



U.S. Department
of Transportation
**Federal Aviation
Administration**

Advisory Circular

Subject: Standards for Using Remote
Sensing Technologies in Airport Surveys

Date: 9/29/2017
Initiated by: AAS-100

AC No: 150/5300-17C
Change: 1

1 **Purpose.**

This Advisory Circular (AC) provides guidance regarding the use of remote sensing technologies in the collection of data describing the physical infrastructure of an airport. This AC describes the acceptable uses and standards for use of different remote sensing technologies in the data collection process.

2 **Cancellation.**

This AC cancels AC 150/5300-17B, *General Guidance and Specifications for Aeronautical Survey Airport Imagery Acquisition and Submission to the National Geodetic Survey*, dated September 29, 2008.

3 **Application.**

The Federal Aviation Administration (FAA) recommends the use of the guidance and specifications in this Advisory Circular for the collection and submission of data using remote sensing technologies. In general, use of this AC is not mandatory. However, the use of this AC is mandatory for all projects funded through the Airport Improvement Program (AIP) or Passenger Facility Charges (PFC) Program. See Grant Assurance No. 34, "Policies, Standards, and Specifications," and PFC Assurance No. 9, "Standards and Specifications."

4 **Principal Changes.**

Revision C is a substantial rewrite of this advisory circular. Users should review the entire document. Major changes include reformatting, more detailed explanations, and new sections on remote sensing technologies other than aerial imagery (primarily LIDAR) for collecting airport data. Revisions made by Change 1 are identified with change bars in the margin.

1. This Change updates the orthoimagery coverage area in paragraphs 4.2 and 4.3.
2. The reference document "Light Detection and Ranging (LIDAR) Requirements" has been moved to the FAA Airports GIS website, and the references in paragraphs

2.1.9.12 and 5.8.2 have been updated to show the current location at <https://airports-gis.faa.gov/public/surveyorsIntro.html>.

3. Only the text revised in this Change is indicated by vertical bars in the margins.
4. Errata Sheet changes have been incorporated into this Change.
5. The format of the document has been updated in this Change, and minor editorial changes have been made throughout.

5 Feedback on this AC.

If you have suggestions for improving this AC, you may use the Advisory Circular Feedback form at the end of this AC.

6 Copies of This AC.

All ACs are available online at
http://www.faa.gov/regulations_policies/advisory_circulars/.



John R. Dermody
Director of Airport Safety and Standards

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CHAPTER 1. REMOTE SENSING TECHNOLOGIES

1.1 What are the acceptable remote sensing technologies for use in airport surveys?

There are three basic technologies in wide use today for the collection of data on and surrounding the airport. Each of these technologies has advantages and disadvantages to its use. The airport proponent should understand the capabilities of each technology including its benefits and limitations before deciding which technology or combination of technologies is appropriate for their project.

1.1.1 Aerial Imagery.

Aerial imagery is the most common technology being used in the planning, design, construction, and analysis activities of an airport. In collecting aerial imagery, an aircraft fitted with a camera (film or digital) flies a series of flight lines over the airport and surrounding area to capture images. Aerial imagery is a passive collection system since it relies on capturing the radiation (generally from the sun) reflected off an object and captured by the camera.

1.1.2 Light Detection and Ranging (LIDAR).

Light Detection and Ranging (LIDAR) scanning technology is a rapidly evolving field of active source remote sensing providing accurate spatial coordinates of individual points. The LIDAR systems calculate the spatial coordinate of an object using three variables. First, the system measures the reflected energy of a laser pulse. Second, it also uses the time of flight for each pulse. And third, when necessary, corrects the data for instrument platform motion to generate a geospatially referenced point cloud representation of the objects within its view. LIDAR scanners are used for a variety of survey tasks and currently fall into four principle categories:

- Ground Based LIDAR (GBL), generally used for measuring atmospheric composition
- Airborne LIDAR Mapping (ALM), also known as Airborne Terrestrial LIDAR Mapping (ATLM)
- Mobile Compensated LIDAR Mapping (MCLM)
- Terrestrial LIDAR Mapping (TLM), sometimes referred to as Ground Based LIDAR Scanning (GBLS or GBLM)

The use of the acronym GBLS and GBLM can create confusion between GBL and systems used for survey mapping (GBLS or GBLM), so all these systems are referred to as TLM.

These are the only LIDAR systems classified for the collection of airport data.

1.1.3 Satellite Imagery.

Satellite imagery uses the same basic concept as aerial imagery except the camera platform is a satellite in space. Currently, using satellite imagery to collect airport data is not an approved method. The FAA continues to research and identify new uses and standards regarding satellite imagery.

CHAPTER 2. REMOTE SENSING PROJECT PLANNING

2.1 What are the remote sensing plan requirements?

All projects incorporating the use of remote sensing technologies require the project's organization to submit a plan outlining their proposal for completing the data acquisition.

Submit a remote sensing plan to the FAA through Airports GIS (<http://airports-gis.faa.gov>) for review and approval prior to beginning data acquisition. Provide the plan in a non-editable format such as Adobe Portable Document Format (PDF)TM, detailing the following information:

2.1.1 General Project Information.

1. Airport Name.
2. Airport Identifier.
3. Submitting Organization. Include:

Name
Address
City
State
Zip Code
Telephone Number
FAX Number
Organization's Contact Person Name
Email

For this report, the submitting organization is the airport operator, owner, or sponsor. If used, identify the consultant collecting the information within the Airports GIS project.

2.1.2 Project Purpose.

Briefly outline the purpose of the project. State why the airport is undertaking the data acquisition. If the project supports a larger project such as construction activity, indicate this in the project purpose.

2.1.3 Project Boundaries.

Briefly describe project boundaries of the data acquisition project. Include details about the surfaces and/or areas the data acquisition will cover such as obstruction identification surfaces (OIS), airport properties and other areas controlling the extent of the acquisition. Should the imagery acquisition include OIS, specify the runways for evaluation, as in [Figure 2-3](#). In addition to the description, provide an active (Keyhole Markup Language) KML file of the project areas. See [Figure 2-1](#) for an example. See also paragraph [2.4](#).

2.1.4 Project Parameters.

Briefly detail the data acquisition for the proposed aerial imagery mission. Describe the planned digital image resolution, horizontal and vertical accuracy values in the data collection. Provide the accuracies in feet using Root Mean Square Error (RMSE). The values must comply with values in Table 3-1. If using multiple flight missions at different flying heights for the imagery acquisition, provide details for each flight mission. Include at a minimum the following information:

- Flying height or Above Ground Level (AGL)
- Overlap percentage
- Sidelap percentage
- Number of flight lines
- Number of total exposures
- Camera film type (if using film)
- Flight mission date range and sun angle range
- Active Keyhole Markup Language (KML) file of the flight lines, as shown in Figure 2-1.

An “active KML” file means a file generated using Google Earth™ to generate the KML code. KML is a file format to display geographic data in an Earth browser such as Google Earth™, Google Maps™, and Google Maps for mobile™. KML uses a tag-based structure with nested elements and attributes and is based on XML. All tags are case-sensitive and must appear exactly as listed in the KML Reference materials available from Google™.

Figure 2-1 shows the KML code for a sample placemark.

Figure 2-1 Sample KML Code for a Placemark

```
<?xml version="1.0" encoding="UTF-8"?>
<kml xmlns="http://www.opengis.net/kml/2.2">
  <Placemark>
    <name>Simple placemark</name>
    <description>Attached to the ground. Intelligently places itself
      at the height of the underlying terrain.</description>
    <Point>
      <coordinates>-
        122.0822035425683,37.42228990140251,0</coordinates>
    </Point>
  </Placemark>
</kml>
```

In addition to the other requirements, in projects using ATLM technologies, define the data collection parameters. Identify for the entire flight (not each individual flight line) the:

- Flying height
- Speed over ground

- Scan angle
- Pulse Repetition Frequency (PRF)
- Overall density of the horizontal and vertical point spacing for the data acquisition

Figure 2-2 illustrates a combined flight line and supporting ground control network for the project. The yellow points are the control stations, the blue lines are low-level flight lines and the red lines are high-altitude flight lines.

Figure 2-2 Combined Flight Line and Supporting Ground Control Network

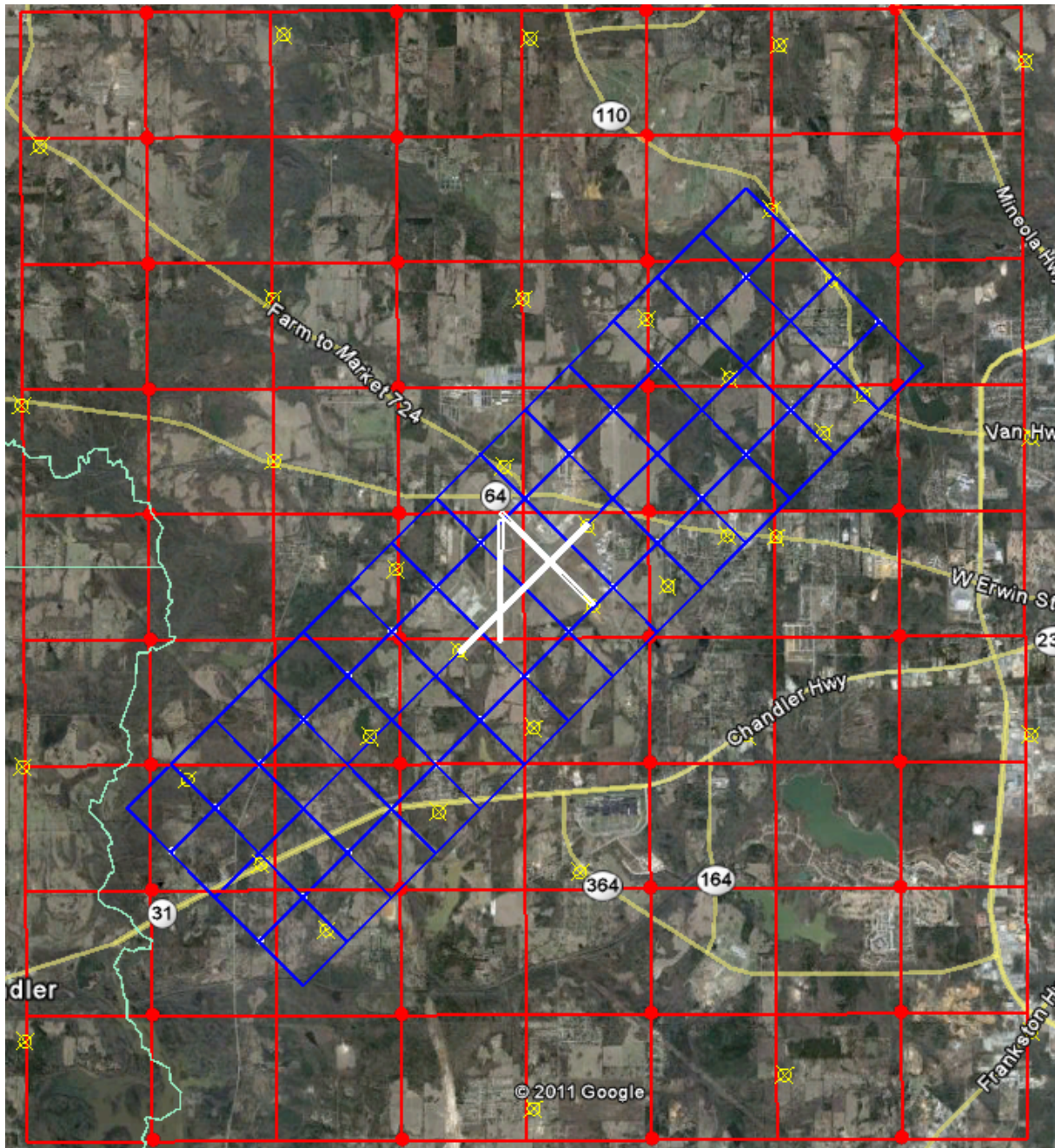
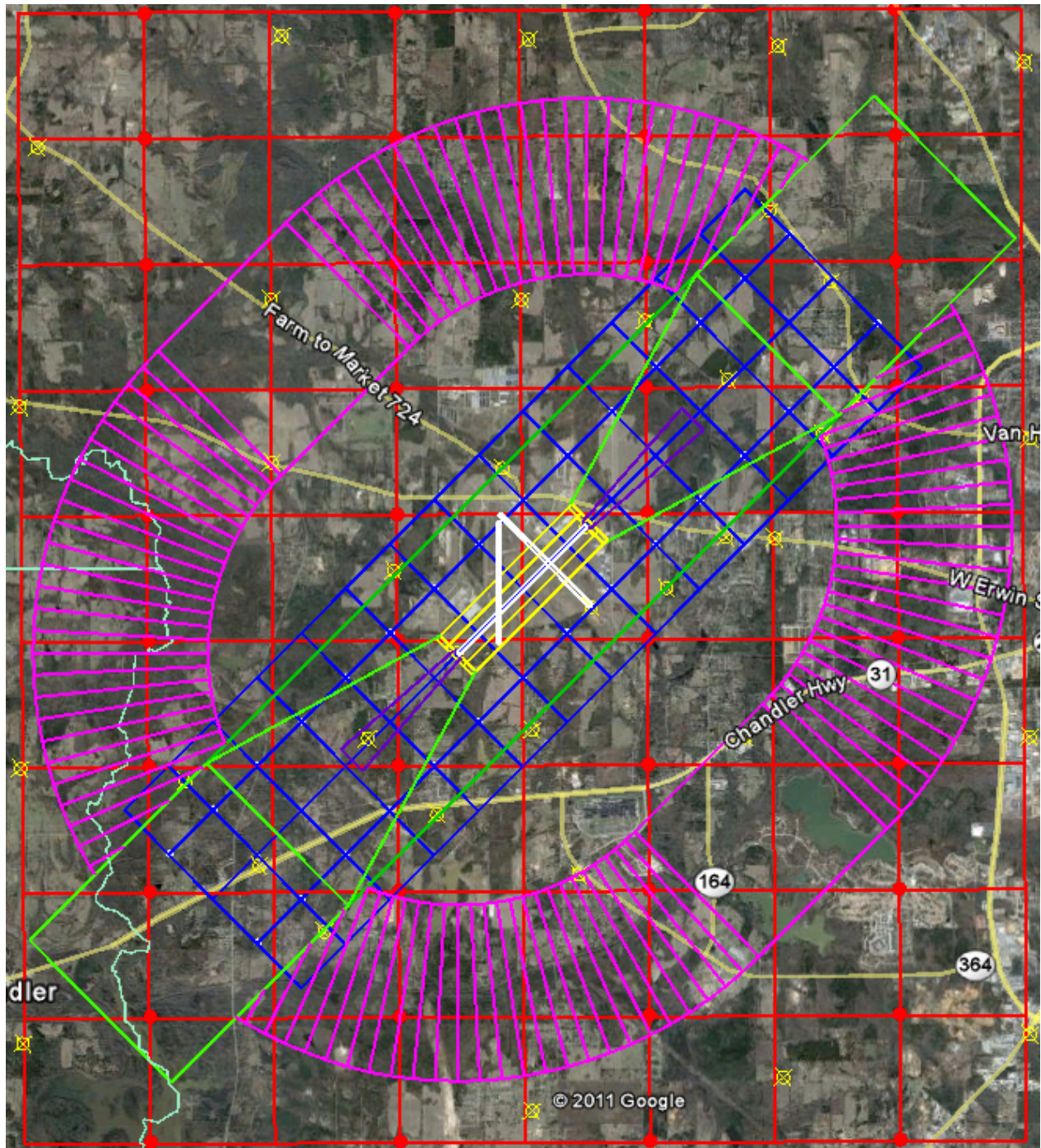


