 10.36 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.12 hrs
Time Interval	= 1 min	Hydrograph Volume	= 126,504 cuft
Inflow Hydrographs		Total Contrib. Area	= 0.0 ac



Hydrograph Report

Hydrology Studio v 3.0.0.43

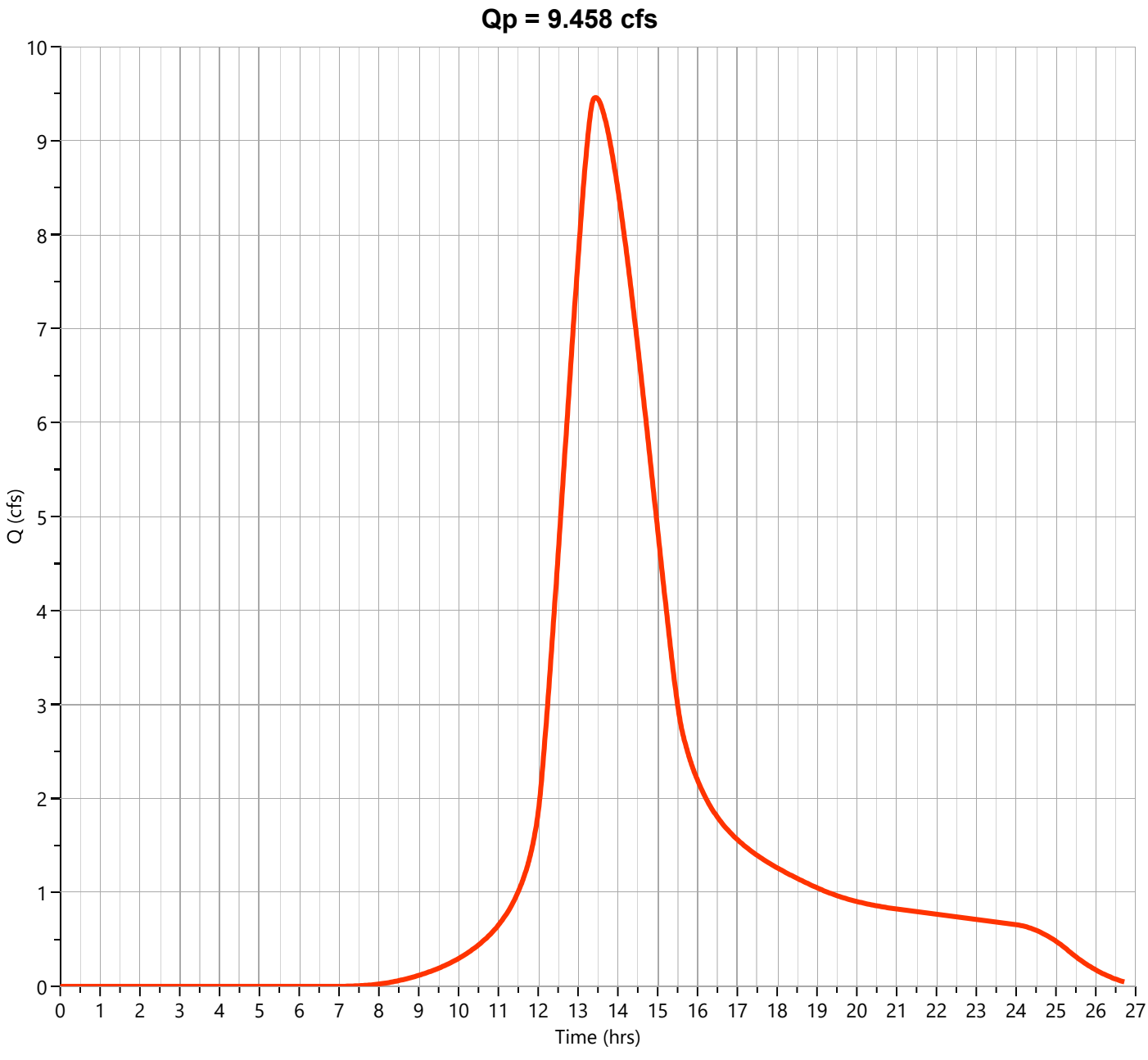
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03-19-2026

PR WSHD B Perv

Hyd. No. 26

Hydrograph Type	= NRCS Runoff	Peak Flow	= 9.458 cfs
Storm Frequency	= 10-yr	Time to Peak	= 13.43 hrs
Time Interval	= 1 min	Runoff Volume	= 125,308 cuft
Drainage Area	= 9.5 ac	Curve Number	= 77.00
Tc Method	= User	Time of Conc. (Tc)	= 134.0 min
Total Rainfall	= 6.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

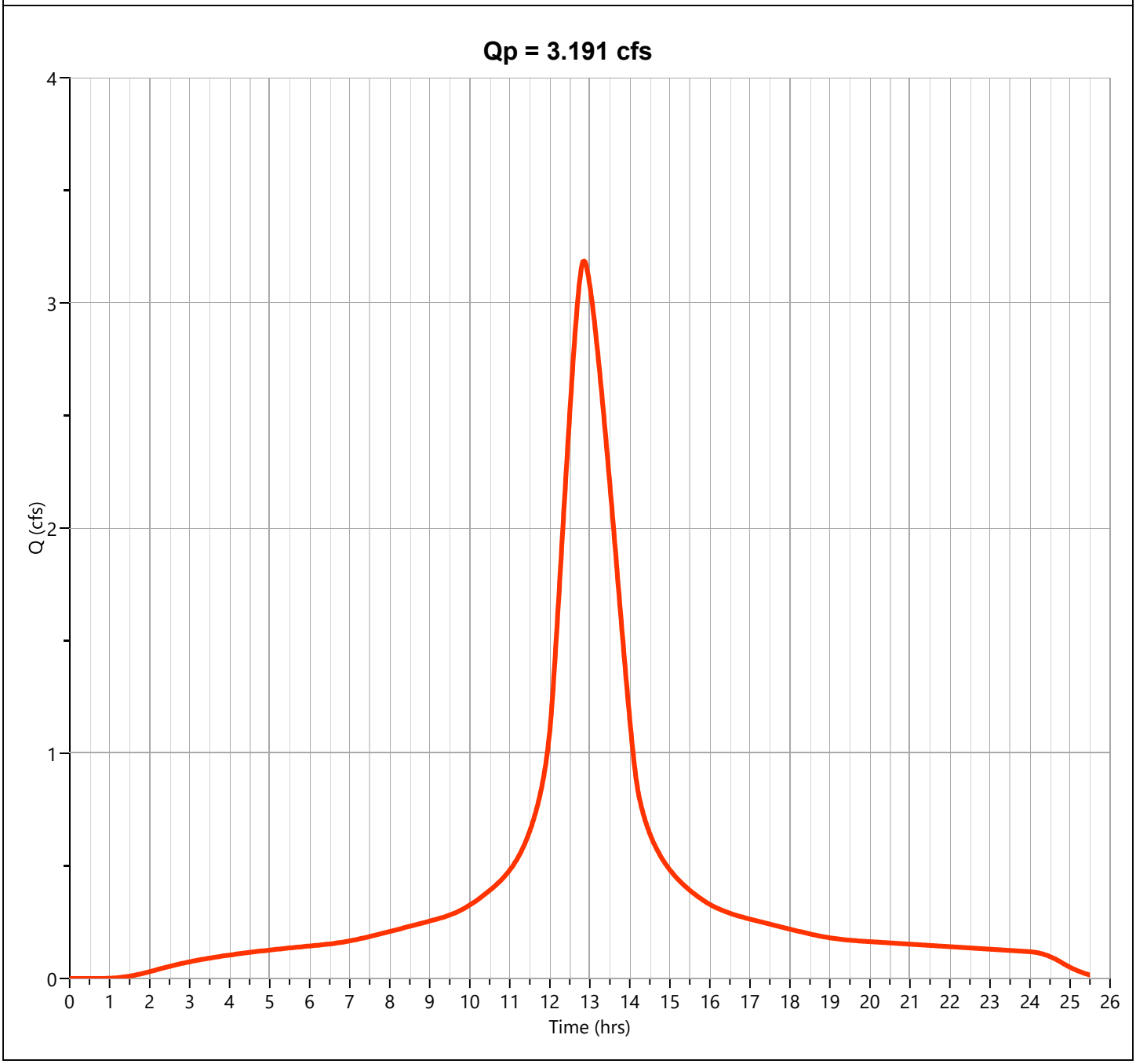
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03-19-2026

PR WSHD B Imperv

Hyd. No. 27

Hydrograph Type	= NRCS Runoff	Peak Flow	= 3.191 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.85 hrs
Time Interval	= 1 min	Runoff Volume	= 34,451 cuft
Drainage Area	= 1.6 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 81.0 min
Total Rainfall	= 6.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

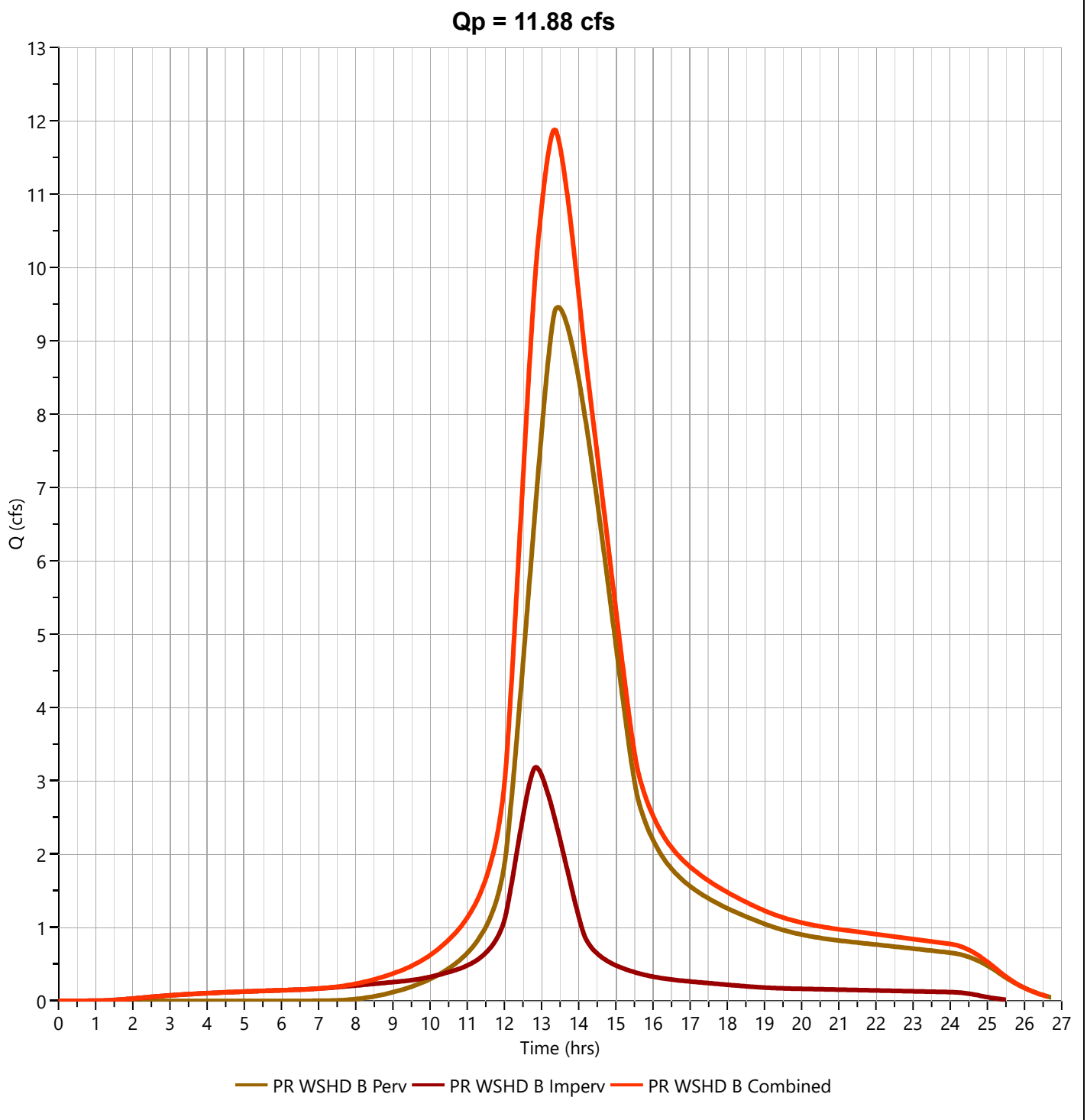
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File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

PR WSHD B Combined

Hyd. No. 28

Hydrograph Type	= Junction	Peak Flow	= 11.88 cfs
Storm Frequency	= 10-yr	Time to Peak	= 13.35 hrs
Time Interval	= 1 min	Hydrograph Volume	= 159,759 cuft
Inflow Hydrographs	= 26, 27	Total Contrib. Area	= 11.1 ac