Figure 2-3 illustrates the combined flight line and supporting ground control network with the obstruction identification surfaces for the airport included.

Figure 2-3 Combined Flight Line and Network with Obstruction Identification Surfaces



2.1.5 Satellite Imagery Acquisitions.

Reserved for later implementation.

2.1.6 Proposing MCLM Technologies.

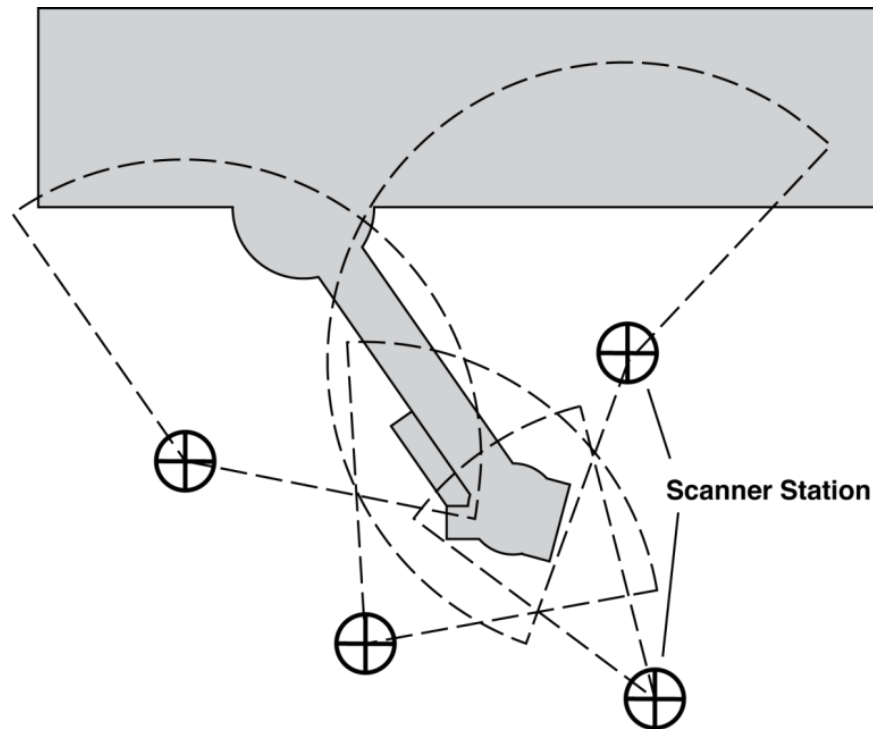
If proposing MCLM technologies, define the plan to verify the satellite availability during the times of data acquisition. MCLM projects must have a minimum of six satellites in view for the Global Navigation Satellite System (GNSS) Control Stations and the GNSS unit in the MCLM system. In the plan, identify the predicted Position Dilution of Precision (PDOP) for the times of data acquisition. For MCLM collections, the predicted and actual PDOP during acquisition must be a value of five or less.

2.1.7 Proposing TLM Technologies.

If proposing TLM technologies, reconnoiter the site to identify and document potential data collection stations. Document the proposed sites using digital photographs. The photographs will show the proposed target with electronically added captions. TLM systems capture features in great detail and can detect change at centimeter levels, and in some cases millimeter levels. However, achieving these levels of accuracy require multiple scanner setups. (See [Figure 2-11.](#)) Prior to survey execution, plan station selections for the entire survey area demonstrating capture of the features of interest. Ideally, also determine and document survey control and evaluation locations during reconnaissance. Plan to locate survey targets in areas of scan-arc overlap realizing the actual field conditions may require alternative locations for both scanner and target setup. Work with local airport authorities to determine the times of day when traffic has minimum impact on data quality. Plan to scan high traffic areas during times with the lightest traffic. Identify areas of vegetation and/or poor visibility. Detail how and when this data will be acquired from these areas.

In the remote sensing plan, provide a drawing or illustration depicting the location and scan coverage for each potential site.

[Figure 2-4](#) illustrates the scanner and target locations for collection of a passenger loading bridge feature.

Figure 2-4 Scanner and Target Locations for Passenger Loading Bridge Feature

The schematic of an airport terminal feature (passenger loading bridge) shows the potential numbers of stations necessary to fully capture the plan view outline of the feature. The dashed polygons in the figure depict the regions of capture available from each particular station.

2.1.8 Remote Sensing Equipment.

In this section, provide a brief description of the remote sensing equipment planned to be used during the project. At a minimum, include:

2.1.8.1 For projects incorporating aerial imagery technologies, include the:

1. Type of acquisition camera/sensor (make/model)
2. Focal length of proposed camera
3. Serial number
4. Area-weighted average resolution (AWAR) value and calibration date
5. Calibration Certificate for all equipment. The date of the calibration certificate must be within three years of the estimated completion of the data collection.
 - a. If using a digital camera, provide the calibration report and/or information regarding the manufacturer's recommended equivalent procedure.

- b. If providing a manufacturer's recommended procedure, include a Statement of Compliance on company letterhead. The statement of compliance certifies completion of the manufacturer's recommended procedure at the recommended intervals, it will identify the date the procedure was last accomplished before the imagery was flown, and be signed by an authorized representative of the company submitting the Statement of Compliance.
6. Detail how imagery from these sensors will be georeferenced, the collection bands to be used, and proposed imagery format.
7. When using a film camera, provide the name and model number of the photogrammetric scanner used to create digital images.

2.1.8.2 For projects incorporating LIDAR technologies, include:

1. The makes, models, serial numbers, and any applicable software version numbers for all equipment the data provider proposes to use in data acquisition.
2. Before and after collecting the data, ensure the calibration of all equipment in the system according to the manufacturer's specifications. Refer to paragraph 5.4 for system calibration requirements.
3. There is no standard format for the calibration reports, but they must contain, at a minimum:
 - a. The date the calibration was performed.
 - b. The name of the person, company, or organization responsible for performing the calibration.
 - c. The methods used to perform the calibration.
 - d. The final calibration parameters or corrections determined through the calibration procedures.
 - e. A discussion of the results.
4. Provide the maintenance history of the sensor for acquiring LIDAR.

2.1.8.3 Satellite Imagery Requirements. Reserved for later implementation.

2.1.9 Control Point Requirements.

2.1.9.1 **Remote Sensing Projects.**

All remote sensing projects require some type of survey control to register or georeference the data to the National Spatial Reference System (NSRS). If airborne GPS procedures are integrated into the flight mission, make sure to reference this in the remote sensing plan. In this section of the plan, describe the ground control network proposal, including

characteristics such as panel point, photo identifiable, and others; locations; and expected accuracy of measurements in horizontal and vertical axis (stated as accuracy RMSE in feet).

2.1.9.2 Station Location and Visibility Diagram.

Develop and provide a Station Location and Visibility Diagram for each control point using the form available in the Surveyors' section of the Airports GIS Web site at <http://airports-gis.faa.gov>. Include on the form a sketch of the area surrounding the control point.

2.1.9.3 Take Digital Photos.

Take digital photos of the station as prepared to support the data acquisition. Electronically add to the photo a caption to uniquely identify it. (See [Figure 2-5](#).) Include the filenames of the digital images for the station in the sketch section of the appropriate Station Location and Visibility form.

2.1.9.4 KML File.

Include an active KML file showing control points supporting the data acquisition for the project area. See [Figure 2-1](#).

2.1.9.5 Control Points.

The number and placement of the control points must be sufficient to georeference the imagery within the accuracy requirement necessary to meet the purpose of the project. A good control point is a very small, recognizable, and symmetrical photographic image with distinct boundary of a relatively high to a lower contrast. Some examples of "well-defined" control points are:

1. A point at well-defined junctions of intersecting features such as sidewalks, abutments, and roads.
2. Corner points of any clear, well-defined feature such as a parking lot, a tennis court, or a road intersection.
3. An easily identifiable pre-marked or paneled point on the imagery.

There is no minimum number of control points. Use the National Geodetic Survey (NGS) Online Positioning User Service (OPUS) to determine point positions. See paragraph [2.1.9.12](#).

[Figure 2-5](#) shows a sample digital photograph of an imagery control point with the antenna located over the point. Note the caption added to the photo to identify the point.

Figure 2-5 Sample Digital Photograph of Imagery Control Point with Antenna



2.1.9.6 Data Providers Proposing to use TLM Scanner Systems.

Data providers proposing to use TLM scanner systems to scan airports must provide a data sample demonstrating the reflective properties of targets they propose to use in the survey. There are basically two types of LIDAR targets: object targets and reflective targets.

2.1.9.7 Object Targets.

Object targets are objects of known size. Commercially the most common type is a plastic sphere. Constructing cylindrical targets using reflective tape applied to pipe for a registration cross is another appropriate target. Once captured by the scanner system, the point cloud data for the target is fit to the appropriate geometry. The quality of the fit is a measure of the accuracy of the target at a particular range.

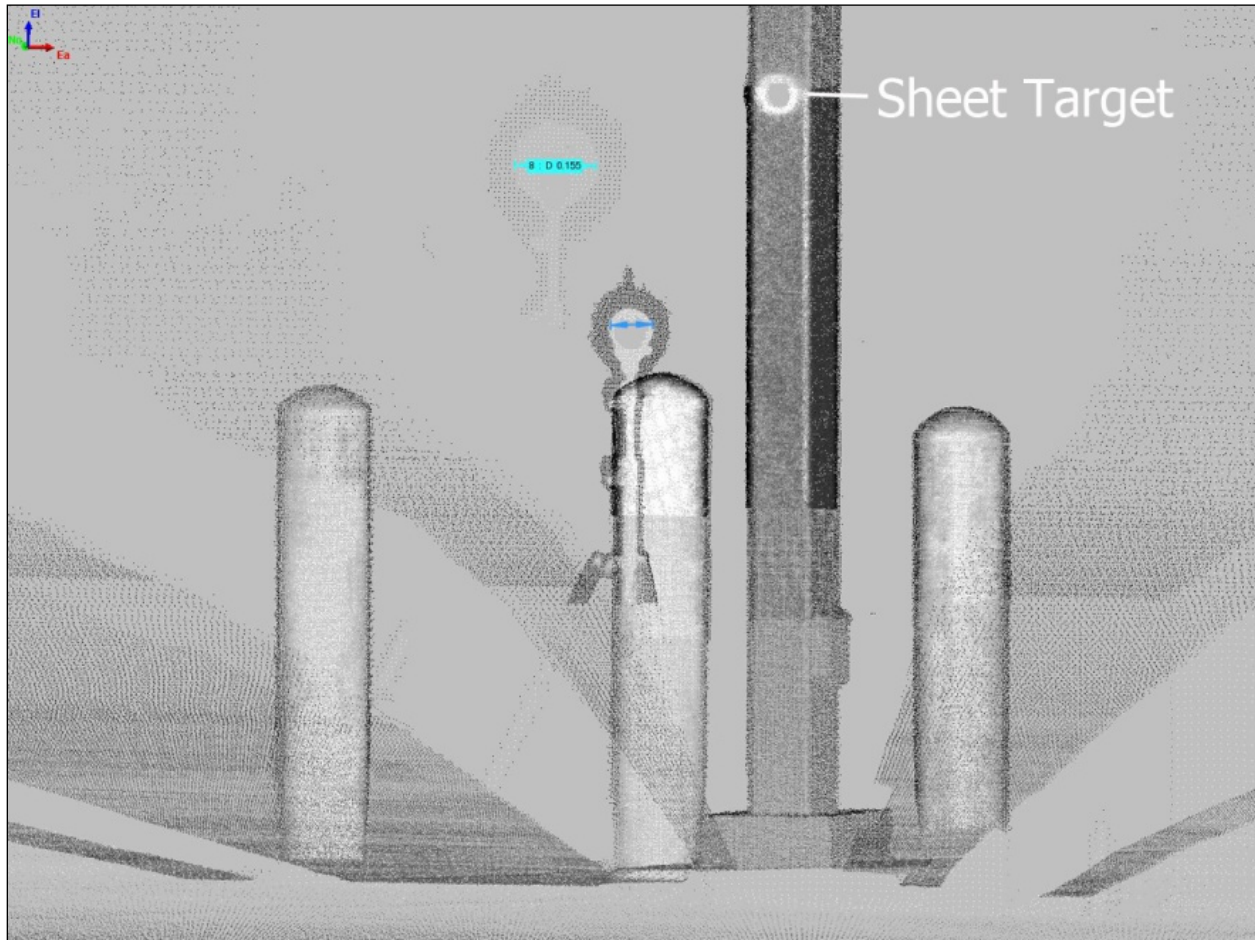
Figure 2-6 shows a typical shape target for LIDAR surveys. The material is a durable, rough-finish, solid plastic sphere machined into a 6-inch sphere with a mounting bolt for attaching it to a standard 5/8-inch survey tripod.

Figure 2-6 Typical Shape Target in LIDAR Surveys for MCLM or TLM**2.1.9.8 Reflective Targets.**

Reflective targets are commercially available for most systems. These targets are two-dimensional, come in a variety of sizes, and are constructed of a reflective material and applied directly to a feature. Not all reflective sheet targets are ideal for some systems. Standard sheet targets commonly used for laser theodolites may reflect too strongly resulting in a saturated reading by the scanner system (see [Figure 2-6](#)). Using an inappropriate target results in a bright halo defined by the edges of the target and loss of data at the target center. This gap makes it less than ideal for precise georeferencing of the point cloud data set.

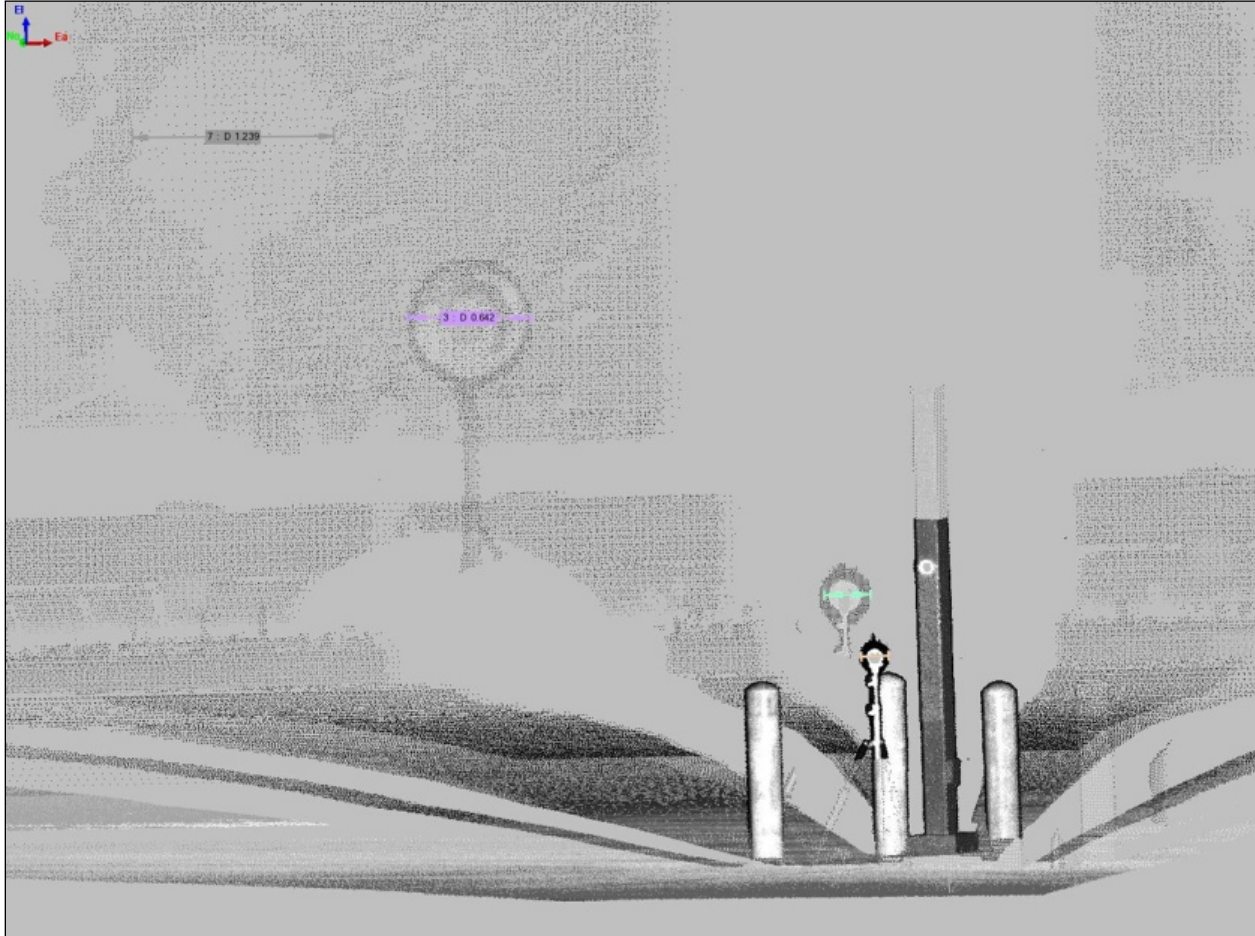
[Figure 2-7](#) shows the LIDAR response of highly reflective sheet target mounted to the side of a light post.

Figure 2-7 LIDAR Response of Sheet Target Mounted to a Light Post



- 2.1.9.9 Demonstrate that both target types are reflective to the scanner of choice. Many manufactures provide some type of LIDAR target, but any object of regular, known dimensions is acceptable as long as it is reflective to the wavelength of laser. Lasers in the 900 or 1500 nanometer wavelength (near IR) will reflect accurately off of any material that is light in color, non-polished, and non-hydrated.

Figure 2-8 illustrates the potential data shadowing if a traditional survey prime is used as a LIDAR target.

Figure 2-8 Potential Data Shadowing if Survey Prime is a LIDAR Target**2.1.9.10 Do Not Use Surveying Prisms as LIDAR Targets.**

The orthogonal mirror geometry produces a measurement error for any incident beam that does not strike the exact center of the target. Secondly, the highly reflective target material will capture any beam that overlaps the target producing data shadow behind the target that becomes larger with beam divergence.

2.1.9.11 ASCII Text File.

Provide an ASCII text file of the final imagery control point values identifying any changes from the remote sensing plan. This section should only include the coordinate values (easting, northing, elevation) for the photo control.

Table 2-1 illustrates an ASCII Control Point file ready for submission.

Table 2–1 ASCII Control Point File

Imagery Control Point Coordinates — Sample				
Airport Name _____				
Coordinate System _____				
UTM Zone _____				
(The state plane coordinate system in which the Airport Reference Point is located may also be used.)				
Reference Ellipsoid _____				
Horizontal Reference Frame: NAD 83 (CORS 96)				

Vertical Reference Frame: NAVD 88 (Geoid)				

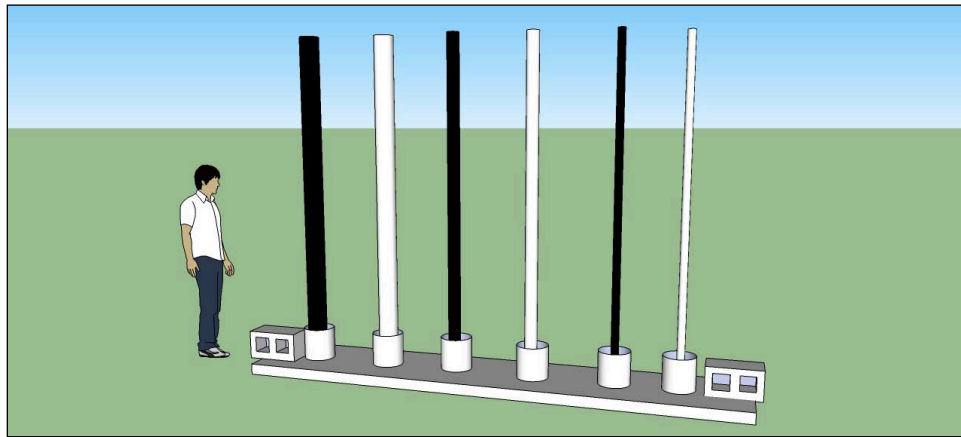
All heights are in feet.				
Station Name	Northing	Easting	Orthometric Height	Ellipsoidal Height
P01	2086849.62	3579322.68	115.48	83.34
P02	2086905.37	3583818.97	78.47	46.29
P03	2092134.98	3584776.85	93.59	61.45
P04	2093245.00	3586869.35	97.09	64.94
P05	2089958.84	3591583.70	88.78	56.53
P06	2084575.11	3596417.02	51.81	19.39
P07	2080281.03	3598531.32	12.47	-20.02
P08	2075655.30	3602180.66	3.04	-29.52
P09	2075499.76	3599408.29	11.76	-20.77

2.1.9.12 **Include NGS OPUS Check Points.**

All technologies require providing additional control point locations for use as “check points.” The purpose of the check point when using remote sensing technologies is to provide an additional means of verifying the georeferencing of the data. In addition to control points, collect and provide additional check points within the project area. The check points are used for independent check of accuracies and consist of NGS OPUS points. Use the NGS OPUS to determine point positions. Do not use these check points as part of georeferencing solution for the data. Submit a copy of the NGS OPUS and GPS solution for each check point.

1. For aerial imagery technologies, provide at least five check points within the project area. Provide a separate table describing the check points similar to the points used to georeference the imagery. (See Figure 3-1.)

2. For LIDAR data projects:
 - a. When using temporary or non-permanent survey ground control within or near the airport as airborne GPS base stations, tie to at least three NGS first-order horizontal and vertical and third-order or better vertical control. Constrain the stations in a 3D network adjustment to determine final positions and ellipsoid elevations. Submit GPS data to NGS OPS to verify the results of the network adjustment. Determine length of occupation using the standards in AC 150/5300-16, General Guidance and Specifications for Aeronautical Surveys: Establishment of Geodetic Control and Submission to the National Geodetic Survey.
 - b. ATLM technologies require field test apparatus.
 - i. Construct the Field/Test of the following materials and methods:
 - A base plate or tube approximately 10 feet long.
 - Two (2) 6-inch diameter sections of PVC pipe, 8 feet long, (1) black-color, (1) white-color.
 - Two (2) 4-inch diameter sections of PVC pipe, 8 feet long, (1) black-color, (1) white-color.
 - Two (2) 2-inch diameter sections of PVC pipe, 8 feet long, (1) black-color, (1) white-color.
 - Mount all the PVC sections vertically to the base plate/support as plumb as possible. Equally space the sections across the base plate. The base plate must have two marks, one at each end, to geographically locate the test apparatus once installed in the survey area.
 - ii. Figure 2-9 shows an example of the Field/Test Apparatus deployed in survey area, stabilized by supports and weights. (Source: “Light Detection and Ranging (LIDAR) Requirements” at the FAA Airports GIS website, <https://airports-gis.faa.gov/public/surveyorsIntro.html>.)

Figure 2-9 Field/Test Apparatus Deployed in Survey Area and Stabilized

Using an appropriate GPS Survey grade receiver, collect the position of each end of the support base and process the information through the NGS OPUS to derive the geographic position of each end of the support.