Hydrograph Report

Hydrology Studio v 3.0.0.43

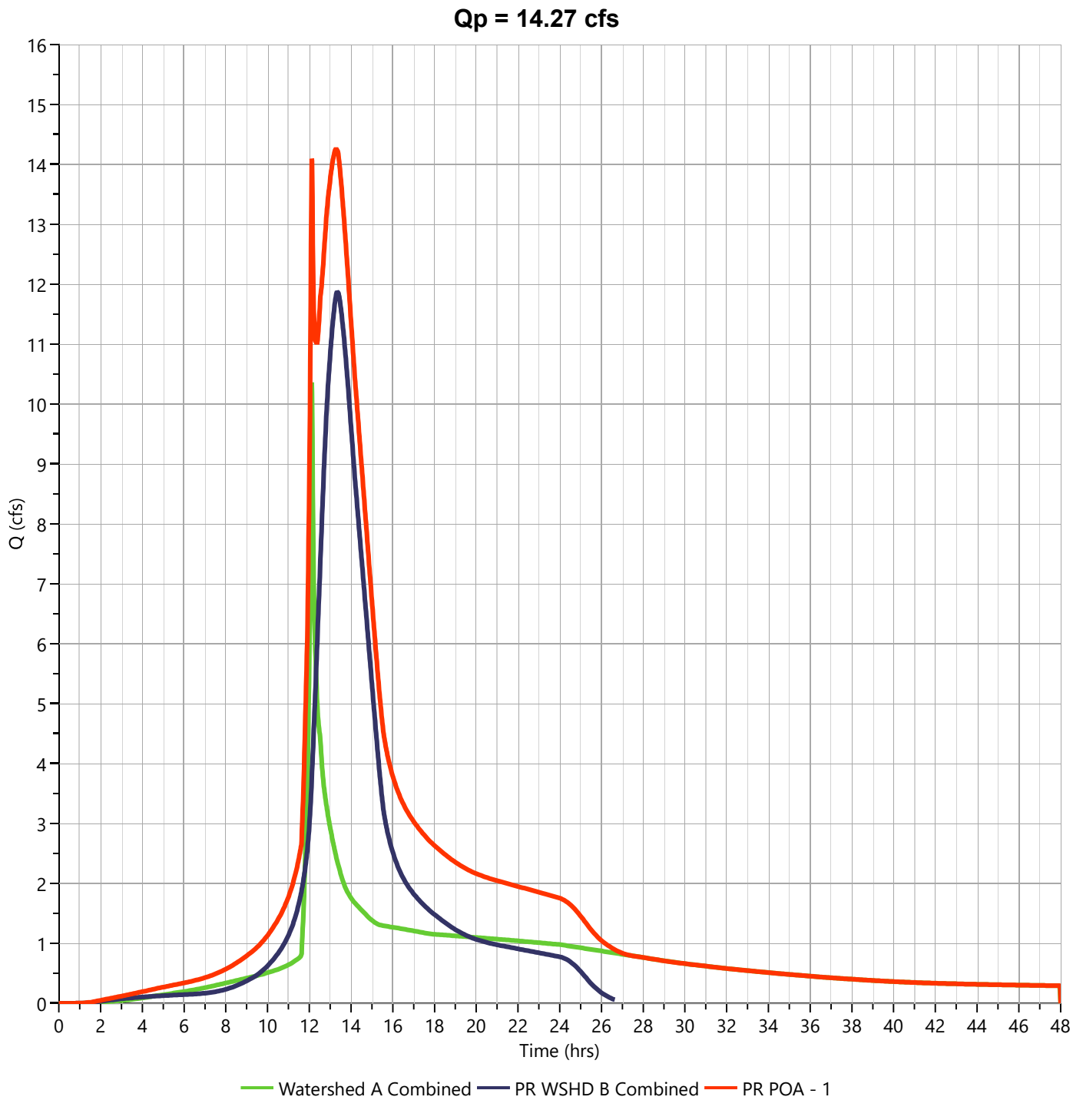
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03-19-2026

PR POA - 1

Hyd. No. 30

Hydrograph Type	= Junction	Peak Flow	= 14.27 cfs
Storm Frequency	= 10-yr	Time to Peak	= 13.30 hrs
Time Interval	= 1 min	Hydrograph Volume	= 286,264 cuft
Inflow Hydrographs		Total Contrib. Area	= 0.0 ac



Hydrograph Report

Hydrology Studio v 3.0.0.43

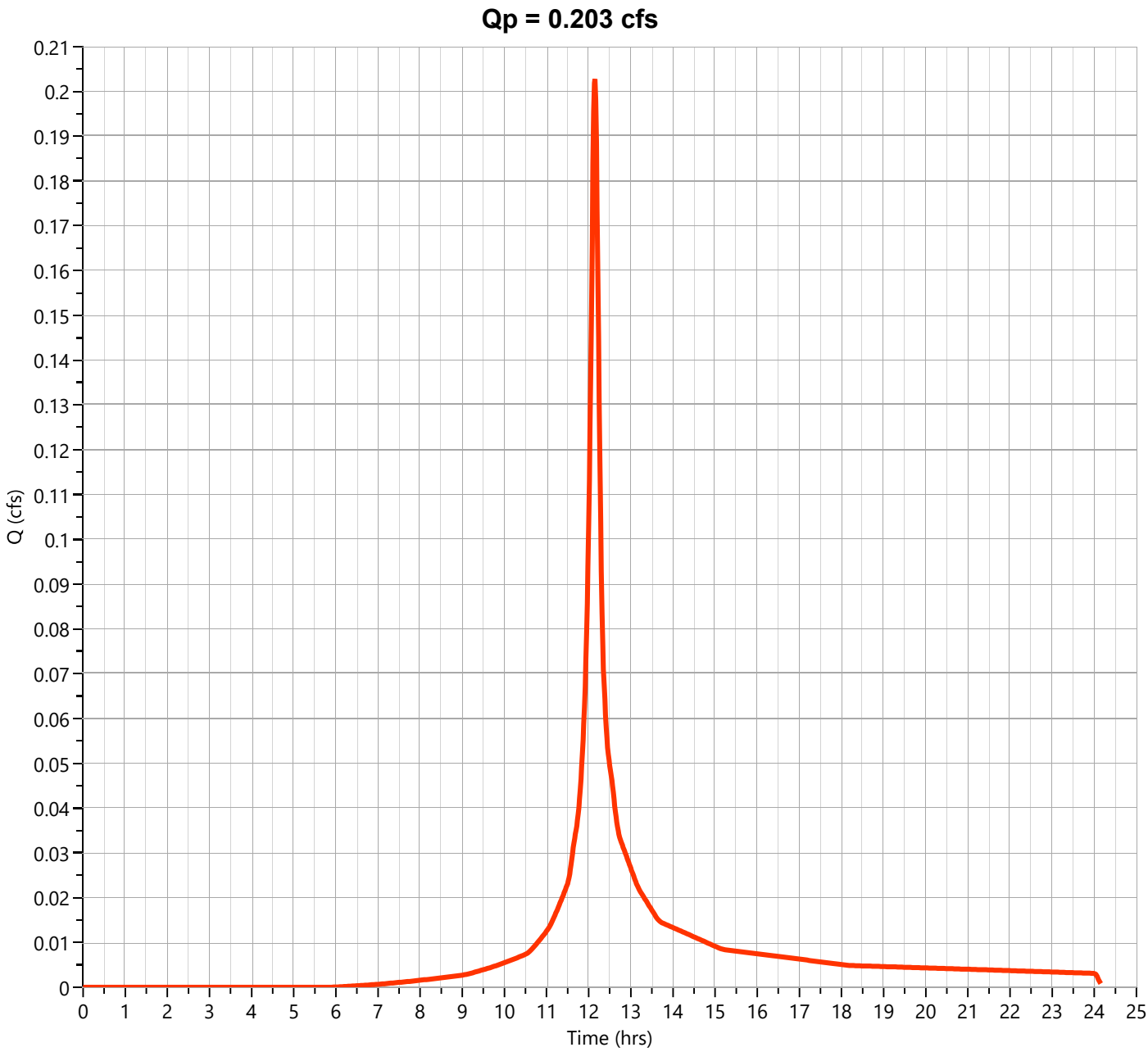
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03-19-2026

PR WSHD C PerPOA-2

Hyd. No. 33

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.203 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.15 hrs
Time Interval	= 1 min	Runoff Volume	= 714 cuft
Drainage Area	= 0.05 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 10.0 min
Total Rainfall	= 6.17 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph 100-yr Summary

Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

03-19-2026

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	NRCS Runoff	PR WSDH A2 Imperv	12.90	12.10	43,592	----		
2	Pond Route	Pervious Pavement A2	3.905	12.23	20,456	1	3.90	24,961
4	NRCS Runoff	PR WSHD A3 Imperv	27.43	12.10	94,614	----		
5	Pond Route	Stormtrap A3	0.904	14.93	30,142	4	4.00	73,838
7	NRCS Runoff	PR WSHD A4 Imperv	24.74	12.10	85,315	----		
8	Pond Route	Stormtrap A4	0.538	17.05	20,855	7	3.92	71,393
9	Junction	A3 &A4 outflow	1.352	16.03	50,998	5, 8		
11	NRCS Runoff	PR WSHD A5 Imperv	8.637	12.10	29,188	----		
12	NRCS Runoff	PR WSHD A5 Perv	0.510	12.10	1,505	----		
13	Junction	PR WSHD A5 Combined	9.147	12.10	30,693	11, 12		
14	Pond Route	Pervious Pavemnt A5	6.404	12.13	26,601	13	6.07	9,883
16	NRCS Runoff	PR WSHD A6 Imperv	4.599	12.10	15,542	----		
17	NRCS Runoff	PR WSHD A6 Perv	1.543	12.13	5,323	----		
18	Junction	PR WSHD A6 Combined	6.086	12.10	20,865	16, 17		
19	Pond Route	Pervious Pavement A6	4.747	12.13	15,412	18	5.45	8,030
21	NRCS Runoff	PR WSHD A7 Imperv	18.73	12.10	63,304	----		
22	Pond Route	Stormtrap A7	12.83	12.13	57,075	21	5.54	12,996
24	Junction	Watershed A Combined	27.39	12.13	170,542	2, 5, 8, 14, 19, 22		
26	NRCS Runoff	PR WSHD B Perv	21.85	13.40	291,637	----		
27	NRCS Runoff	PR WSHD B Imperv	5.904	12.85	64,694	----		
28	Junction	PR WSHD B Combined	26.38	13.35	356,330	26, 27		
30	Junction	PR POA - 1	36.81	12.13	526,872	24, 28		
33	NRCS Runoff	PR WSHD C PerPOA-2	0.438	12.15	1,606	----		

Hydrograph Report

Hydrology Studio v 3.0.0.43

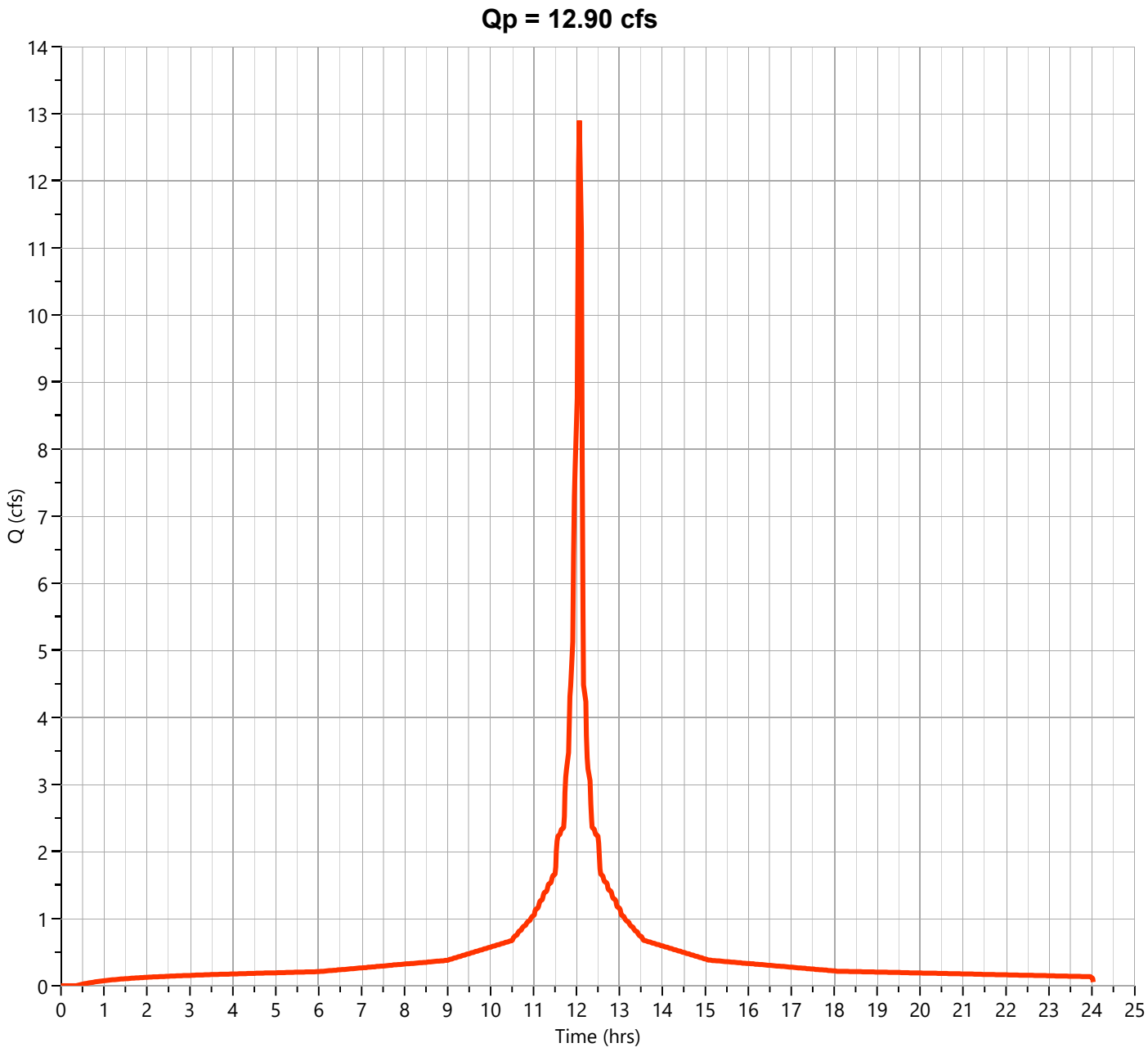
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03-19-2026

PR WSDH A2 Imperv

Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 12.90 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 43,592 cuft
Drainage Area	= 1.15 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 3.0 min
Total Rainfall	= 11.38 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