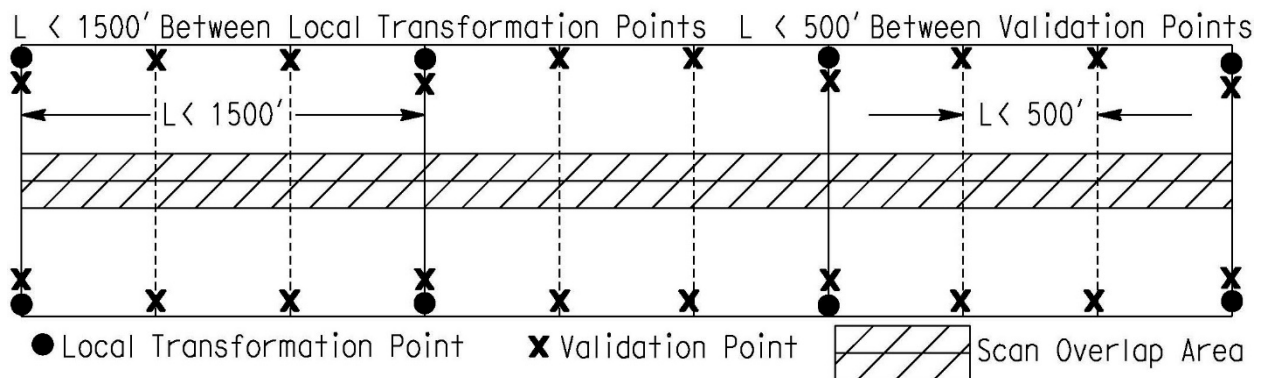
- iii. **Field/Test Apparatus Placement.** To the extent practicable, locate the test device in a safe, flat, level terrain, an open area, within a normal section of the survey area. Support the structure if necessary by sand bags or other means so the device will remain erect and stable in windy conditions and so it is not prone to be toppled by passers-by or others.
- iv. **Flight Regime.** The data provider must use the standard mission parameters planned for the survey. Do not collect extra data of the test apparatus location, and do not use any special maneuvers or flight lines to enhance or “densify” data distribution on the test object.
- v. **Field/Test Apparatus data analysis.** Provide the results to the FAA (including the NGS OPUS results for the positioning of the end points) for analysis of the test data to determine the following:
 - The number and intensity of LIDAR returns from each vertical component of the device
 - The vertical point spacing/density
 - The differences between the known top elevations of the vertical components of the device and the elevations determined from the LIDAR data
 - The ability to resolve each vertical component

2.1.10 MCLM Technologies.

For MCLM technologies, incorporate targets occupying known horizontal and vertical control in MCLM surveys to serve as known control points for point cloud adjustment and validation points for QA/QC. Bracket the scanned area on both sides of project area with scan control (local transformation) points at a maximum of 1500-foot stationing intervals. Validation points should be on both sides of the scanned area, at centerline stationing intervals not exceeding 500 feet for Type A MCLM surveys (see [Figure 2-10](#)). Type B MCLM control and validation points should be placed at a maximum of 2400-foot and 800-foot stationing intervals respectively. Targets should be located as close to the MCLM vehicle path possible without compromising safety. The MCLM vehicle operators should adjust the vehicle speed at the target area so that the targets will be scanned at sufficient density to ensure good target recognition.

[Figure 2-10](#) depicts the typical MCLM Type A local transformation and validation point layout.

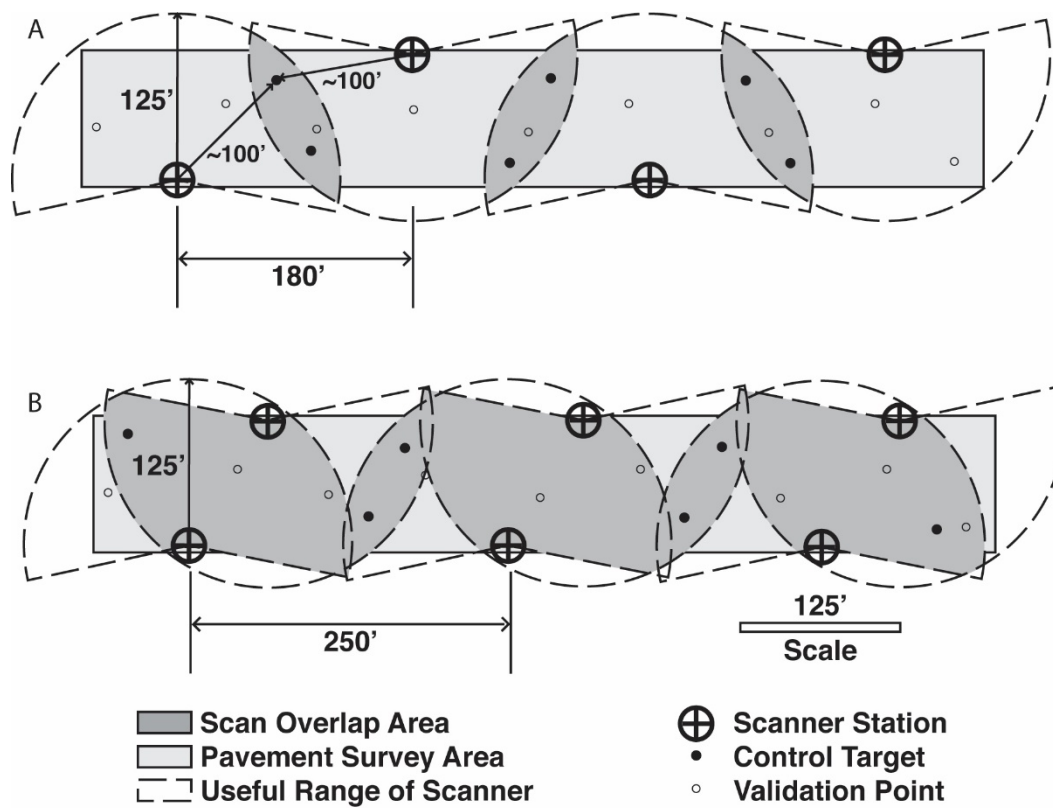
Figure 2-10 Typical MCLM Type A Scan Control Points Layout



2.1.11 TLM Technologies.

For TLM technologies, locate control points prior to the scanning campaign. Because the acquisition geometry of TLM scanner systems is radial, it is necessary to establish control points lying within regions of overlap between scanner stations and position reflectors to the geometry (See [Figure 2-9](#)). The scanner stations can also be used as part of the survey control by either replacing the scanner head with a survey target or collecting the position of the station using a total station or by using a high-quality GPS solution for the scanner station. TLM systems should use Real Time Kinematic (RTK) GPS base stations where appropriate to maintain/increase resolution and precision.

[Figure 2-11](#) illustrates typical setup for scanner locations, target locations, validation point locations, areas of overlap and usable scanner range. This example depicts a high-density pavement survey plan requiring 0.00125 degree step size at 125 feet. Note some scanners have dynamic spacing that is uniform at any range, which greatly reduces data acquisition time and number of points collected. Regions of low-no overlap should have adequate spacing because these areas are within 75 feet of the scanner station.

Figure 2-11 Scanner Setup Locations**2.1.12 Georeferencing Requirements.**

In this section of the plan, describe the techniques and tools (including software and version number) planned to be used in georeferencing the data. Include a statement of expected accuracies and associated tolerances derived from the proposed georeferencing methodology.

2.1.13 Remote Sensing Schedule.

Provide a generalized schedule identifying anticipated acquisition dates and times, delivery of required data, orthoimagery and other information to the FAA.

2.1.14 Quality Assurance and Quality Control (QA/QC).

Describe the methodologies for ensuring the quality of the data and all associated information such as forms, digital photos, and other information. Discuss the processes and procedures for ensuring the data is checked, complete, reliable, and meets the accuracy requirements (including error analysis) for the type of project.

2.2 How to document the location of a proposed runway extension in aerial imagery?

Locate the design runway end location using GPS methodologies, then construct a standard photogrammetric panel at the location and document it as any other control point. Post-process the location using NGS OPUS and provide a separate ASCII control point file containing the appropriate information. Ensure the header information of the ASCII file clearly identifies the information as pertaining to the proposed runway end location when the proposed runway ends (either a new runway or extension) have been identified.

2.3 What are the requirements for horizontal and vertical ties to the NSRS?**2.3.1 Imagery Control Points.**

Tie all imagery control points to the NSRS horizontally using the North American Datum of 1983 (NAD 83) and vertically using the North American Vertical Datum of 1988 (NAVD 88). Use the most current adjustment of NSRS. In Alaska and other areas outside the continental United States where NAVD 88 benchmarks are not available, contact the NGS for further guidance.

2.3.2 Accuracy Requirements.

The accuracy requirements for image photo control points must meet a minimum accuracy of 1 foot (0.3 meters) horizontally and a vertical accuracy of 4 inches (0.1 meter) relative to the NSRS.

2.4 Can LIDAR be used to perform an obstruction analysis?

As a fairly recent and rapidly evolving technology, the FAA limits the use of LIDAR in collecting airport data and requires certain additional requirements and supporting documentation. Use LIDAR technologies (specifically ATLM) in the capture of data *supporting* an obstruction analysis. See more about using LIDAR in obstruction analysis in the section and Figure 5-1.

2.5 Can LIDAR be used to collect airport features that are non-airport related?

The FAA is continuing its research into the use of LIDAR in airport data collection and will update these standards as appropriate. LIDAR mapping can collect many airport data elements listed in AC 150/5300-18, General Guidance and Specifications for Submission of Aeronautical Surveys to NGS: Field Data Collection and Geographic Information System (GIS) Standards. However, any single technology will not capture all feature classes. Key factors include the view angle and the positional accuracy requirements for individual features. For example, ATLM can detect with sub-meter precision objects viewed from the nadir position, vertical surfaces or objects smaller than 12 - 16 inches that cannot be adequately captured. Supplemental data collection will always need a separate sensor or other surveying technologies. Furthermore, any LIDAR-acquired data require post-processing interpretation, and the FAA must pre-

approve any automated feature class recognition software. Always check the results for accuracy by comparing digital photography, or by spot-checking results at the survey site, or both.

2.6 What are the data delivery requirements for remote sensing projects?

Deliver all data on removable media such as a DVD clearly labeled with the following information: Project Name, Collection Dates, Data Provider's name, and disk Contents. Deliver all project removable media to the FAA at the following address.

FAA Airport Engineering Division (AAS-100)
Airport Surveying-GIS Program Manager
800 Independence Avenue, Suite 621
Washington DC, 20591
Front Office Phone: 202-267-7669

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CHAPTER 3. AERIAL IMAGERY SPECIFIC STANDARDS AND RECOMMENDED PRACTICES

3.1 What is the timeframe for imagery acquisition?

Acquire imagery for use in planning, design, construction, or analysis activities within 6 months of intended use. This short time frame ensures the imagery accurately depicts the environment on and surrounding the airport. The decision for extending the use of aerial imagery beyond the 6-month period should be made by the ADO with the airport sponsor's input. Generally, if vegetation growth or construction surrounding the airport is not significant or can be accounted for, re-using the imagery at 7-12 months is acceptable. The decision rests with the local ADO and airport sponsor who know the airport and environment best. For design data being collected to support instrument flight procedure development, re-using imagery of up to three years old is acceptable. New imagery must be collected during the as-built phase of the project.

3.2 Capture the imagery in a leaf-on or leaf-off condition?

The determining factor of leaf-on or leaf-off condition in the acquisition of the imagery is a function of the project. If the project requires an Airport Airspace Analysis, then full leaf-on conditions are required. If the purpose of the imagery supports a project (such as an engineering or planning project) not requiring Airport Airspace Analysis, then the acquisition in a leaf-off condition is acceptable. The data provider should identify in the remote sensing plan if the imagery collection is leaf-on or leaf-off. This may require flights in both conditions to meet the project objective, however in most cases flying leaf-on is the preferred condition. The FAA reserves the right to accept or reject the data.

3.3 What are the equipment and supplies requirements when using aerial imagery technologies?

The consultant should take care in proposing the camera. All cameras are different, and poor quality image features and/or inability to accurately measure features will be cause for rejection and possible re-flight.

3.3.1 Frame (Film-based) Cameras 230 millimeters.

A single lens metric camera must provide an equivalent resolution to or exceeding the capabilities of a Wild RC 30 or Z/I Imaging RMK-TOP 15, with forward motion compensation. The lens must have an area-weighted average resolution (AWAR) of at least 85.0 lines per millimeter. An AWAR ≥ 100 is preferred.

3.3.2 Aerial Film.

Use high-resolution color negative aerial film to obtain aerial imagery. Film types such as Kodak 2444 or AGFA Aviphot X-100 are preferred for their proven ability to consistently meet quality and contract requirements. The low-contrast target resolution

of color negative emulsions must be rated at greater than or equal to 80 line pairs per millimeter (lp/mm). Emulsion and filter combinations selected must be sensitive to and record on the film the green, yellow, orange, and red hues of the tree leaf canopy.

3.3.3 Photogrammetric Scanner.

Convert aerial film to raster imagery on photogrammetric scanners with high geometric accuracy capable of producing scan pixel size of 15 microns or less. Identify the type of scanner that will be used in the remote sensing plan.

3.3.4 Large-format Frame Digital Cameras.

Digital cameras with high quality frame imagery such as the Zeiss DMC, Ultracam, DDS, or equivalent system are permissible but require approval on a case-by-case basis. The sensor must be geometrically stable and have a calibrated system suitable for high-accuracy photogrammetric mapping. The sensor must provide a high enough resolution and have a large Field of View (FOV) to meet the project requirements.

3.3.5 Flight Planning.

3.3.5.1 **Flying Height.**

The flying height must provide the appropriate level of image resolution. Proposed flying heights must not exceed 12,000 feet above ground level (AGL) for film-based systems using a 6-inch focal length camera. Choose the flying height for a digital camera system to produce an image resolution and quality greater than or equal to the resolution specified in Table 3–1. The flying height variation must not exceed 2 percent below or 5 percent above the flying height. Table 3–1 identifies recommended and maximum flying scales for film based systems at different pixel resolutions.

Table 3–1 Map Accuracies as a Function of Photo/Map Scale

Specification	Recommended	Maximum	Recommended	Maximum	Recommended	Maximum
Pixel Resolution	0.25 foot	0.25 foot	0.5 foot	0.5 foot	1 foot	1 foot
Film Scale	1 inch = 300 feet	1 inch = 440 feet	1 inch = 600 feet	1 inch = 880 feet	1 inch = 1200 feet	1 inch = 1720 feet
Flying Height	1800 feet	2700 feet	3600 feet	5400 feet	7200 feet	10800 feet
Scan Resolution	21	14	21	14	21	14

3.3.5.2 Image Overlap and Sidelap.

For frame imaging systems, the forward overlap must average 60 percent between consecutive exposures, while forward overlap must not be greater than 68 percent or less than 55 percent in any pair of consecutive images. Planning for the appropriate sidelap normally equates to 50 percent overlap for a film-based system with the acceptable range being 30 to 60 percent. A sidelap of 50 percent is recommended to ensure objects are seen in stereo from multiple views. 50 percent sidelap is critical in forested areas.

3.3.5.3 Flight Line Navigation.

All flight lines should be continuous and not broken or patched. If a line requires a second flight, it must have the original flight line number.

3.3.5.4 Tilt.

Ensure the tilt (departure from the vertical) of the camera is kept to a minimum. Tilt must not exceed ± 3 degrees for any photographic frame. The average tilt for the entire project must not exceed ± 1 degree.

3.3.5.5 Crab.

Ensure the imaging system compensates for the crab of the aircraft, with a resultant error not exceeding ± 5 degrees, as measured from the average line of flight with a differential between any two successive exposures not exceeding ± 5 degrees.

3.4 What information to include in the Aerial Photogrammetric Report?**3.4.1 Aerial Photogrammetric (AP) Acquisition Report.**

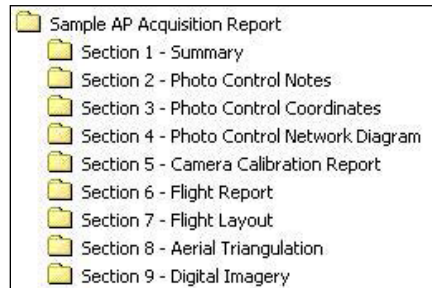
Include in the Aerial Photogrammetric (AP) acquisition report the raw stereo imagery (and accompanying set-up files) and supporting documentation. Submit the raw stereo imagery and AP acquisition report to the FAA after completing the flight mission but prior to the delivery of any associated survey data. Provide the AP report and supporting deliverables in the following directory structure. ("Raw stereo imagery" has not been georeferenced or corrected through a rectification process and subsequent digital artifact clean-up. The FAA reviews the imagery straight from digital scanning, which should be a 1:1 procedure.)

3.4.2 Directory Structure.

Establish the directory structure (see [Figure 3-1](#)) so each folder contains pertinent information about each item. Name the root directory using the assigned FAA airport location identifier, for example, BOS AP Acquisition Report for Boston Logan International Airport. Develop and include a table of contents for each submission and

store it in the root directory. Figure 3-1 illustrates a sample directory structure for an AP acquisition project.

Figure 3-1 Sample Directory Structure for an AP Acquisition Project



3.4.3 Upload AP Acquisition Report.

Upload all sections of the AP Acquisition Report with the exception of Section 9 – Digital Imagery, to the FAA using the Airports GIS Web site. Deliver Section 9 – Digital Imagery and associated supporting documentation for review at the following address.

FAA Airport Engineering Division (AAS-100)
Airport Surveying-GIS Program Manager
800 Independence Avenue, Suite 621
Washington DC, 20591
Front Office Phone: 202-267-7669

3.4.4 Provide Additional Information.

The following sections provide additional information regarding each section.

3.4.4.1 **Summary.**

Provide a summary of project details and any deviations to the remote sensing plan content. Add any supplemental information considered to be useful or explanatory for use by FAA in reviewing the usability of the imagery. Add comments (required) when the project is completed differently than identified in the remote sensing plan due to unusual circumstances or problems, equipment malfunctions, changes to proposed methodologies/equipment or any deviations from these specifications.

3.4.4.2 **Photo Control Notes.**

Provide an image control point and check point photographs (properly labeled and matching the point listing in the ASCII text file supplied), location/visibility sketches and a copy of the NGS OPUS solution for the five check points not used in the Aerial Triangulation process.

3.4.4.3 Photo Control Coordinates.

Provide an ASCII text file of the final imagery control point values identifying any changes from the remote sensing plan (as illustrated in Table 3-2). This section should only include the coordinate values (easting, northing, elevation) for the photo control and the five NGS OPUS check points.

Table 3-2 depicts a sample ASCII Imagery Control Points File.

Table 3-2 Sample ASCII Imagery Control Points File

Imagery Control Point Coordinates—Sample				
Airport Name: _____				
Coordinate System: _____				
UTM Zone: _____				
(The state plane coordinate system in which the Airport Reference Point is located may also be used.)				
Reference Ellipsoid: _____				
Horizontal and Vertical Datum: _____				
All heights are in feet.				
Station Name	Northing	Easting	Orthometric Height	Ellipsoidal Height
P01	2086849.62	3579322.68	115.48	83.34
P02	2086905.37	3583818.97	78.47	46.29
P03	2092134.98	3584776.85	93.59	61.45
P04	2093245.00	3586869.35	97.09	64.94
P05	2089958.84	3591583.70	88.78	56.53
P06	2084575.11	3596417.02	51.81	19.39
P07	2080281.03	3598531.32	12.47	-20.02
P08	2075655.30	3602180.66	3.04	-29.52
P09	2075499.76	3599408.29	11.76	-20.77

3.4.4.4 Photo Control Network Diagram.

Provide a KML (see Figure 2-1) depicting all control stations the data used in georeferencing the imagery including information regarding their tie to the NSRS.

3.4.4.5 Camera Calibration Report.

Provide a copy of the camera calibration report for the camera used during the acquisition of the photo mission. Provide a complete listing of the equipment the contractor proposes to use in the survey, including model


and serial numbers, specifications, calibration reports, and equipment maintenance reports for the field (aerial) and office equipment and software used. The contractor must provide the appropriate Calibration Certificate for the aerial cameras proposed for use in the project. This calibration certificate must be dated within three years of the estimated completion of the collection. If using a digital camera, provide the calibration report and/or the manufacturer's recommended equivalent procedure. If a manufacturer recommended procedure is provided, a Statement of Compliance on company letterhead will be submitted. The statement of compliance must:

- Certify the manufacturer's recommended procedure was completed at the recommended intervals as required.
- Identify the date the procedure was last accomplished before the imagery was flown.
- Get it signed by an authorized representative of the company submitting the Statement of Compliance.

3.4.4.6 Flight Report.

Provide an Imagery Flight Report using the Photographic Flight Report form available at <http://airports-gis.faa.gov>. Figure 3-2 shows the form. Follow the instructions below to complete it.

Figure 3-2 Photographic Flight Report Form

 Federal Aviation Administration		Airport Name		Airport Surveying-GIS Program									
		Location Identifier		Photographic Flight Report									
Roll Number						Cassette/Magazine Numbers		Feed Takeup					
Emulsion Number						Camera/Drive Unit Number							
Expiration						Mission Number							
Sheet		of				Aircraft							
Type of Film						Pilot							
ASA Index Used						Copilot							
Filter						Photographer							
		Flightline #	Flightline #	Flightline #	Flightline #	Flightline #	Flightline #	Flightline #	Flightline #	Flightline #	Flightline #	Flightline #	
Date	Line Number												
GMT	Local												
Comp Head	Drift												
Add Number	S												
	E												
	B												
Number of Exposures													
Visibility (Statute Miles)													
Clouds (Use Codes at end)													
Temperature (° Celsius)													
Altitude (feet Above Ground Level – AGL)													
Vacuum													
Shutter													
Aperture													
Rheostat													
Endlap (Percentage)													
Number of blanks to start roll													
Meter readings													
Crab													
Tilt													
Sun Angle													
Overlap/Sidelap (Percentage)													
Cloud Cover Codes ☉ = Scattered or Broken ⊕ = High Thin ● = Solid Overcast ○ = Clear													
Remarks													

- Date. Enter the date the film is first loaded into the cassette of magazine. Print “LOADED” and Date.
- Roll Number. Enter year, camera system designator, film type (CN = Color Negative), and sequential roll number for that calendar year.
- Emulsion Number. Enter the number taken directly from the film can upon loading.
- Expiration Date. Enter the date taken from film can upon loading.
- Sheet Number. Enter X of Y sheets. For example, 1 of 4, 2 of 4, and so on.
- Film Type. Enter color, color negative, other.
- ISO Index. Enter film speed actually used (not EAFS from film can).
- Filter. Enter wavelength of filter used, in nanometers.
- Cassette/Magazine. Enter feed and take-up cassettes or magazine identification number.
- Camera/Drive Unit Numbers. Enter camera identification number or lens serial number/drive unit number.
- Mission Number. Enter aircraft type. For example, Cessna Citation II.
- Aircraft. Enter aircraft tail number. For example, N52RF.
- Pilot. Print the surname.
- Copilot. Print the surname.
- Photographer. Print the surname.
- Date and Line Number. Enter date of photography (month, day, year), flight line number (30-002, indicating a scale of 1:30,000 and Line No. 2). Add note “NEW DAY” to indicate a date change. Place near the date entry.
- GMT/Local Time. Enter Coordinated Universal Time or GMT in hours and minutes and associated local time.
- Comp Head/Drift. Enter the magnetic heading in degrees/variances in degrees left or right of the path of the aircraft and ground tracking over the planned flight line.
- Add Numbers. Enter the first and last frame numbers of the line.
- Number of Exposures. Leave blank.
- Visibility. Enter distance in statute miles out from the aircraft, in the direction of the sun, at which tree crowns are still separately discernable.

- Clouds. Enter an estimate of cloud cover from choices at the end of the photographic flight report.
- Temperature. Enter the temperature in degrees Celsius at the time of the photography.
- Altitude. Enter feet above ground level (AGL) over airports.
- Vacuum. Enter vacuum reading from gauge or from camera display panel (600 millimeter waves, or nominally 64 MB standard).
- Shutter. Enter speed of shutter during line of photography. Enter, if in automatic mode, variances in shutter speeds (450-550).
- Aperture. Enter the actual aperture used. Use the final adjustment from camera indicator, not the base exposure from an automatic light meter.
- Rheostat. Enter the rheostat setting as a function of the ISO. For example, “per xxx ISO.”
- Endlap. Enter the planned percentage of endlap as a whole number: 60, 80, and so on.
- Number of Blanks to Start of Roll. Enter. “6” is standard.
- Meter Readings. Record the automatic light meter readings. For example, 4 @ 1000.
- Crab. Provide the crab angle of the aircraft during acquisition.
- Tilt. Provide the camera tilt (departure from vertical) during acquisition.
- Sun Angle. Record the sun angle at the time of acquisition.
- Overlap/Sidelap. Enter the planned percentage of overlap and sidelap as a whole number: 60, 80, and so on.
- Remarks. Describe the terrain, local ambient conditions, and any abnormalities.

3.4.4.7 **Flight Layout.**

Provide a Flight Layout diagram showing “actual” flight line length, direction of flight, flight line number, exposure numbers on the flight line and any deviations from original flight mission plan. Provide this as an active KML file (see [Figure 2-1](#)).

3.4.4.8 **Aerial Triangulation.**

Provide an ASCII file (as in the [Table 3-3](#)) containing camera focal length and the X, Y, Z, omega, phi, and kappa of each image. Provide the information in the Universal Transverse Mercator (UTM) or State Plane Coordinate System, NAD83. Specify the Zone used and include with the submitted file this information:

- Strip Number
- Image Number
- Easting specified in the coordinate system Unit of Measure (UoM) to the hundredth
- Northing specified in the coordinate system UoM to the hundredth
- Orthometric Height specified in UoM to the hundredth
- Omega specified in decimal degrees to six (6) decimal places
- Phi specified in decimal degrees to six (6) decimal places
- Kappa specified in decimal degrees to six (6) decimal places
- Final adjusted Aerial Triangulation report showing RMSE

Table 3–3 shows the proper formatting for submission to NGS of the geo-referencing information for the imagery.

Table 3–3 Sample ASCII Image File

ASCII Image File (Results of the Geo referencing) —Sample

GEO-REFERENCING RESULTS

Header Information: _____

Airport Name: _____

UTM Coordinate Zone: _____

(Must match the system and zone used to report ground image control points)

Reference Ellipsoid: _____

Horizontal and Vertical Datum: _____

Camera Focal Length: _____

Strip #	Image #	Easting (Meters)	Northing (Meters)	Ortho Height (NAVD 88)	Omega	Phi	Kappa
(Angles are decimal degrees; ground to photo)							
1	1	3579254.35	2089643.60	3824.12	-1.201358	1.010730	-68.873243
1	2	3580688.07	2087953.67	3823.95	-1.162651	2.000519	-68.884133
1	3	3582126.18	2086260.81	3829.93	-1.004605	1.100225	-68.882666
2	1	3582017.30	2092108.36	3821.09	-0.006452	0.003406	112.853920
2	2	3583490.60	2090446.64	3833.50	0.009585	1.006764	112.852786
2	3	3584965.37	2088806.15	3825.61	-1.219045	-1.003069	112.846104

3.4.4.9 Digital Imagery.

Provide digital stereo imagery (and supporting set up files) of the area of analysis. The extent of aerial imagery coverage depends on the type of survey the contractor is requested to perform.