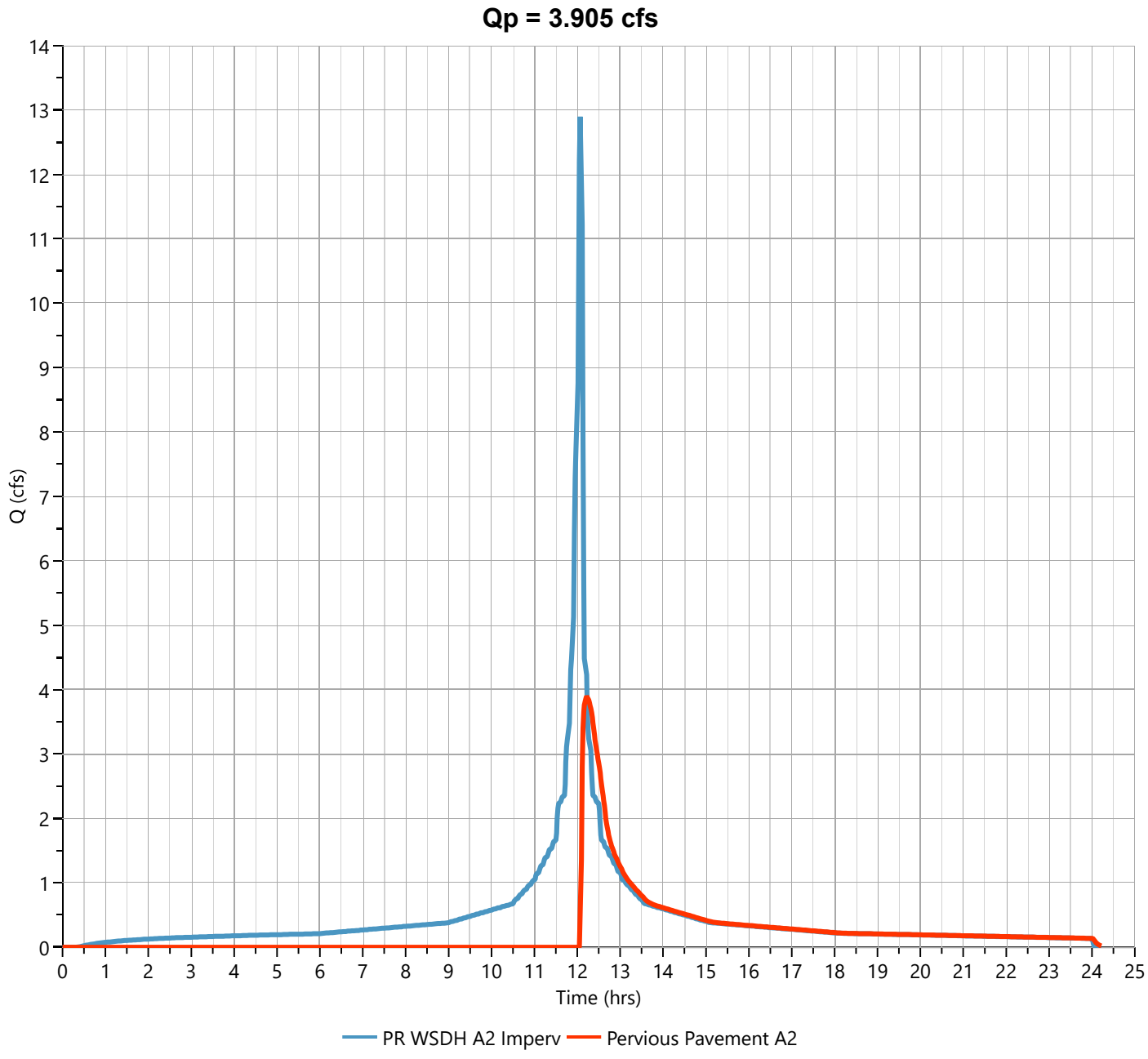
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03-19-2026

Pervious Pavement A2 — Hyd. No. 2

Hydrograph Type	= Pond Route	Peak Flow	= 3.905 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.23 hrs
Time Interval	= 1 min	Hydrograph Volume	= 20,456 cuft
Inflow Hydrograph	= 1 - PR WSDH A2 Imperv	Max. Elevation	= 3.90 ft
Pond Name	= Pervious Pavement A2	Max. Storage	= 24,961 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 3.68 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 2.78 hrs



Hydrograph Report

Hydrology Studio v 3.0.0.43

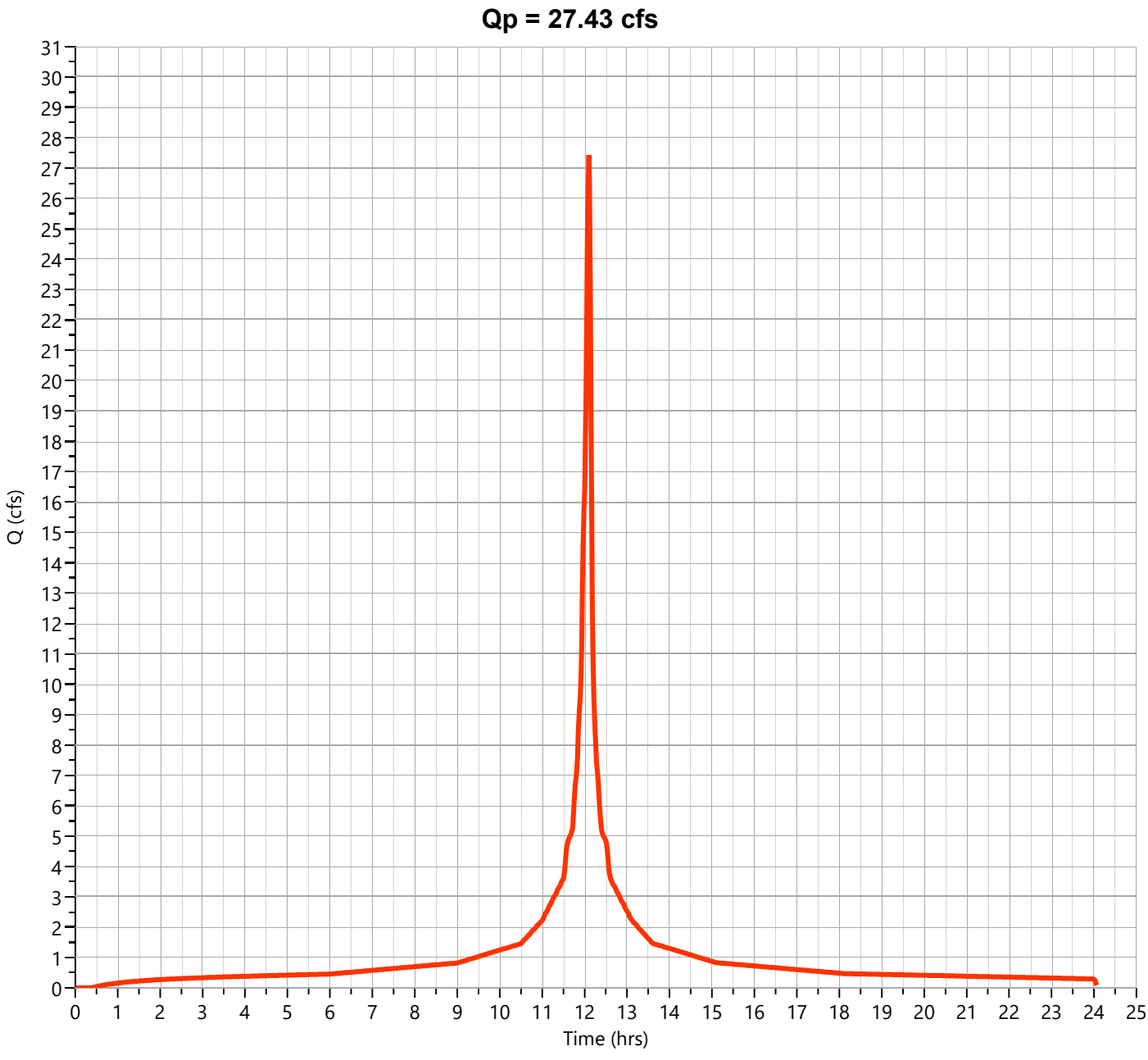
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03-19-2026

PR WSHD A3 Imperv

Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 27.43 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 94,614 cuft
Drainage Area	= 2.34 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 4.0 min
Total Rainfall	= 11.38 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

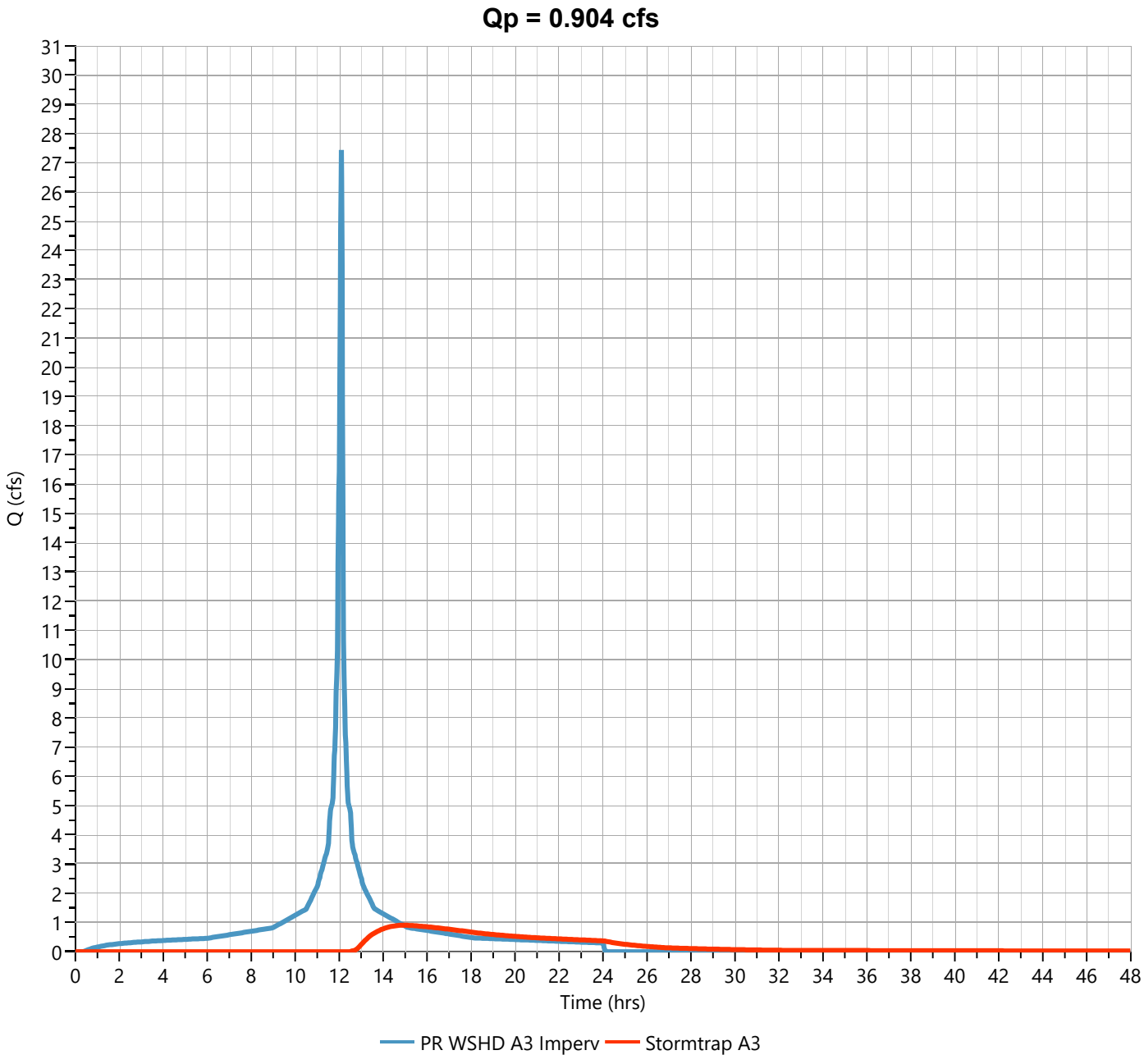
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

Stormtrap A3 — Hyd. No. 5

Hydrograph Type	= Pond Route	Peak Flow	= 0.904 cfs
Storm Frequency	= 100-yr	Time to Peak	= 14.93 hrs
Time Interval	= 1 min	Hydrograph Volume	= 30,142 cuft
Inflow Hydrograph	= 4 - PR WSHD A3 Imperv	Max. Elevation	= 4.00 ft
Pond Name	= Stormtrap A3	Max. Storage	= 73,838 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 3.68 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 7.91 hrs



Hydrograph Report

Hydrology Studio v 3.0.0.43

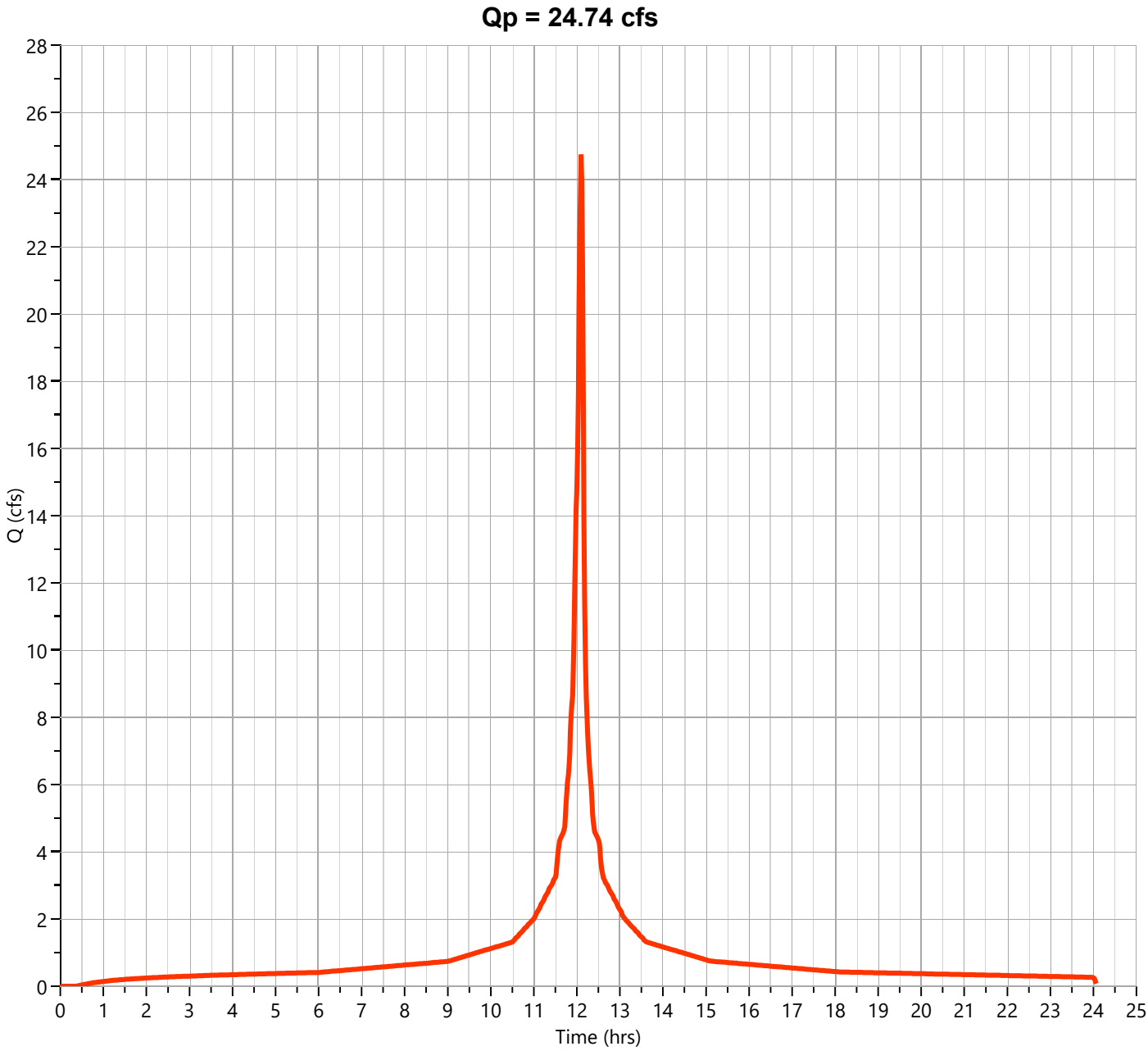
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03-19-2026