3.4.4.10 Imagery Captured on Film.

Provide scanning pixel size value in dots per inch (DPI) or microns (μm). Include a summary of quality assurance test results.

3.4.4.11 Stereo Imagery Deliverable.

All Stereo Imagery must be free of abrasions, blemishes, scratches, tears, and irregularities and conform to the following requirements:

1. Delivery medium. DVD.
2. Stereo Image File Format. (Uncompressed) TIFF (Tagged Image File Format) or VITec Scanner Raster Format. Match the naming and numbering of images to listings in the aerial triangulation report.

3.4.4.12 Station Location and Visibility Diagram.

Develop and provide a Station Location and Visibility Diagram for each control point using the form available in the Surveyors section of the Airports GIS Web site at <http://airports-gis.faa.gov>. Sketch on the form the area surrounding the control point.

3.4.4.13 Digital Photos.

Take a sufficient number of digital photos of the station as prepared to support the data acquisition, as in [Figure 2-5](#). Electronically add to the photo a caption to uniquely identify it. Include the filenames of the digital images for the station in the sketch section of the appropriate Station Location and Visibility form.

CHAPTER 4. DIGITAL ORTHOIMAGERY STANDARDS AND RECOMMENDED PRACTICES

4.1 Data Content Standard.

Develop the orthoimagery using the specifications defined by the Federal Geographic Data Committee (FGDC) in FGDC-STD-008-1999, but do not resample the imagery. Provide FGDC standard FGDC-STD-008-1999 compliant metadata for the orthoimagery.

4.2 Coverage.

Create digital orthoimagery covering the extent of the airport boundary. Use an appropriate tile size. The U.S. Geologic Survey requests 1500×1500 meters.

4.3 Ground Sample Distance.

The orthoimagery must meet the image criteria in paragraph 8.1.1.2. During the orthoimagery production process, the input Ground Sample Distance (GSD) should be smaller than or equal to the final output GSD (DO NOT re-sample the imagery to a smaller GSD than the source imagery).

4.4 Horizontal Positional Accuracy Testing and Reporting.

Horizontal accuracy of orthoimagery must, at a minimum, meet RMSE as outlined in Table 3–1 at the 95 percent confidence level. Positional accuracy will be tested and reported following the guidance in the National Standard for Spatial Data Accuracy (FGDC-STD-007.3-1998). National Standard for Spatial Data Accuracy (NSSDA) certification typically requires a minimum of 20 check points. FAA guidelines for imagery acquisition currently only require six checkpoints.

4.5 Deliverable Requirements.

Develop and provide digital orthophotos for all projects using aerial or satellite imagery technologies. For projects using LIDAR technologies, supplying the point cloud fulfills this requirement.

4.6 Orthoimagery Delivery.

Provide digital orthoimagery conforming to the following requirements:

4.6.1 Delivery Medium.

DVD.

4.6.2 File Image Format.

(Uncompressed) TIFF (Tagged Image File Format).

4.6.3 Submit orthoimagery to:

FAA Airport Engineering Division (AAS-100)
Airport Surveying-GIS Program Manager
800 Independence Avenue, Suite 621
Washington DC, 20591
Front Office Phone: 202-267-7669

CHAPTER 5. LIGHT IMAGING DETECTION AND RANGING (LIDAR) SPECIFIC STANDARDS

5.1 What are the differences in LIDAR technologies in the collection of airport data?

Selecting which LIDAR technology to survey airports requires consideration of a broad range of factors. The following are some considerations.

5.1.1 Using ATLM Technologies.

ATLM considerations include the following factors. There may be others, so use good judgment.

5.1.1.1 **No details below 20 centimeters.**

Most LIDAR instrumentation has a limited ability to capture detail below 20 centimeters, but the method is relatively fast. So LIDAR airborne surveys are ideal for capturing large facilities and defining an accurate framework of airport-related features. However, some features may be below the level of detection for LIDAR.

5.1.1.2 **Limited Look Angle.**

Airborne LIDAR is also limited by the look angle, which may be up to 20 degrees from vertical, looking forward of the aircraft. Some instruments are limited to a strictly nadir view. So airborne surveys benefit from multiple survey passes with differing flight line trajectories and from the integration of ground-based, higher-resolution equipment.

5.1.1.3 **Unpredictable Point Cloud Data Sets.**

Airborne LIDAR scanner systems are subject to errors in pointing, range, and elevation, same as other surveying technologies. Elevation precision for most systems is less than 4 to ~10 inches. Planimetric precision is similar for most systems. However, the technology is limited in the maximum number of points it can collect along and across the flight track. Typical values along track point spacing are ~12 to 16 inches with the across track point spacing being ~12 to 20 inches. Given these values, expect accuracy in the decimeter range for airborne survey point clouds. Moreover, given the minimum spacing of points, a single pass can capture features on the ground where the largest dimension of the object is greater than 28 to 36 inches in size. Flying repeat surveys with multiple look angles can improve capture resolution, but it is difficult to predict the complete representation of features in the final point cloud data set.

5.1.1.4 **Local Conditions and Survey Size.**

Collection and processing times depend highly on the size of the survey area and the distance necessary to mobilize the equipment to the scanning site. Any request to survey an airport should seek a local consultant to

minimize mobilization costs. Processing data also requires a precise GPS base station solution to reference the data. Such solutions are regionally available. When suitable and available, a good mission plan should adopt solutions for nearby permanent GPS solutions.

5.1.2 Using MCLM Technologies.

MCLM considerations include the following factors. There may be others, so use good judgment.

5.1.2.1 **Accuracy Standards.**

The equipment used to collect MCLM data, control the data, and maintain quality control validation points must meet accuracy standards. Most MCLM systems use a time-of-flight approach to make spatial measurements and are accurate to ~2 to 20 inches. These accuracies are sufficient for capturing most airport related features at resolutions of ~4 to 39 inches.

5.1.2.2 **Adequate Compute Time for Large Data Sets.**

Collection and processing times depend on survey design (resolution) and the size of the survey. Resolution data can easily max out most standard desktop configurations. Generate a processed data product for feature documentation. With such large data sets, processing can take days to weeks. For example, using MCLM-acquired data, an eight-mile stretch of freeway can be expected to take approximately 1 week to process into a finished deliverable point cloud and associated Computer-Aided Drafting and Design (CADD) or mesh model representation. Many airports will have considerably more paved surface than this stretch of road. Be sure to include processing time into their schedule.

5.1.3 Using TLM Technologies.

TLM considerations include the following factors. There may be others, so use good judgment.

5.1.3.1 **Single-Pulse or Multi-Pulse Averaging.**

Most TLM systems are highly programmable and operate between two end-member modes: single pulse, or multi-pulse averaging. Single pulse systems are faster but have a larger range error. Multi-pulse systems will make three or more measurements and report an average point value after rejecting any outlier values.

5.1.3.2 **Scanner Mountings.**

Scanners can be tripod-mounted or mounted on a truck or an elevated boom. However, it should be noted, the factory specified accuracies are valid when the equipment is tripod-mounted. Vehicle mounted and boom-

mounted systems are subject to sway or mid-scan movement related to wind, tire expansion, and engine vibration. Most systems are battery or AC powered and are highly portable, although some systems (scanner, power supply, tripod, and accessories) may weigh over 100 pounds. Some systems come with internal GPS receivers with antennas of varying quality.

5.1.3.3 **Scanner Accuracy.**

It is important to note systems with GPS are only accurate to the millimeter level when combined with a local base-station solution. All systems produce XYZ point clouds. Some provide tools for mapping red, green, blue (RGB) values, but these are never as accurate as the LIDAR data itself, meaning some points may not have the correct RGB value assigned. Depending on the design of the instrument, TLM systems have millimeter-to-centimeter scale precision and accuracy and can generate point clouds with sub-millimeter point spacing. In general, more precise systems use a focused beam design, limiting their maximum range. Provide statements of certified accuracy and environmental or setup conditions that may limit scanner accuracy. For example, wood or aluminum tripods tend to expand in sunlight and can change the position of the scanner during scan acquisition.

5.1.3.4 **Processing Times.**

Collection and processing time depends on survey specifications and deliverables. To scan a flat, featureless seven-acre bare earth site at a resolution of three inches typically requires 8 hours to acquire the data with a 3,000 points/second acquisition rate. Processing this same data may require up to 12 additional hours to provide a 6-inch contour digital elevation model based on 25 - 30 million points.

5.2 **What are the basic considerations in using LIDAR to collect airport data?**

LIDAR scanning technologies, regardless of type, are line-of-sight instruments and unable to detect what is not visible to the sensor. Be aware of the scanner's limitations and local terrain variations, and plan the mission to cover any potential gaps or shadows in data coverage.

5.2.1 Multiple Look Angles.

For ATLM systems, multiple flight line orientations may be needed to form a grid survey. For MCLM and TLM systems, reposition the sensor around the object to acquire data from multiple orientations.

5.2.2 Multiple Resolutions and Managing Multiple Data Sources.

ATLM systems can scan geometries at the decimeter scale with centimeter scale precision. MCLM and TLM systems can typically capture data at centimeter scale resolution, and in some cases millimeter scale precision. Data from different sources produces a point cloud where resolution can be tailored based on the desired capture resolutions of individual features within a survey area. Therefore, plan the survey to combine the different data acquisition techniques. Include common georeferenced control points to ensure alignment in the final data set.

5.2.3 GNSS Survey Planning.

Both ATLM and MCLM depend on continuous high-quality GNSS position solutions for georeferencing during collection. As a result, plan data acquisitions during periods of good satellite visibility and low solar and geophysical variability. Or include additional control surveys. TLM is more resilient to poor GNSS availability because they use additional control surveys before or after LIDAR data acquisition.

5.2.4 Point Cloud Processing.

There are a variety of commercial and in-house applications to register, classify, feature-extract, and model data sets. This AC does not attempt a generic overview. Just be sure to record all details of the process as a final deliverable. In some cases, it may be necessary to develop new tools and processes to meet specific project goals. Be sure to document their performance, testing, and quality assurance processes to ensure data quality.

5.3 **Why calibrate LIDAR systems?**

Inadequate calibration or incomplete calibration reports may cause the FAA to reject submitted data. Provide calibration reports for all LIDAR systems used from beginning to end.

Sensor calibration reduces or eliminates systematic errors in the LIDAR data. Specifically, the calibration procedures involve solving for a set of calibration parameters to minimize the mean square error using ground control and data in overlapping swaths. The specific calibration parameters are a function of the optical sensor model, but may include roll, pitch, and range offsets, scanner scale, offset, or higher order polynomial coefficients. If performed properly, calibration ensures the highest possible data accuracy and eliminates artifacts in the data such as discontinuities (vertical jumps) in swath overlap areas near the edges of scan lines, horizontal offsets between positions in data from opposing flight lines. Prepare and document the results of this calibration for each project, or every month, or as dictated by analysis of the data, whichever interval is shortest. Additionally, document any calibration procedure employing software and reports generated by the software. Include a basic description of the software.

Lab-calibrate the airborne LIDAR system if the manufacturer deems it necessary. System calibration in aircraft is needed whenever the sensor is removed/reinstalled, serviced, or whenever data indicates misalignment, range issues, or other errors.

Radiometric Performance/Certification is necessary for all LIDAR scanners. The system electronics process signals at the speed of light and are therefore very sensitive. The sensors commonly use mechanical systems to direct the laser light pulses and require stepper motors capable of changing mirror positions in microdegrees. Both systems can wear and require calibration or servicing on a regular basis. All LIDAR equipment must meet factory certification.

5.4 **What are the system calibration requirements for using LIDAR to collect airport data?**

5.4.1 ATLM Calibration.

Factory calibration of the LIDAR system must address both radiometric and geometric performance and calibration. (It does not eliminate the need for the radiometric qualification test for obstruction surveying.) The following briefly describes the parameter testing according to the manufacturer's test procedures. Some procedures and parameters may be unique to a manufacturer.

5.4.1.1 **Radiometric Calibration (Sensor Response).**

1. Ensure the output of the laser meets specifications for pulse energy, pulse width, rise time, frequency, and divergence for the model of LIDAR being tested against the manufacturer's published specifications.
2. Measure the receiver response from a reference target to ensure the response level of the receiver is within specification for the model of LIDAR system being tested against the manufacturer's specifications.
3. Check the alignment between transmitter and receiver and certify the alignment is optimized and within the manufacturer's published specifications.
4. Measure *TO* response of receiver (that is, the response at the time the laser is fired) to ensure the *TO* level meets the manufacturer's published specifications.

5.4.1.2 **Geometric Calibration.**

Determine rangefinder calibrations including first/last range offsets, temperature dependence, range dependence on return signal strength, and frequency offset of rangefinder electronics. Provide updated calibration values.

5.4.1.3 Scanner Calibration.

Verify the scanner passes accuracy and repeatability criteria. Provide updated scanner calibration values for scanner offset and scale.

5.4.1.4 Position Orientation System (POS) - Laser Alignment.

Check the alignment of the output beam and POS. Also, provide updated POS misalignment angles.

5.4.2 MCLM Calibration.

Scanning equipment used for airport survey applications should have a current certificate of calibration provided from the scanner manufacturer or certified accuracy established by a locally licensed surveyor. Any certification documentation should not be dated older than 6 months prior to instrument deployment date. If a certificate of calibration is provided by a licensed surveyor, the certificate must be accompanied by a description of calibration method, scanner precision and accuracy capabilities, and a description of maximum usable range and related instrument error that is a function of range. Any scanner used for airport survey applications must have a maximum range error of 15 millimeters at 100-meter range and a maximum positional error of 20 millimeters. Errors tend to increase as a function of range: the larger the beam divergence, the shorter the effective range of the scanner instrument. Scanners used for mapping airports should have a maximum divergence of 0.001 degree.

5.5 What are the specific requirements for Airborne Terrestrial LIDAR Mapping (ATLM) sensors?**5.5.1 Data Acquisition Standards.**

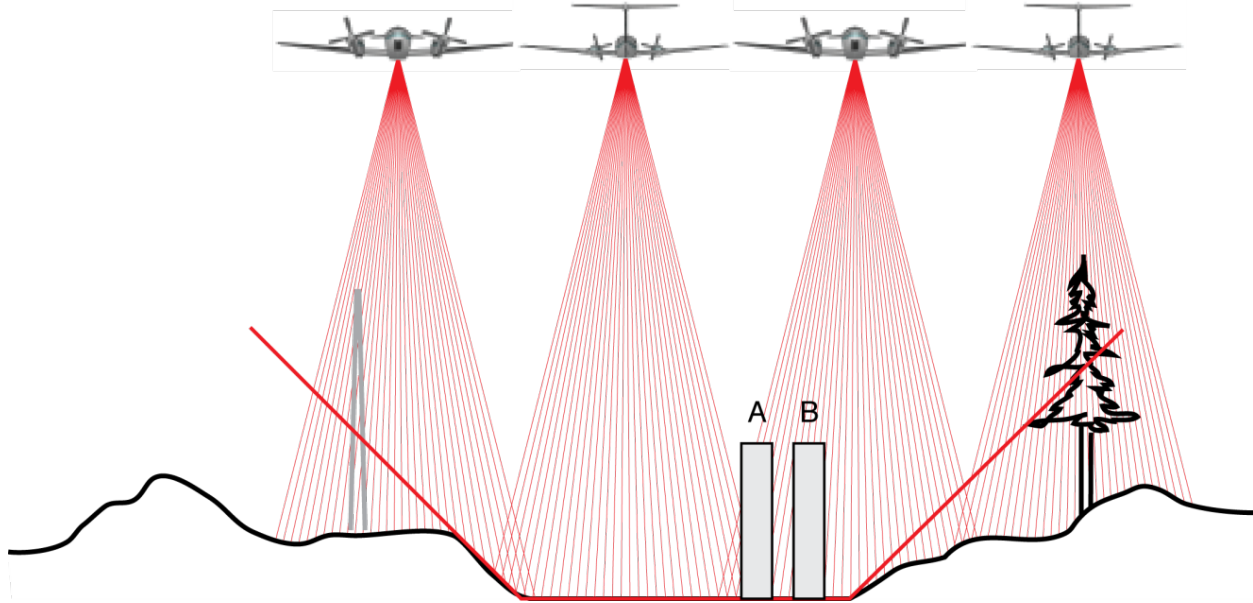
5.5.1.1 Position Dilution of Precision (PDOP) and Vertical Dilution of Precision (VDOP) must be less than 3.

5.5.1.2 Horizontal along-track and across-track LIDAR point spacing must not exceed the limits specified in Table 5-1, which spells out the horizontal and vertical point spacing requirements for ATLM sensors.

Table 5–1 LIDAR Data Acquisition Point Spacing Parameters

Parameter	Spacing
Maximum Across-Track Horizontal Point Spacing	7 inches (18 centimeters)
Maximum Along-Track Horizontal Point Spacing	7 inches (18 centimeters)
Maximum Vertical Point Spacing (Tilted Sensor only)	18 inches (50 centimeters)
Corresponding Point Density	30 points / square meter ¹
¹ Use a helicopter to meet this specification. Flight line overlap should be a minimum of 50 percent each for adjacent flight lines. Parameters are based on aircraft altitude and aircraft speed.	

- 5.5.1.3 Vertical point spacing must not exceed the limits specified in Table 5–1.
- 5.5.1.4 Aircraft bank angle must not exceed 20 degrees.
- 5.5.1.5 Ensure the flying height provides a high-probability of object detection, it is typically desirable to fly as low as possible, within the applicable eye-safety limits. Depending on the airport and the minimum eye-safe altitude, this may necessitate airspace coordination.
- 5.5.1.6 Adjacent swaths must have a minimum overlap of 50 percent of the mean swath width. Overlap should be sufficient to fully capture tall objects within the proposed survey.
- 5.5.1.7 Figure 5-1 illustrates a proposed flight plan failing to adequately capture Tower A. Shift left of the flight line or increase the overlap between flight lines to provide a predictable maximum height of data capture.

Figure 5-1 Proposed Flight Plan Fails to Capture Tower A

- 5.5.1.8 Fly flight lines in either direction. However, fly adjacent, parallel flight lines in opposite directions (reciprocal aircraft headings) to aid in eliminating systematic errors.
- 5.5.1.9 Using LIDAR as a supporting tool in obstacle analysis, fly at least one cross-line (orthogonal to the “primary” flight lines) per runway approach. Cross flights are necessary to align the strips of data.
- 5.5.1.10 Use a special test device within the survey area as a means of validating the system performance. The device will simulate real-world conditions of objects with the potential to intersect and protrude above the Obstruction Identification Surface and should be detectable by the LIDAR sensor. Point-cloud data of the object should appear in the complete data-set. See Figure 2-9 for construction and installation details.

5.5.2 Waveform Digitization.

Full-waveform data is highly recommended. Research by the NGS shows full-waveform LIDAR data provides significantly more information about vertical structures. This extra definition helps detect and recognize objects in the project area. When planning to acquire full-waveform data, submit a description of the waveform post-processing strategy to the FAA as part of the remote sensing plan.

5.6 What are the specific requirements for MCLM sensors?

5.6.1 Redundancy.

Conduct the MCLM data capture to ensure data redundancy. Collect the data with an overlap of at least 20 percent between scans. Allow sufficient time to elapse between successive runs to ensure the satellite constellation accesses at least three different satellites between runs.

5.6.2 Monitoring Acquisition.

Monitoring various component operations during the scan session is an important step in the QA/QC process. The system operator should be aware and note when the system encountered the most difficulty and be prepared to take appropriate action in adverse circumstances.

Monitor the MCLM equipment throughout the data collection to track the following as well as any other factors requiring monitoring:

- Loss of GNSS reception
- Distance traveled with, or time duration of, uncorrected Inertial Measurement Unit (IMU) drift
- Proper functioning of the laser scanner
- Vehicle Speed

5.6.3 Local Transformation Points and Control Points.

Perform a local transformation of the point clouds to increase the accuracy of the collected and adjusted geospatial data. Most common is a least-squares adjustment of the horizontal and vertical residuals between established local transformation points and the corresponding values from the point clouds to produce the transformation parameters of translation, rotation, and scale for the horizontal values and an inclined plane for the vertical values. Apply these parameters to the point cloud for more accurate final geospatial data.

- Type A MCLM surveys require local transformation and control points to have positional accuracies of ≤ 0.03 feet horizontally and ≤ 0.02 feet vertically or better.
- Type B MCLM surveys require local transformation and control points to have horizontal and vertical positional accuracies of ≤ 0.10 feet or better.

Determine accuracies with field verification methods. The feature types and capture size should dictate the appropriate methodology.

5.6.4 Data Processing.

Data processing for MCLM systems is similar to processing airborne LIDAR data. Clean and filter the data to remove errors. Georeference the data based on the IMU and

GPS data collected during acquisition. Then classify the data into applicable classes. As practicable, auto-classify buildings, bare earth, and vegetation. However, because the look angle is horizontal through vegetative cover, the signal may not successfully capture bare earth through thick vegetation.

5.6.5 Data Filtering and Clean-up.

Objects passing through the survey generate isolated points. Remove them. Some errors are only detectable after multiple passes, so make as many passes as needed to ensure reliable error interpretation.

5.6.6 Georeferencing.

MCLM systems come with processing tools to resolve vehicle motion using IMU data collected during acquisition. Record GPS positional data referenced to a base station near the survey area. Report relevant error sources that relate to GPS performance.

5.6.7 Data Integration.

A range of commercial products are available to combine LIDAR data from MCLM sources with LIDAR data from other sources. Large surveys require more time to merge data. Report any data errors as RMSE. Deviations should be within the error ranges for the instrument. For example, if TLM data has a range error of 2 - 3 centimeters, and MCLM data has an error range of 5 - 10 centimeters, then the error for combining the two datasets should not exceed 5 - 10 centimeters.

5.7 **What are the specific requirements for TLM sensors?**

5.7.1 Seasonal Considerations.

TLM signal quality is negatively impacted by vegetation, birds, insects, and other agents passing through or moving within the scan target area. They produce noise in the data that can be filtered. TLM is a line-of-sight instrument. What is not visible to the naked eye is usually not visible to the scanner. Scanning of thick vegetation may not adequately capture survey data. Either remove vegetation or scan during seasonal “leaf-off.”

5.7.2 Establishing Useful Scanner Range.

LIDAR scanners vary by design.

- Some scanners use a laser with focusing optics to improve range-finding precision, but have limited use beyond the designated focal point.
- Some scanners impart a cylindrical design to the laser beam, which improves long-range capability but sacrifices precision.

Both designs are subject to beam divergence, especially cylindrical lasers. As the pulse leaves the unit, the beam becomes wider with range. With an increased laser spot hitting the target, the noise within the return signal increases, resulting in lower intensity and precision. Most manufacturers quote maximum ranges of 250 – 4,000 meters, but outdoor conditions can lower the range by 25 - 50 percent. Be aware of scanner limitations.

5.7.3 Scan Acquisition.

Data acquisition requires the operator to a) place the scanner at the desired station, b) set up the region of interest, c) execute the scan, and d) record metadata. With mobile stop-and-go systems or tripod-mounted systems, collect the back-site orientation and/or GPS coordinates of the scanner. Record additional metadata of the scanner orientation at instrument boot-up, and scan parameters for programming the scan execution.

5.7.4 Georeferenced Data Acquisition.

Most TLM systems are not equipped with back-site telescopes for measuring the orientation of a scanner at instrument start-up (where the zero position for the scanner head at start-up faces the instrument toward the internal reference frame +y-axis).

5.7.5 Data Processing.

Data processing generally begins by downloading data from the scanner and converting it to the proper format for the processing software. Some TLM scanners come with their own processing tools, while others rely on third-party software. Use processing software to edit, clean, merge, and export data point clouds. Some vendors may also have in-house software.

5.7.6 Data Filtering and Clean-up.

TLM data requires, or at least benefits from clean-up. Remove errors from objects passing through the scan area during acquisition.

5.7.7 Scan Merging and Acceptable Error Standards.

TLM data collected from multiple stations can be merged into a common reference frame if the scans overlap. Select scanning stations to account for adequate overlap. Generally, 20 percent overlap is sufficient for coverage.

5.7.8 Data Integration.

Both MCLM and TLM scanner data can be merged with other types of data sets. The precision of differing resolutions is limited by the lower precision and accuracy scan of the two datasets. Although the higher resolution scan gives a more precise image of the scan area, inserting it into a more coarse dataset (for example, an ATLM survey) limits its precision to the coarse survey – unless capturing reference targets in both low- and

high-resolution point clouds. When planning to combine TLM data with other data types, include targets in the TLM scan area that are also visible by other survey methods such as laser theodolite, ATLM, MCLM, and so on.

5.7.9 Applicability of RGB Point Clouds.

Most TLM scanners can combine photographic color collected by an on-board camera with LIDAR point data. The output is an XYZ-RGB data file handy for feature recognition. The procedure for assigning RGB values to the point cloud varies from vendor to vendor; none report any errors in their models. At decimeter-to-meter scale resolutions, errors tend to be negligible. But the resolution of TLM systems may assign many points to a single pixel RGB value that may or may not represent reality. Thus TLM systems often have errors along the edges of objects.

5.8 **What are the data processing standards and recommended practices for using LIDAR in airport obstruction data collection projects?**

LIDAR has trouble capturing small or thin objects easily affected by wind or other movement such as antennas and branches. Take care to apply certain rules when using LIDAR data to analyze airport objects as potential obstructions.