PR WSHD A4 Imperv

Hyd. No. 7

Hydrograph Type	= NRCS Runoff	Peak Flow	= 24.74 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 85,315 cuft
Drainage Area	= 2.11 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 4.0 min
Total Rainfall	= 11.38 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

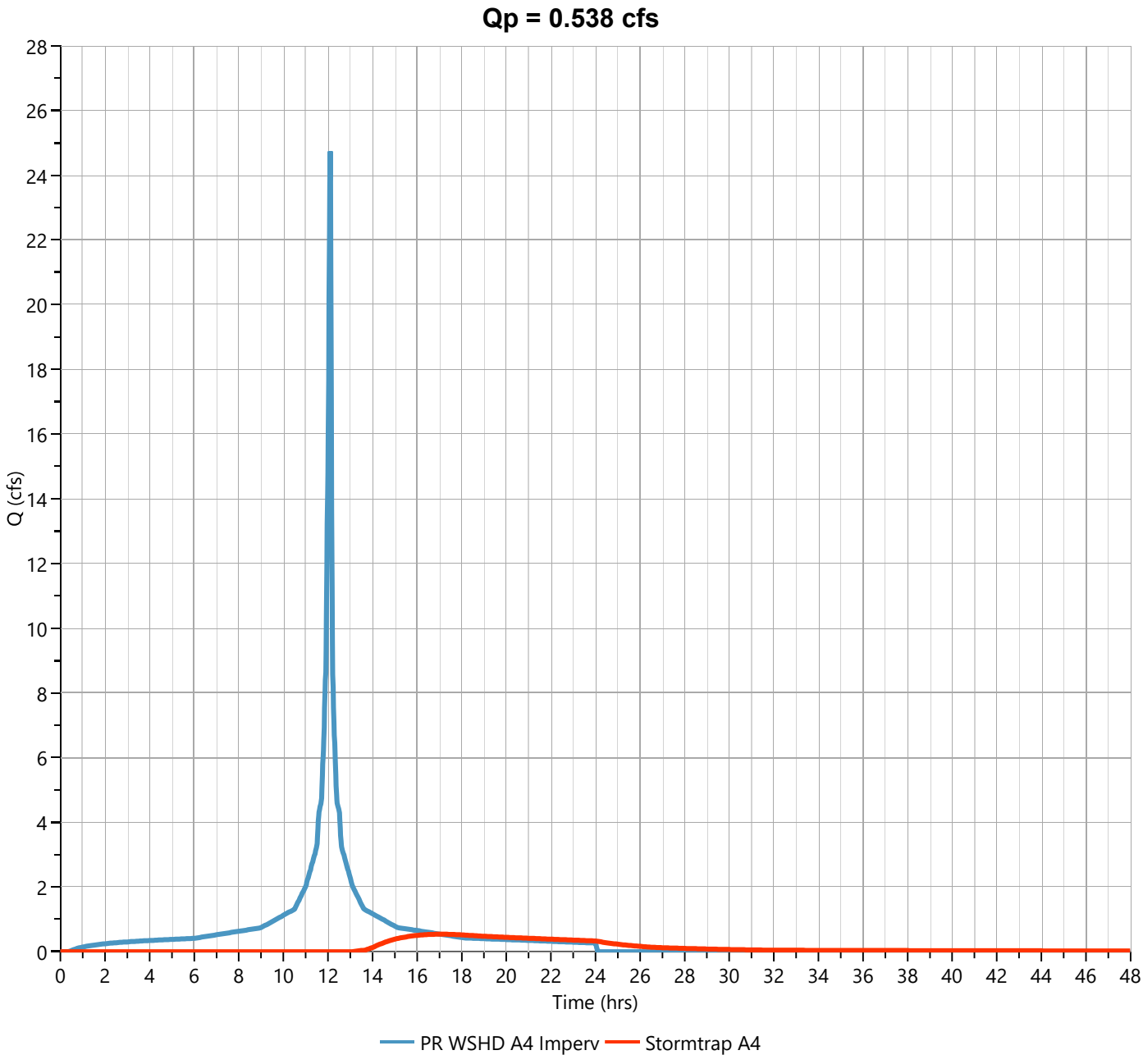
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

Stormtrap A4 — Hyd. No. 8

Hydrograph Type	= Pond Route	Peak Flow	= 0.538 cfs
Storm Frequency	= 100-yr	Time to Peak	= 17.05 hrs
Time Interval	= 1 min	Hydrograph Volume	= 20,855 cuft
Inflow Hydrograph	= 7 - PR WSHD A4 Imperv	Max. Elevation	= 3.92 ft
Pond Name	= Stormtrap A4	Max. Storage	= 71,393 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 3.68 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 9.64 hrs



Hydrograph Report

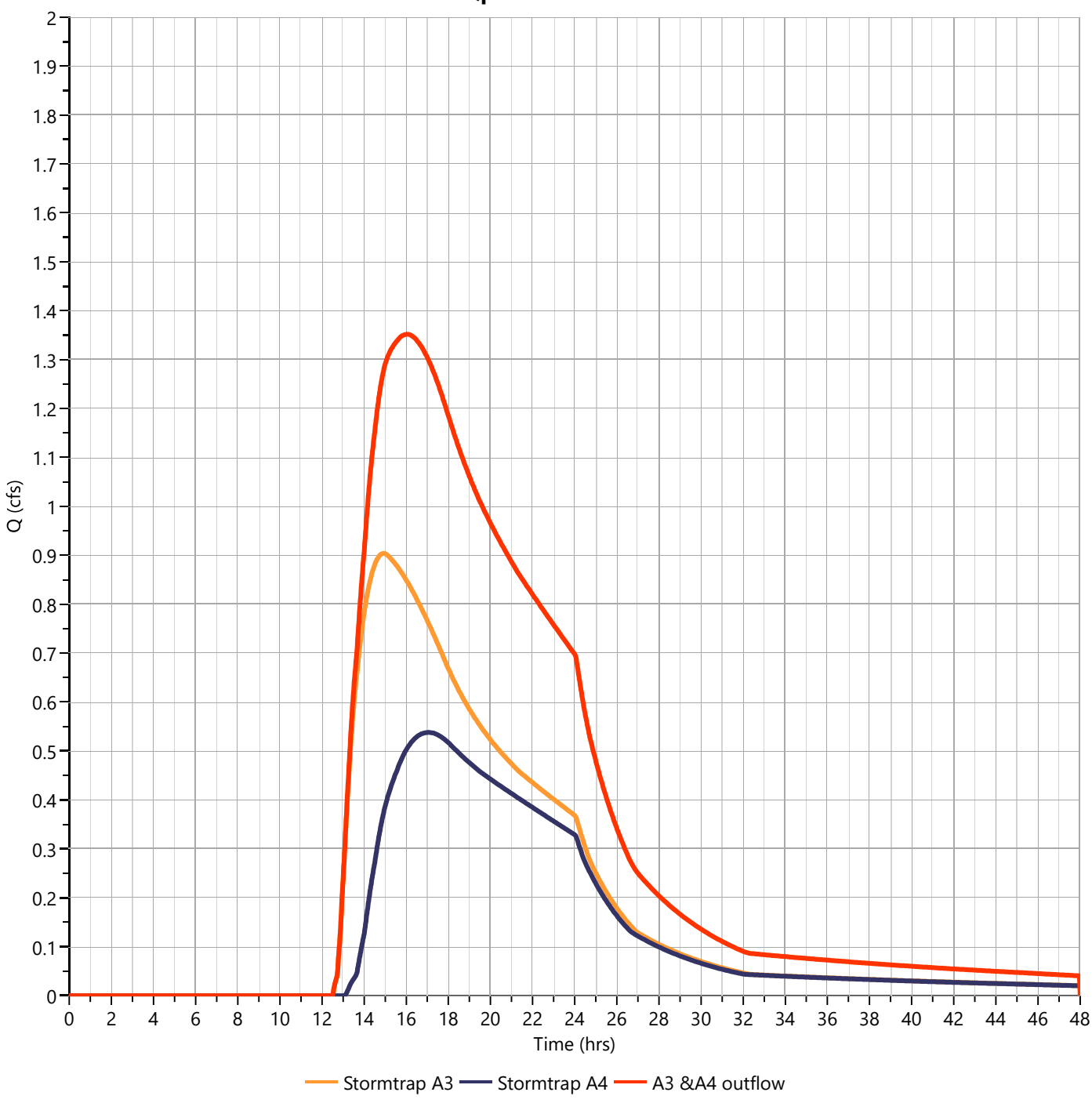
Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

A3 &A4 outflow

Hyd. No. 9

Hydrograph Type	= Junction	Peak Flow	= 1.352 cfs
Storm Frequency	= 100-yr	Time to Peak	= 16.03 hrs
Time Interval	= 1 min	Hydrograph Volume	= 50,998 cuft
Inflow Hydrographs		Total Contrib. Area	= 0.0 ac



Hydrograph Report

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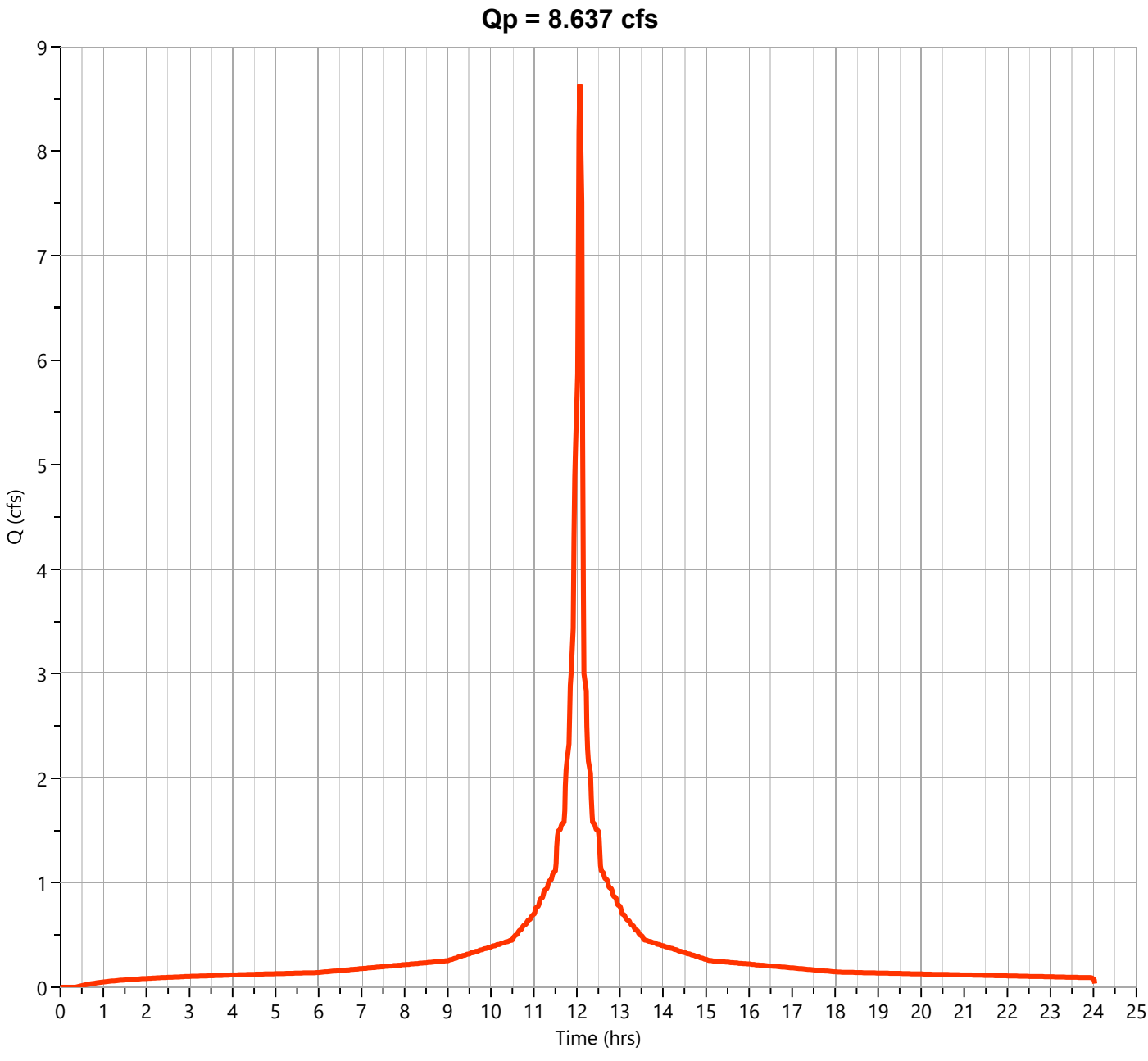
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

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PR WSHD A5 Imperv

Hyd. No. 11

Hydrograph Type	= NRCS Runoff	Peak Flow	= 8.637 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 29,188 cuft
Drainage Area	= 0.77 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 11.38 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

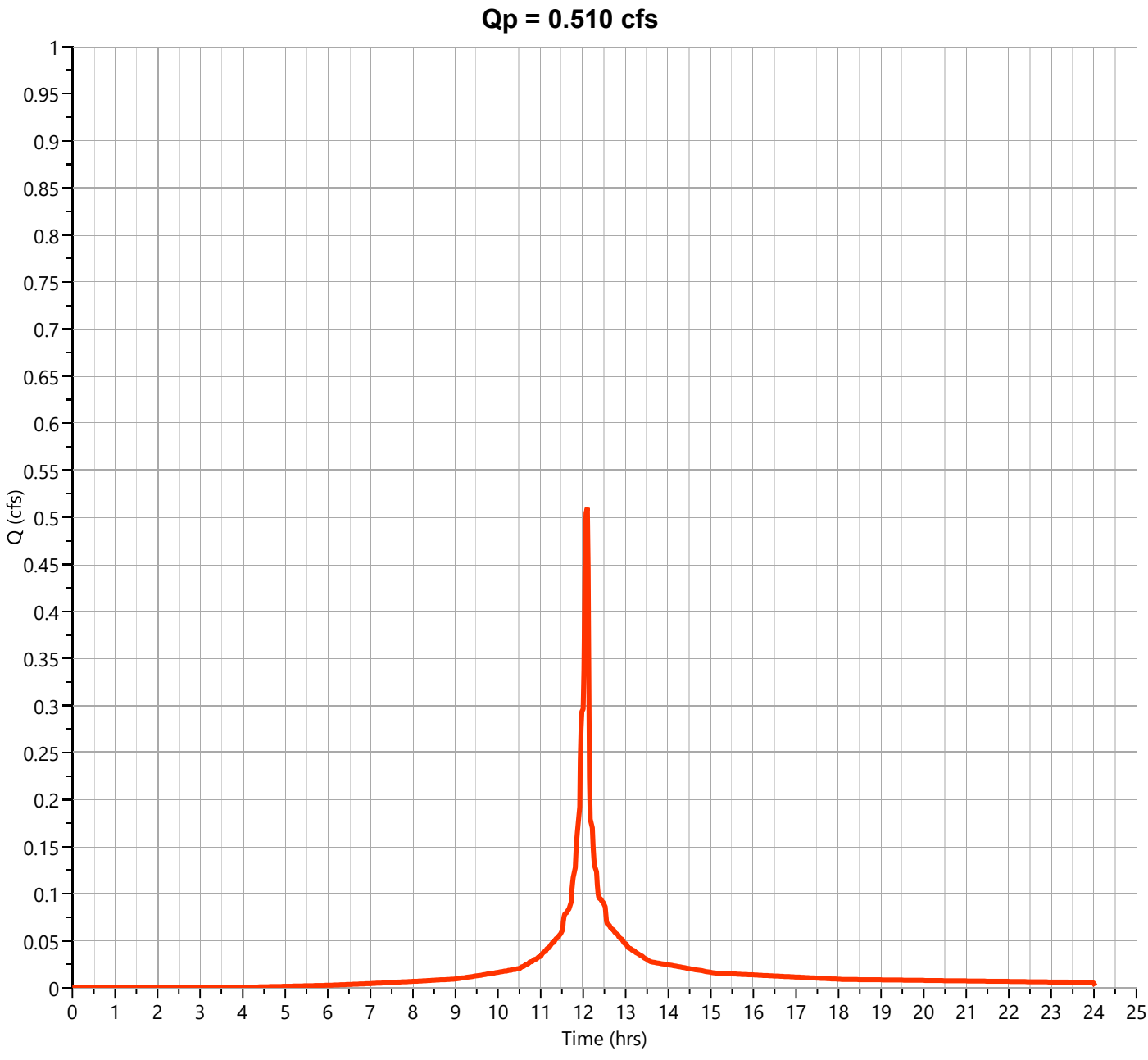
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PR WSHD A5 Perv

Hyd. No. 12

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.510 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 1,505 cuft
Drainage Area	= 0.05 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 11.38 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

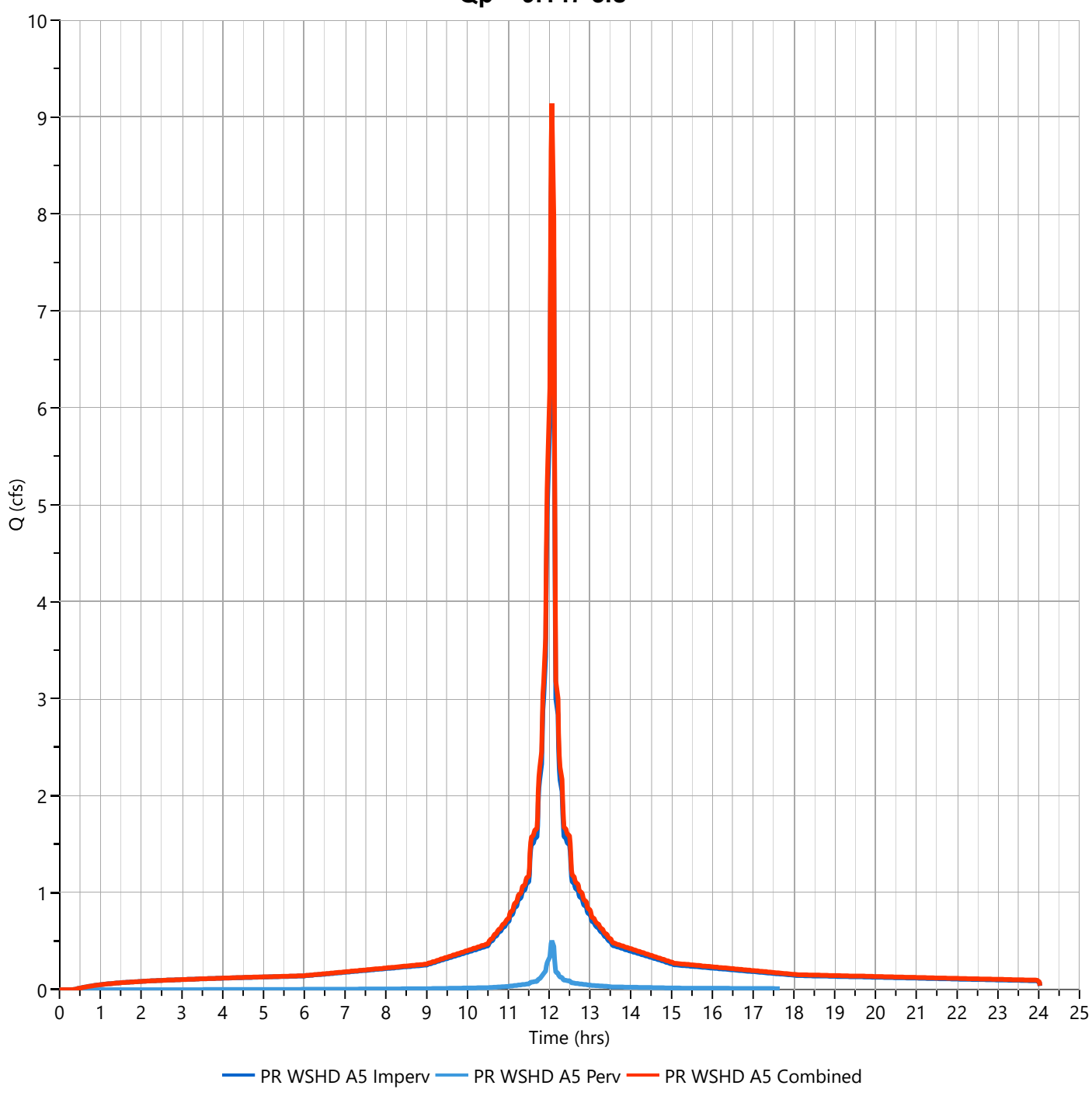
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PR WSHD A5 Combined

Hyd. No. 13

Hydrograph Type	= Junction	Peak Flow	= 9.147 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Hydrograph Volume	= 30,693 cuft
Inflow Hydrographs	= 11, 12	Total Contrib. Area	= 0.82 ac



Hydrograph Report

Hydrology Studio v 3.0.0.43

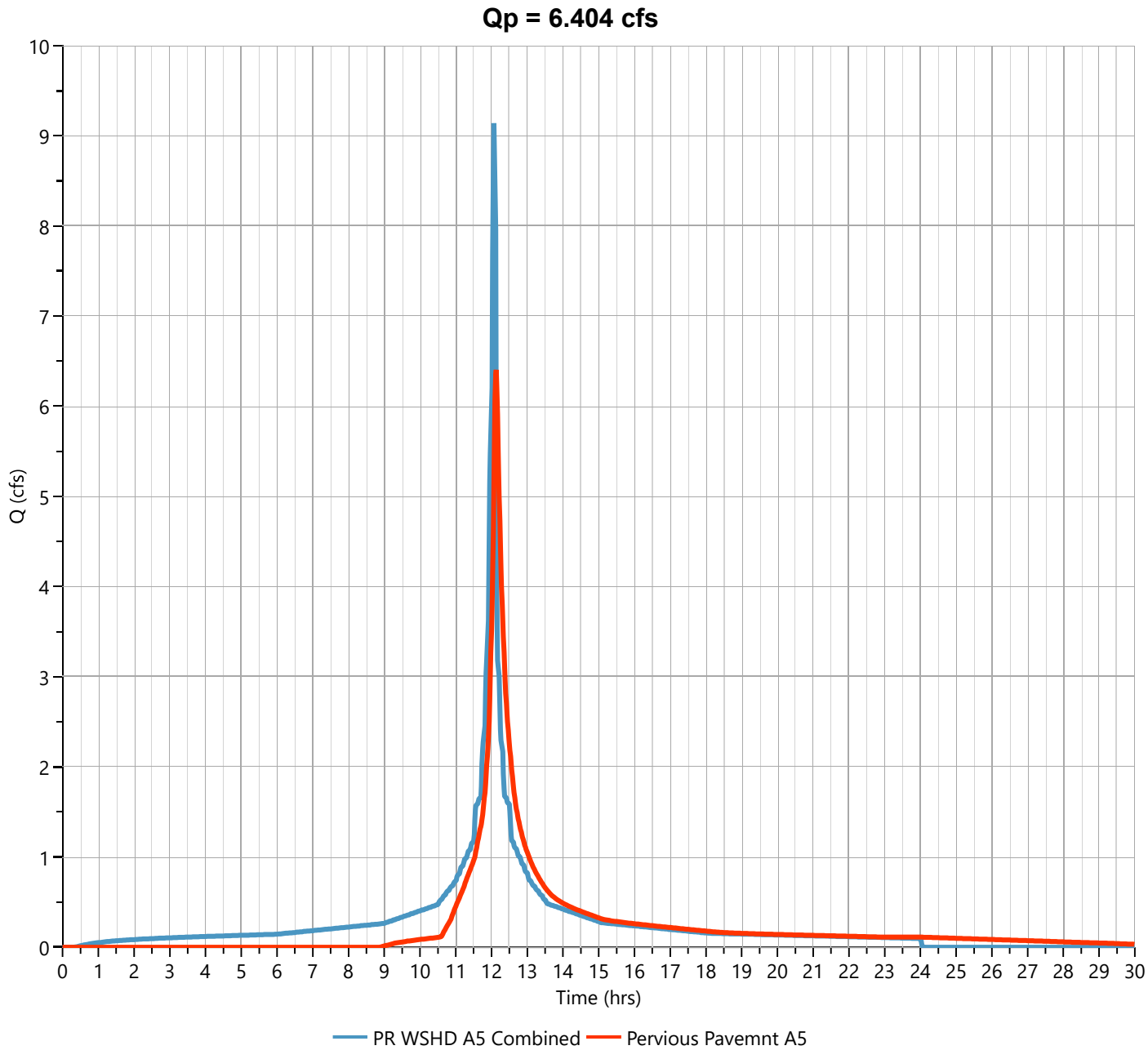
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

Pervious Pavemnt A5 — Hyd. No. 14

Hydrograph Type	= Pond Route	Peak Flow	= 6.404 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 1 min	Hydrograph Volume	= 26,601 cuft
Inflow Hydrograph	= 13 - PR WSHD A5 Combined	Max. Elevation	= 6.07 ft
Pond Name	= Pervious Pavement A5	Max. Storage	= 9,883 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 4.53 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 2.04 hrs



Hydrograph Report

Hydrology Studio v 3.0.0.43

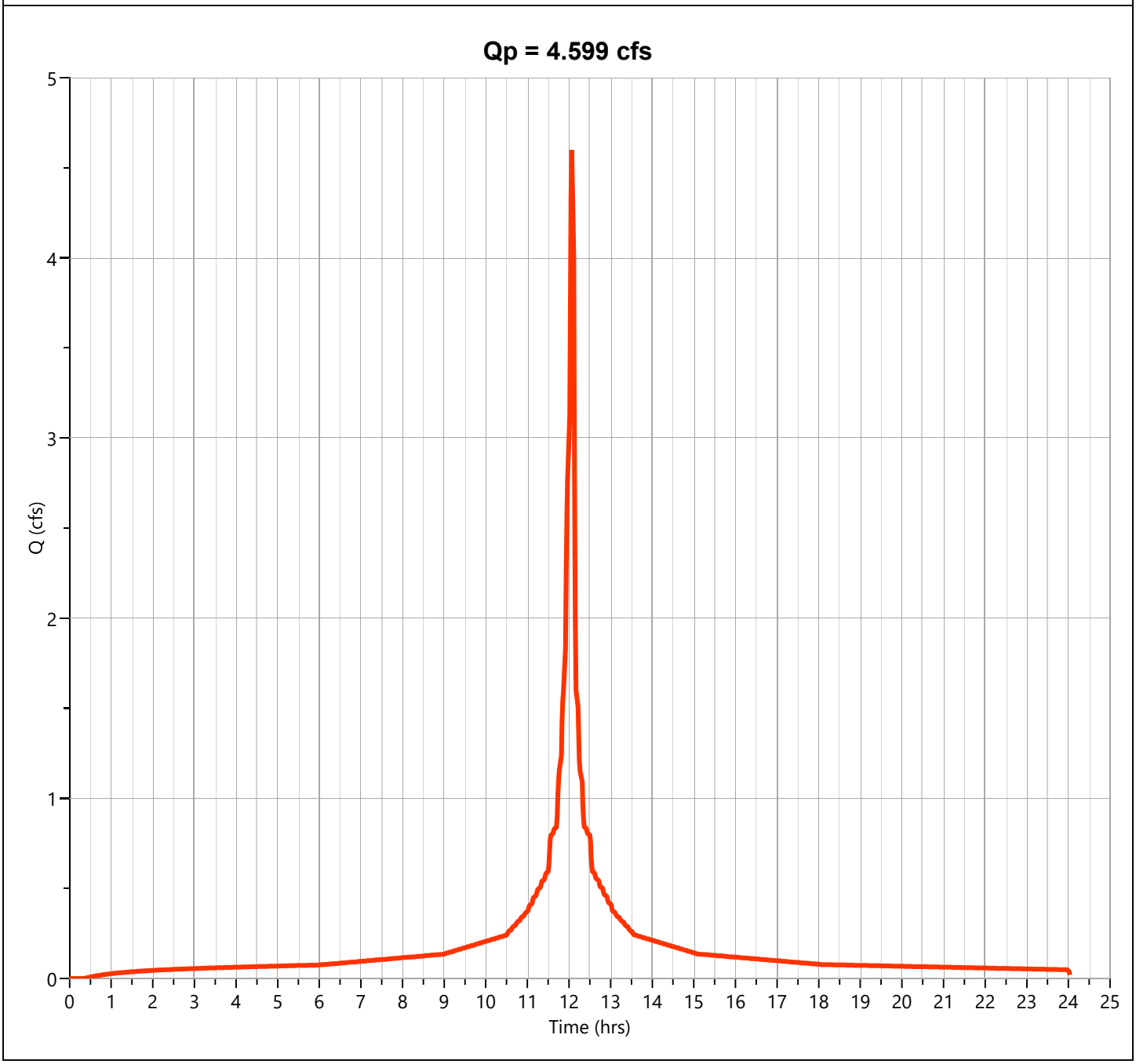
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PR WSHD A6 Imperv

Hyd. No. 16

Hydrograph Type	= NRCS Runoff	Peak Flow	= 4.599 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 15,542 cuft
Drainage Area	= 0.41 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 11.38 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

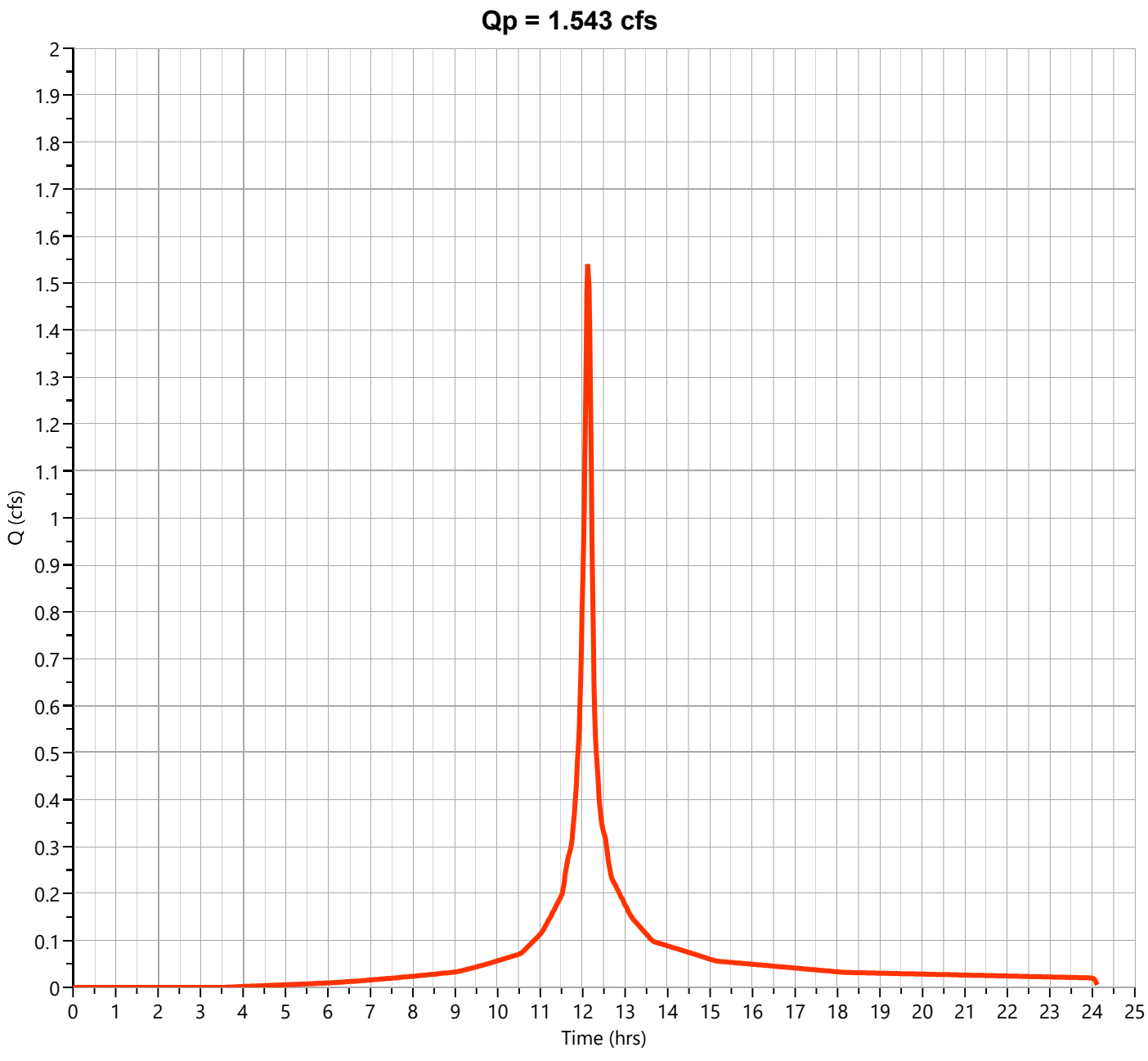
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PR WSHD A6 Perv

Hyd. No. 17

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.543 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 1 min	Runoff Volume	= 5,323 cuft
Drainage Area	= 0.17 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 8.0 min
Total Rainfall	= 11.38 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

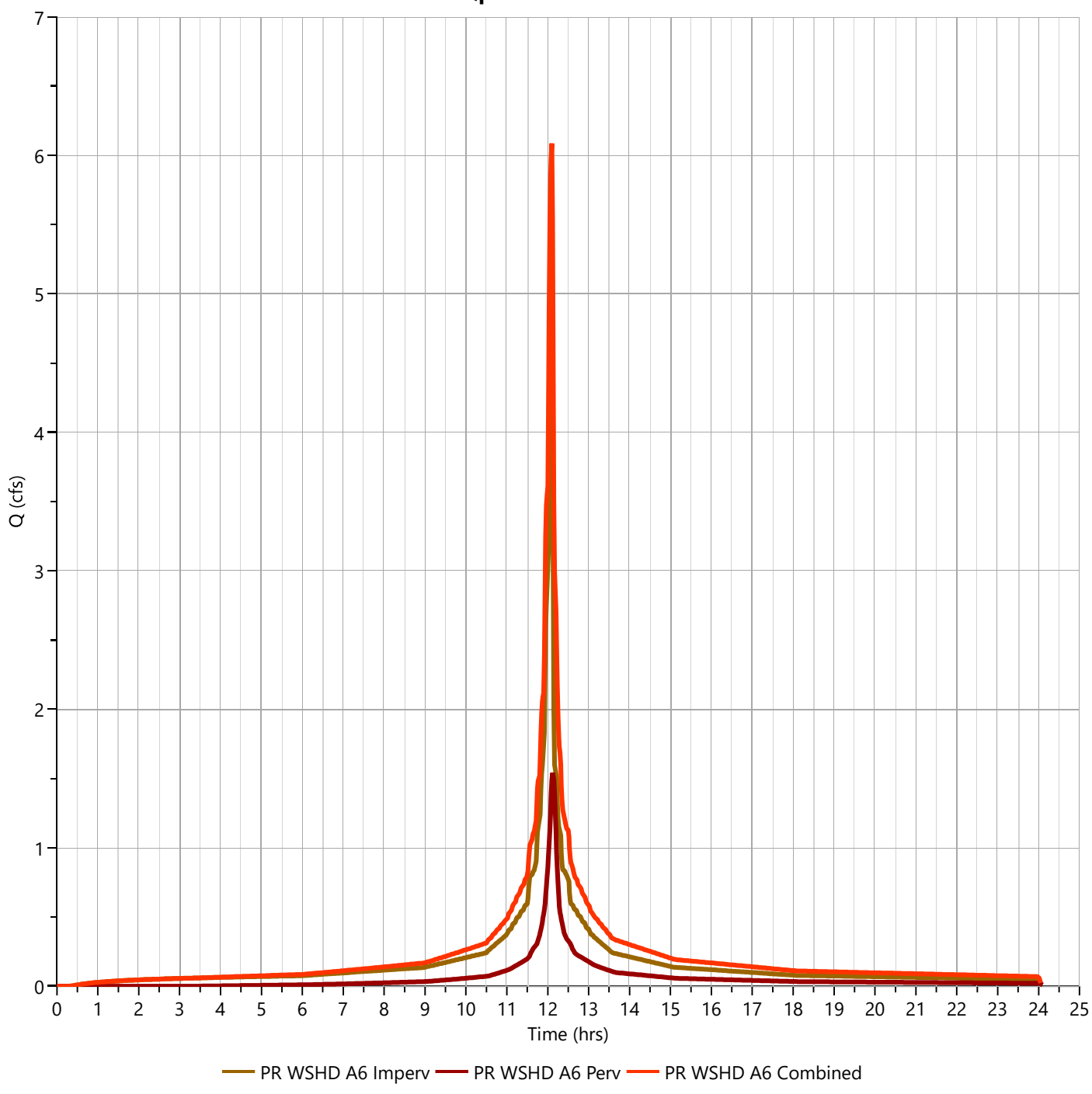
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PR WSHD A6 Combined

Hyd. No. 18

Hydrograph Type	= Junction	Peak Flow	= 6.086 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Hydrograph Volume	= 20,865 cuft
Inflow Hydrographs	= 16, 17	Total Contrib. Area	= 0.58 ac



Hydrograph Report

Hydrology Studio v 3.0.0.43

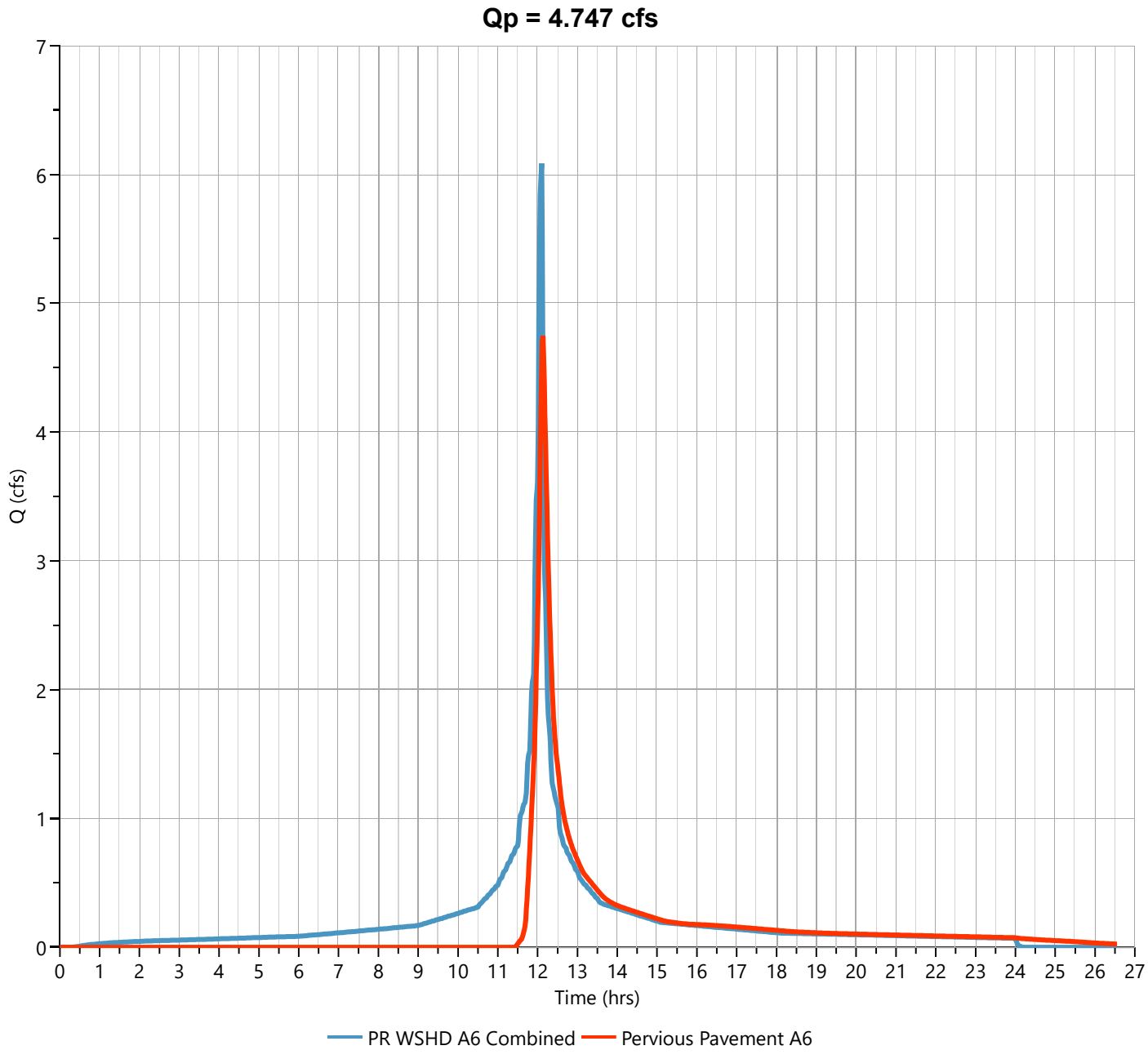
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03-19-2026

Pervious Pavement A6 — Hyd. No. 19

Hydrograph Type	= Pond Route	Peak Flow	= 4.747 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 1 min	Hydrograph Volume	= 15,412 cuft
Inflow Hydrograph	= 18 - PR WSHD A6 Combined	Max. Elevation	= 5.45 ft
Pond Name	= Pervious Pavement A6	Max. Storage	= 8,030 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 4.53 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 1.90 hrs



Hydrograph Report

Hydrology Studio v 3.0.0.43

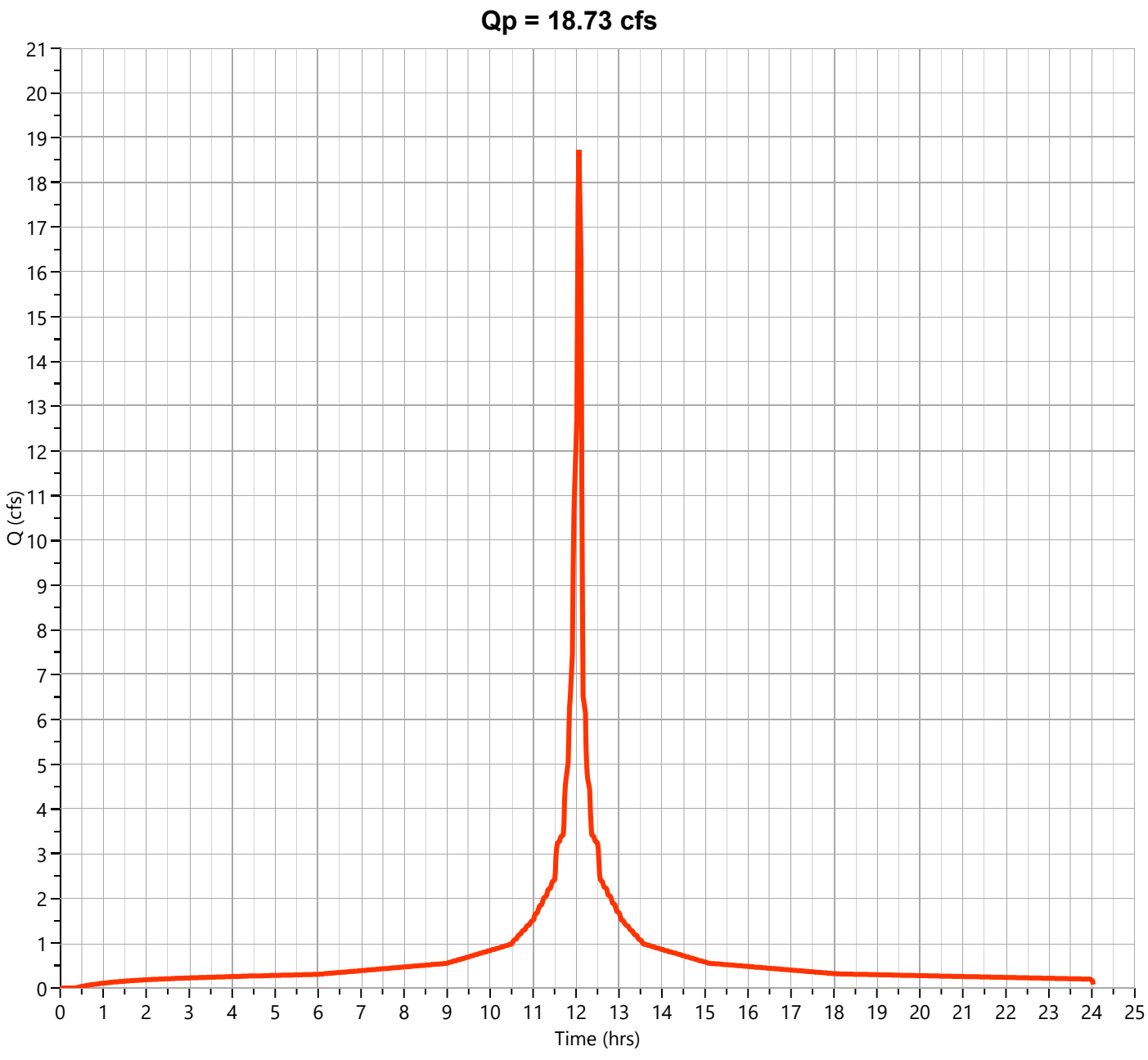
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PR WSHD A7 Imperv

Hyd. No. 21

Hydrograph Type	= NRCS Runoff	Peak Flow	= 18.73 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 1 min	Runoff Volume	= 63,304 cuft
Drainage Area	= 1.67 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 2.0 min
Total Rainfall	= 11.38 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Hydrology Studio v 3.0.0.43

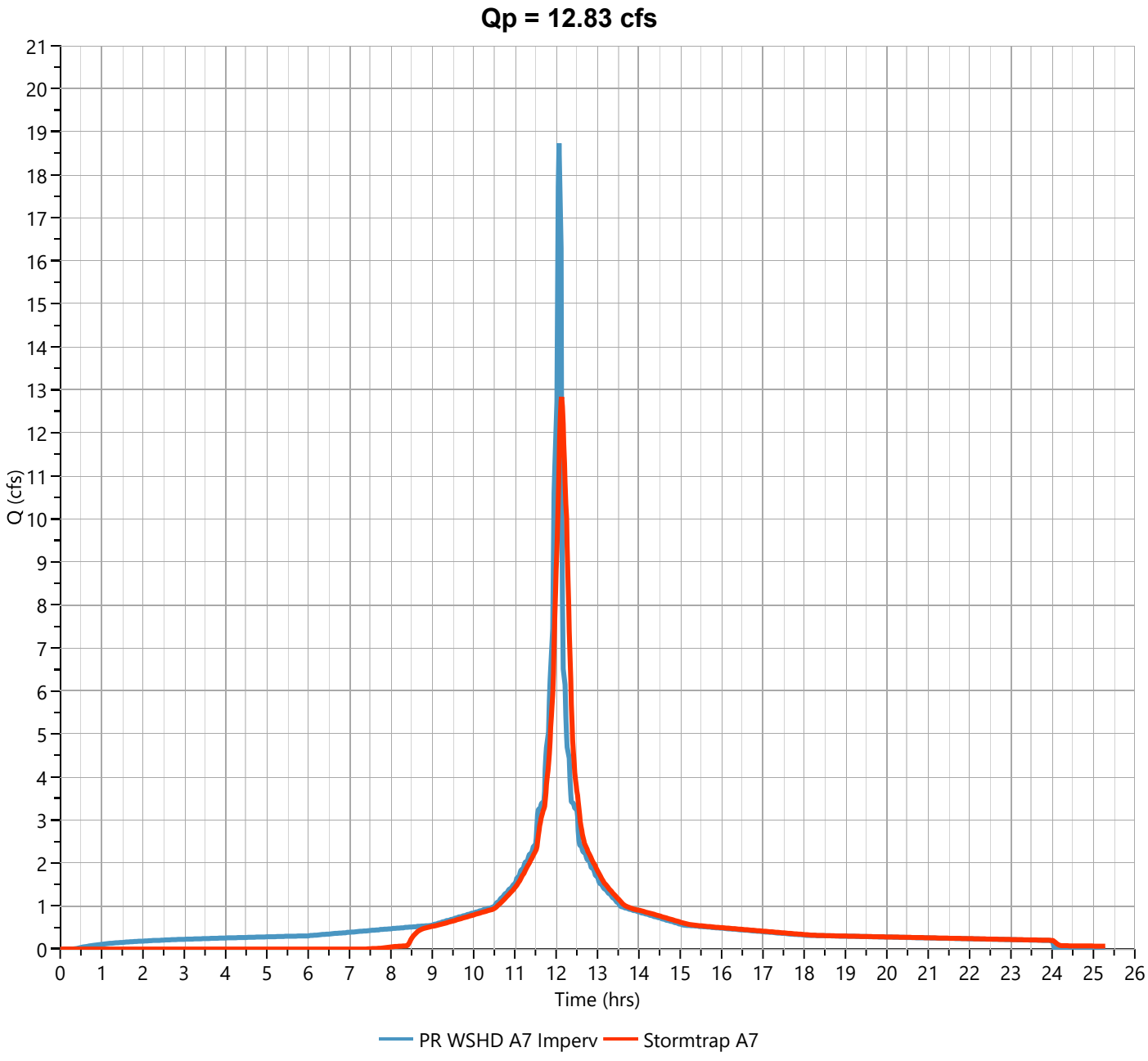
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

Stormtrap A7 — Hyd. No. 22

Hydrograph Type	= Pond Route	Peak Flow	= 12.83 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 1 min	Hydrograph Volume	= 57,075 cuft
Inflow Hydrograph	= 21 - PR WSHD A7 Imperv	Max. Elevation	= 5.54 ft
Pond Name	= Stormtrap A7	Max. Storage	= 12,996 cuft
Routing Option	= Fixed Tailwater	Tailwater Elevation	= 4.29 ft

Pond Routing by Storage Indication Method

Center of mass detention time = 1.08 hrs



Hydrograph Report

Hydrology Studio v 3.0.0.43

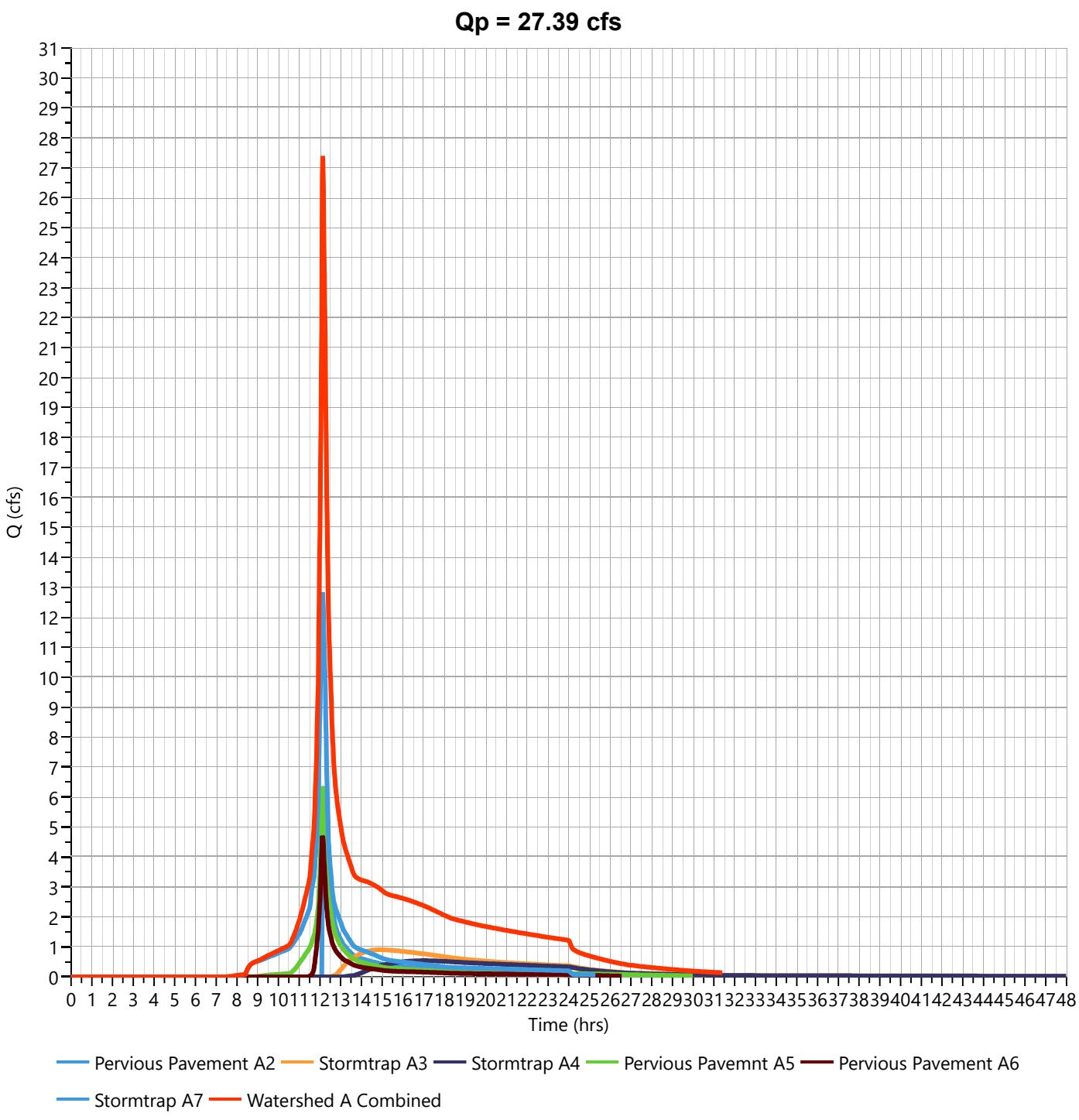
File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

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Watershed A Combined

Hyd. No. 24

Hydrograph Type	= Junction	Peak Flow	= 27.39 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 1 min	Hydrograph Volume	= 170,542 cuft
Inflow Hydrographs		Total Contrib. Area	= 0.0 ac



Hydrograph Report

Hydrology Studio v 3.0.0.43

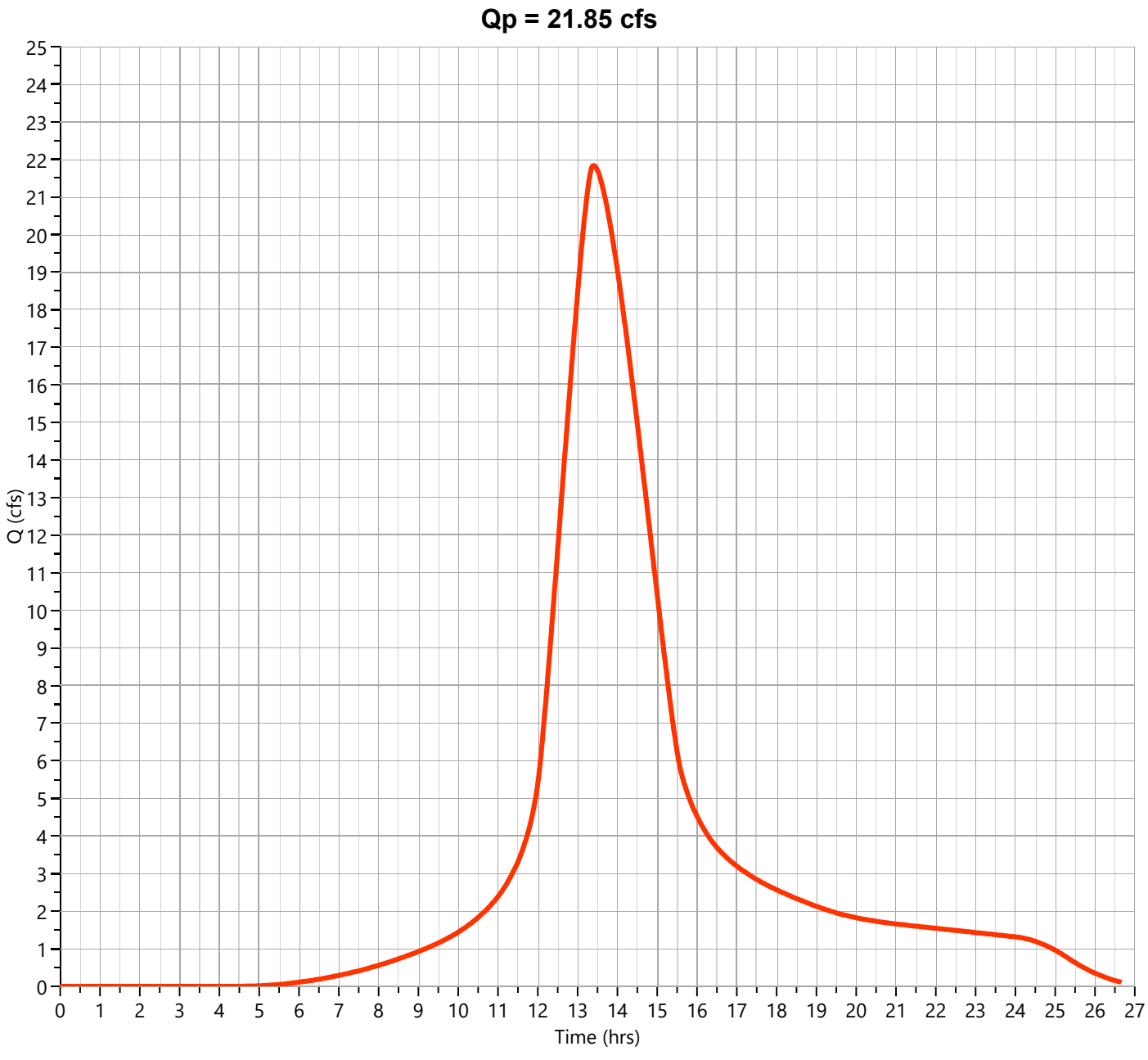
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PR WSHD B Perv

Hyd. No. 26

Hydrograph Type	= NRCS Runoff	Peak Flow	= 21.85 cfs
Storm Frequency	= 100-yr	Time to Peak	= 13.40 hrs
Time Interval	= 1 min	Runoff Volume	= 291,637 cuft
Drainage Area	= 9.5 ac	Curve Number	= 77.00
Tc Method	= User	Time of Conc. (Tc)	= 134.0 min
Total Rainfall	= 11.38 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

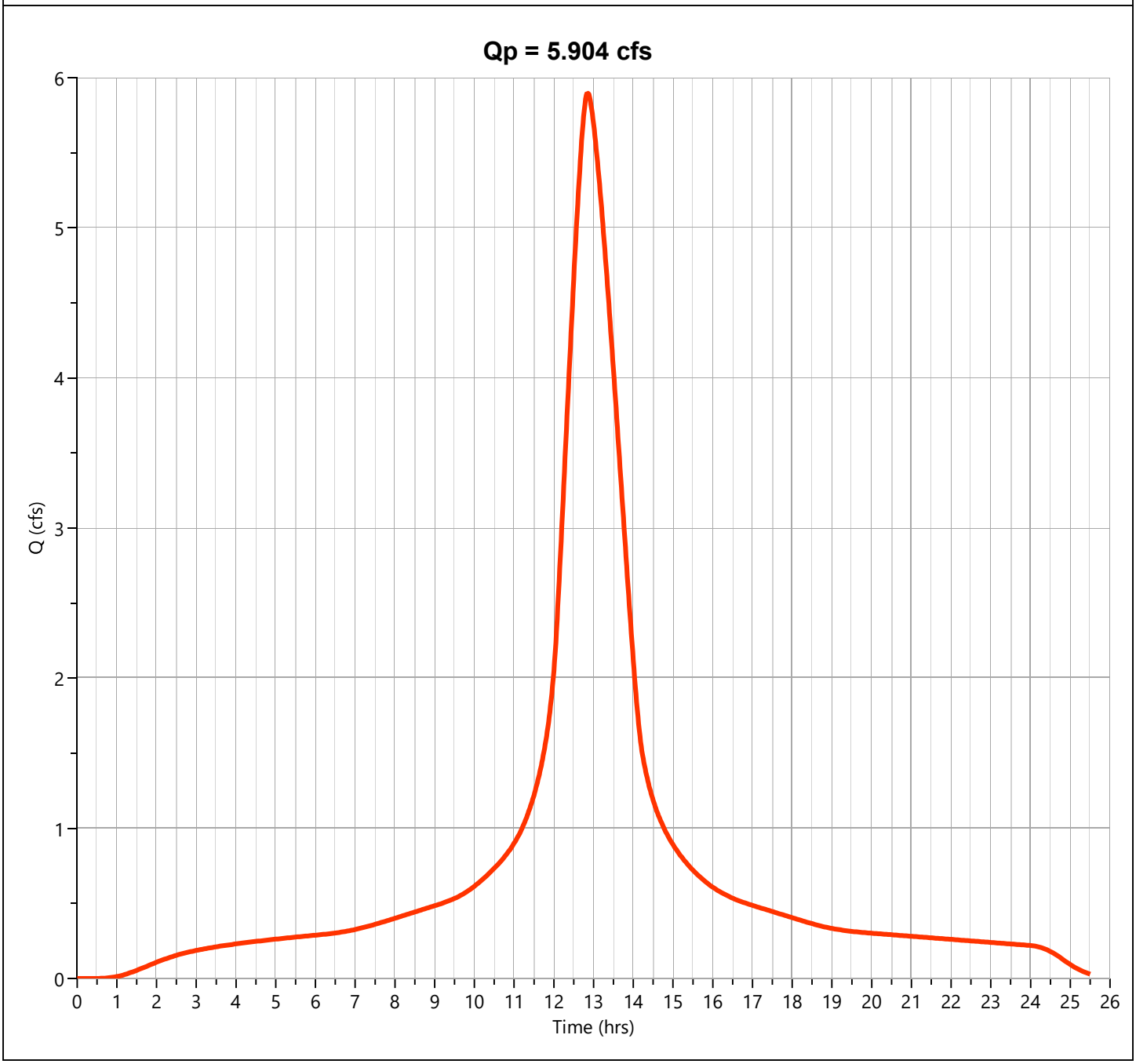
Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

PR WSHD B Imperv

Hyd. No. 27

Hydrograph Type	= NRCS Runoff	Peak Flow	= 5.904 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.85 hrs
Time Interval	= 1 min	Runoff Volume	= 64,694 cuft
Drainage Area	= 1.6 ac	Curve Number	= 98.00
Tc Method	= User	Time of Conc. (Tc)	= 81.0 min
Total Rainfall	= 11.38 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

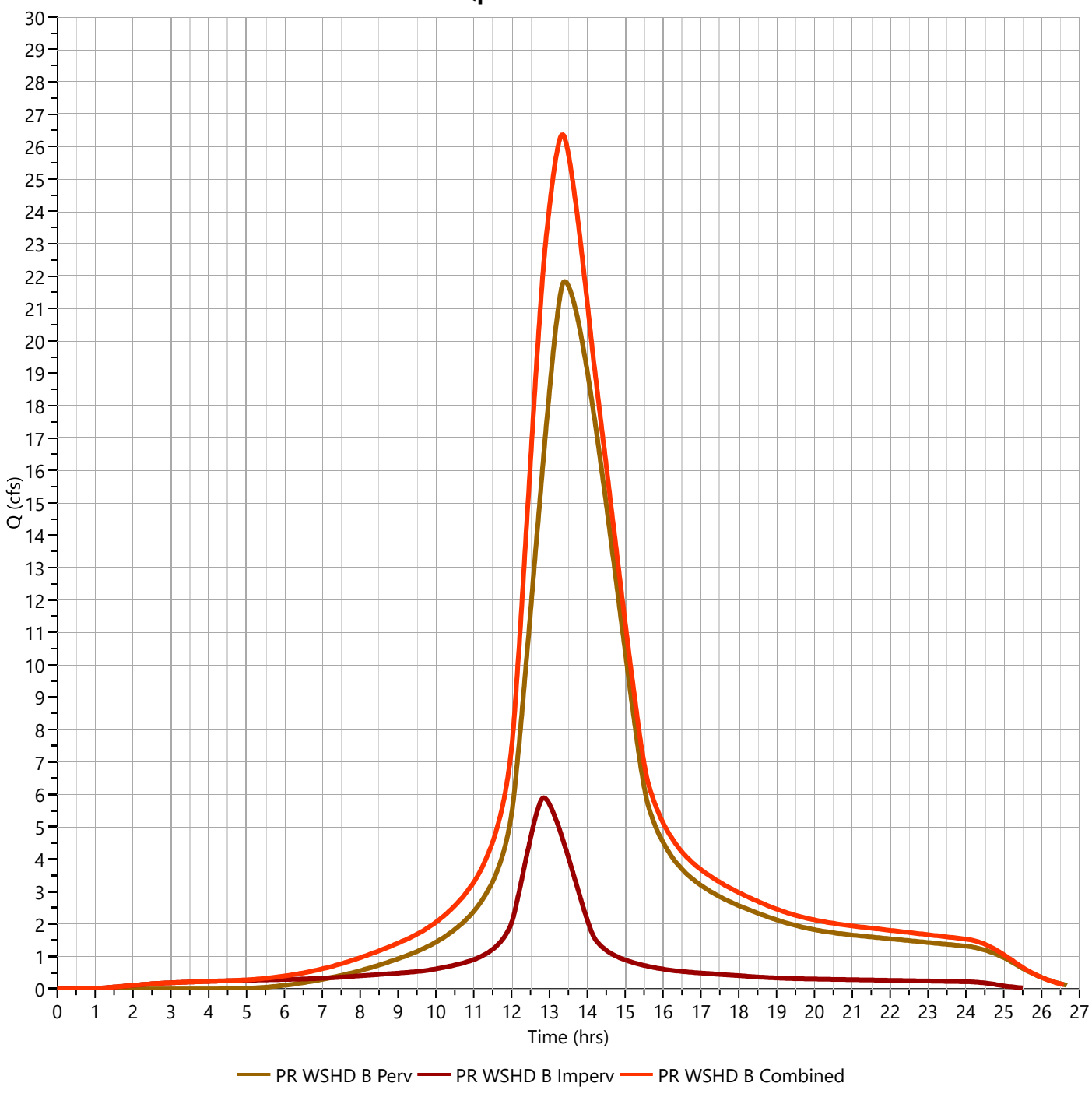
Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

PR WSHD B Combined

Hyd. No. 28

Hydrograph Type	= Junction	Peak Flow	= 26.38 cfs
Storm Frequency	= 100-yr	Time to Peak	= 13.35 hrs
Time Interval	= 1 min	Hydrograph Volume	= 356,330 cuft
Inflow Hydrographs	= 26, 27	Total Contrib. Area	= 11.1 ac



Hydrograph Report

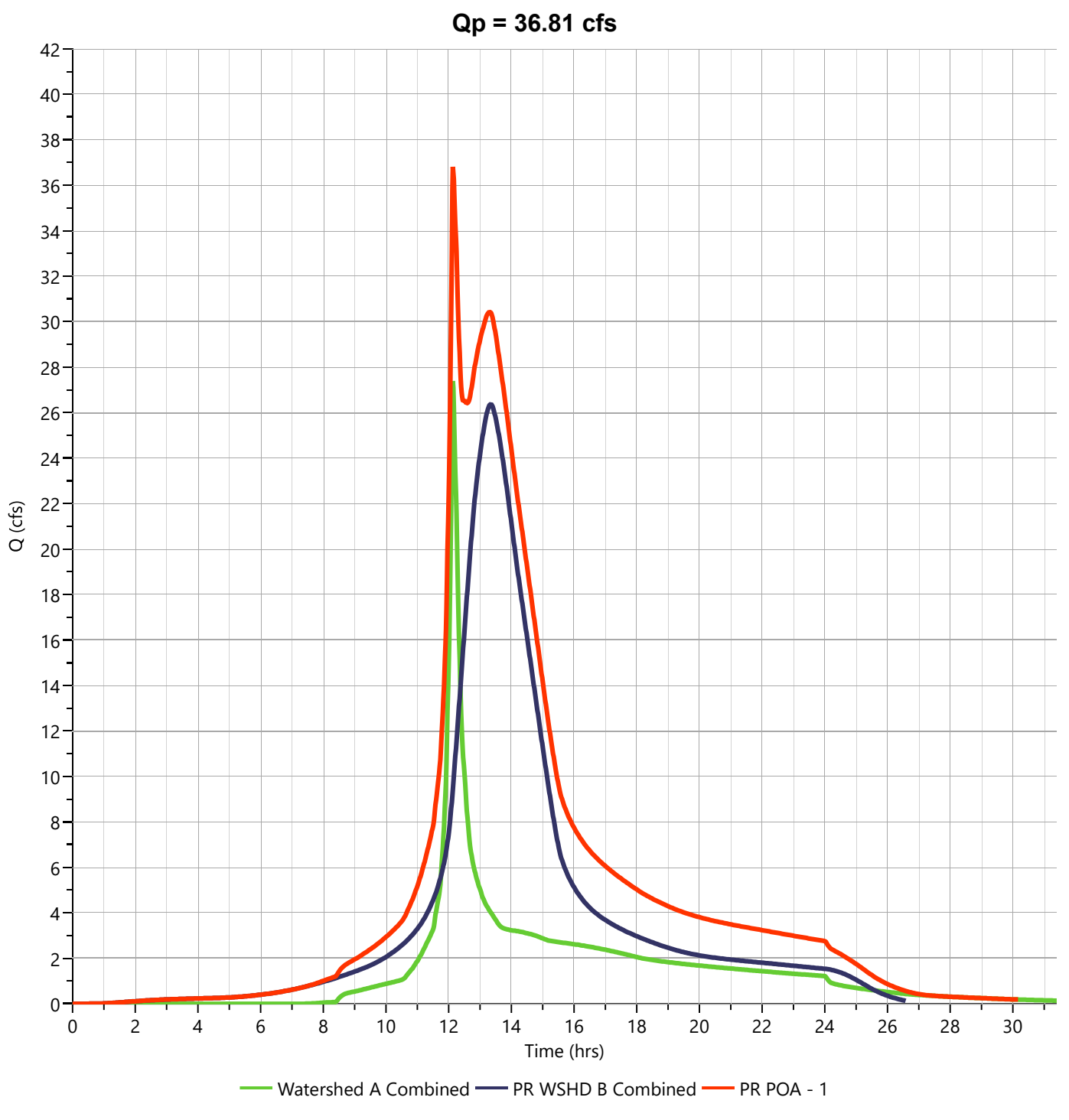
Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys
03-19-2026

PR POA - 1

Hyd. No. 30

Hydrograph Type	= Junction	Peak Flow	= 36.81 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 1 min	Hydrograph Volume	= 526,872 cuft
Inflow Hydrographs		Total Contrib. Area	= 0.0 ac



Hydrograph Report

Hydrology Studio v 3.0.0.43

File: Proposed HydrologyProjected Precipitation TW2.60 plus conveyance.hys

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PR WSHD C PerPOA-2

Hyd. No. 33

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.438 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.15 hrs
Time Interval	= 1 min	Runoff Volume	= 1,606 cuft
Drainage Area	= 0.05 ac	Curve Number	= 80.00
Tc Method	= User	Time of Conc. (Tc)	= 10.0 min
Total Rainfall	= 11.38 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