5.8.1 Rule 1: Avoid Data Cleaning and Filtering.

Do not pre-filter data. Using LIDAR to collect and post-process data for airport projects involves analyzing objects as potential obstructions. This process is very different than other end-user applications such as floodplain mapping. Bare-earth terrain modeling involves a lot of filtering/cleaning of LIDAR points very far up the processing chain, or even during data acquisition. Data clearing and filtering is not appropriate for airports, as it can easily remove points corresponding to reflections from obstructions, which might miss the obstructions altogether. **This is why the FAA requires the collection and submission of complimentary imagery supporting the LIDAR.** Deliver the raw LIDAR point cloud (with absolutely no points removed either in the air or in post-processing) as a project deliverable. Be extremely careful about any cleaning or filtering done during data analysis. In general, it is best to leave all points from the LAS file and allow the Object Detection step to handle filtering. Any pre-cleaning/filtering must be described in the report. Additionally, do not remove these filtered points from the LIDAR Data Exchange File (LAS) file. Keep them attributed as “withheld” in the LAS file (classification bit encoding).

5.8.2 Rule 2: Adjust Data Analysis Workflow.

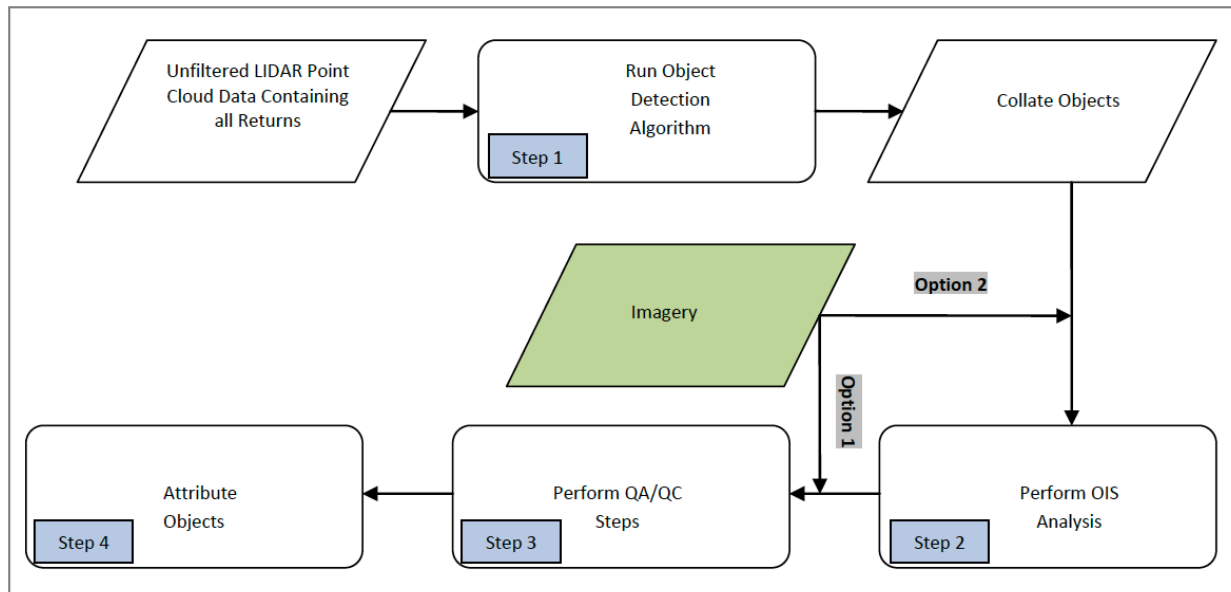
Generating final obstruction data from the LIDAR point cloud is not a trivial task. It requires a tremendous amount of effort and planning. Do not use the raw point cloud as the only source of data for object analysis against an obstruction identification surface. Use it only as a supplementary means associated with aerial imagery. Completing an obstruction analysis in this manner complicates the following tasks:

1. Distinguish between returns from real objects and those due to noise or clutter.
2. Determine which points correspond to laser reflections from the same object.
3. Attribute detected obstructions.

Integrate the steps in [Figure 5-2](#) into the process to reduce or eliminate the possibility of missing objects and increase the probability of detecting objects. One critical aspect is to place the object detection step before the OIS analysis step, so the OIS analysis is run on extracted objects such as, trees, buildings, antennas, poles, towers, and others, rather than on the raw LIDAR points.

[Figure 5-2](#) illustrates recommended steps for analyzing airport objects as obstructions. (Source: “Light Detection and Ranging (LIDAR) Requirements” at the FAA Airports GIS website, <https://airports-gis.faa.gov/public/surveyorsIntro.html>.)

Figure 5-2 Sample Steps for Analyzing Airport Objects as Obstructions



Identify the specific detection algorithms and processes in a step-by-step approach in the appropriate sections of the final report. The workflow in [Figure 5-2](#) is generalized to accommodate different algorithms. It allows for setting the detection threshold very low to minimize the probability of a miss; equivalently, it maximizes the probability of detection.

Using this process requires performing object detection and collation first and then performing the OIS analysis, then object-type attribution and validation using the aerial imagery. False alarms (or “false objects”) due to ground clutter or noise are randomly distributed throughout the project area and typically automatically eliminated in the OIS analysis step, due to not penetrating or falling outside the OIS, and the remainder are easily removed during the image analysis steps. This inherent tolerance of conservative detection thresholds in the object extraction process is an important aspect of the process, since the key consideration in analyzing objects on and surrounding an airport

for their obstruction potential is to avoid missing objects with the potential to jeopardize flight safety.

The use of aerial imagery in using LIDAR as a supplementary source for object analysis is mandatory providing a complementary and independent data source. Data providers can introduce a slight variation on the workflow by introducing the aerial imagery at an earlier step to facilitate a primarily photogrammetric approach to obstruction detection, assisted by LIDAR.

CHAPTER 6. SATELLITE IMAGERY STANDARDS AND RECOMMENDED PRACTICES

6.1 **Reserved.**

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CHAPTER 7. REQUIRED PROJECT DELIVERABLES

In general, submit all data captured in its original unedited format. Submit all supporting forms or documents in a non-editable format such as the Adobe PDF™ format.

7.1 **What deliverables are required for all remote sensing projects?**

7.1.1 Remote Sensing and Survey and Quality Control Plan.

7.1.2 Control Points Information.

7.1.3 Notification of Unusual Circumstances.

Notify the FAA of any unusual circumstances occurring during the performance of this project that may potentially affect the deliverables or their quality. Particularly note any deviation from the project specifications or Statement of Work.

7.1.4 Remote Sensing Final Project Report.

7.2 **What deliverables are required for projects incorporating aerial imagery technologies?**

Aerial imagery projects require the submission of the following items:

1. Control points and associated documentation
2. Camera calibration certificate (as appropriate) or certificate of compliance with manufacturer's procedures
3. Photographic Flight Report
4. Stereoscopic imagery for analysis
5. Digital orthoimagery

7.3 **What deliverables are required for projects incorporating LIDAR ATLM technologies?**

7.3.1 Control Points Information.

If using the test apparatus described earlier, report the proposed location of this target as well as ground control and validation points.

7.3.2 LIDAR Raw Data.

Submit the completed data collection raw output. Formats should include proprietary raw data formats for archival purposes and standard formats that include unfiltered, filtered, and classified point clouds.

7.3.3 LIDAR Products.

Required products include LIDAR point cloud files, intensity images, attributed objects/obstructions and other products described in the appropriate advisory circulars.

7.3.4 Imagery.

Deliver imagery according to the standards outlined in paragraph 7.2.

7.3.5 Flight Reports.

Submit the completed, original LIDAR Flight Logs with the data for FAA review.

7.3.6 GPS Files.

For Global Positioning System (GPS) / IMU files, submit the original, raw data files and processed trajectory files for FAA review.

7.3.7 Airborne Positioning and Orientation Report.

Submit raw GPS and IMU data (in the manufacturer's format) with the final processed GPS trajectory and post-processed IMU data. Also submit a report covering the positioning and orientation of the LIDAR.

7.3.8 Range and Scanner Angle Files.

Submit the original, raw data files for FAA review.

7.3.9 GPS Check Points.

Submit an organized list of all GPS points used for the project as base stations and check points. Indicate which GPS points are pre-existing ground control and which stations are new and positioned relative to the NSRS.

7.3.10 Calibration Reports.

There is no standard format for the calibration reports. However, they must contain, at a minimum, the following information:

- Date the calibration was performed
- Name of the person, company, or organization responsible for performing the calibration

- Methods used to perform the calibration
- Final calibration parameters or corrections determined through the calibration procedures.
- Discussion of the results
- Sensor Maintenance Reports with maintenance history of the sensors used in data collection

7.4 **What deliverables are required for projects incorporating LIDAR MCLM technologies?**

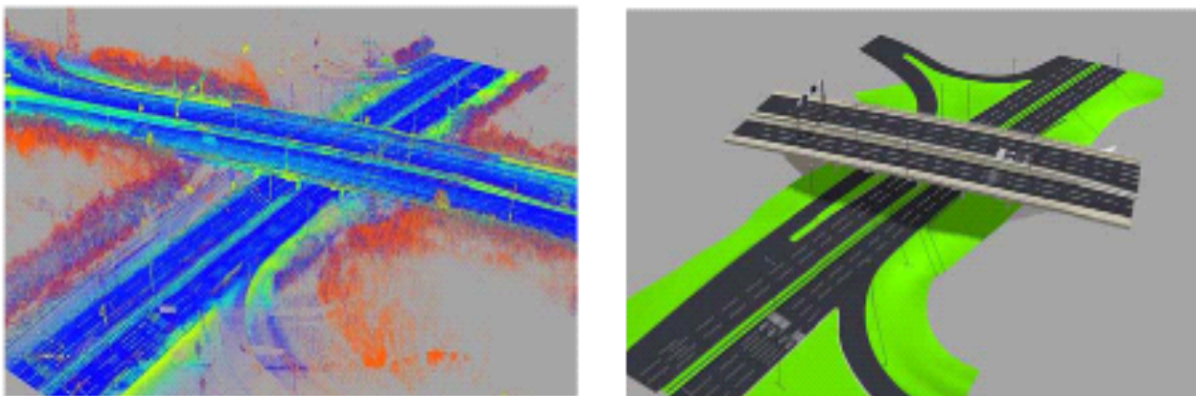
Deliverables from a mobile scanning project should be specified in the contract with the provider.

- If delivering a point cloud, allow considerable office time to extract data in a CADD/ DTM format. Resources must be supplied for data extraction such as computers, programs, and trained personnel.
- If delivering a finished CADD / DTM file, the client's office time will be reduced to QA of the final product.

The simplest form of processed LIDAR data is a "point cloud" saved in a scanner-specific format or an ASCII file containing XYZI (X, Y, Z coordinates and return intensity (I) values) georeferenced data. If image overlay data is available, deliver the post-processed point cloud in an ASCII file containing XYZIRGB (X, Y, Z coordinates, return intensity (I), and RGB color values).

Figure 7-1 shows an example of a point cloud dataset and the same data converted into a CADD model.

Figure 7-1 Sample Point Cloud Dataset and Same Data Converted to CADD



In addition, provide a readme file specifying the units and datum of the XYZ coordinates in ASCII point cloud files and deliver the georeferenced image files (in common image format such as jpg, tiff, png) if available.

Deliverables specific to MCLM surveys may include, but are not limited to:

- XYZI files in ASCII, CSV, or other specified format
- Registered point clouds
- Digital video or photo mosaic files
- Survey narrative report and QA/QC files

7.4.1 Raw data formats.

Raw data formats from mobile systems are generally in a proprietary format that can be exported into any portable format. Points can be processed into a classified point cloud stored in a LAS format (Log ASCII Standard, a commonly accepted public file format for storing and exchanging LIDAR point data).

7.4.2 Modeled data.

Raw data from MCLM surveys are generally too cumbersome for client processing. Therefore, most vendors provide CADD or mesh model representations based on the raw data suitable for survey grade analysis. The maximum resolution of such models is around 200 percent of the survey resolution; however, accuracy may exceed or at least approach the measurement accuracy of the MCLM instrumentation. The latter requires sufficient laser spot overlap to generate models with minimum error.

7.4.3 Survey Properties.

Document a mobile scanning project to show a clear data lineage from the published primary control to final deliverables. The data path of the entire process must be defined, documented, assessable, and allow for identifying adjustment or modification. 3D data without a documented lineage is susceptible to embedded mistakes. The survey narrative report, completed by the Party Chief, contains the following general information, the specific information required by each survey method, and any appropriate supplemental information.

- Project name and identification
- Survey date, limits, and purpose
- Datum, epoch, and units
- Control found, held, and set for the survey
- Personnel, equipment, and surveying methods used
- Problems encountered
- Any other pertinent information such as GNSS observation logs

- Dated signature and seal of the Party Chief

7.4.4 Documentation.

Documentation for mobile terrestrial laser scanning surveys includes, but is not limited to:

7.4.4.1 **Adjustment Report.**

7.4.4.2 **Control for Scanner Registration.**

1. Control target points
2. Local transformation points
3. Validation points
4. GNSS Accuracy Report
5. IMU Accuracy Report
6. Adjustment report for control

7.4.4.3 **Registration Reports.**

1. Results of target and cloud to cloud registration
2. QA/QC reports
3. Results of finished products to validation points
4. MCLM survey narrative report

7.5 **What are the deliverables for projects incorporating LIDAR TLM technologies?**

Identify deliverables for scanning projects in the planning stage. However, TLM systems produce a great deal of data that may prove useful beyond the expected outcomes. In this context, send the FAA the raw data, registered data, filtered and processed data, and all related acquisition data including field notes and digital photographs. Additional deliverables should include QA/QC analysis, metadata, and useful definitions of procedures.

7.5.1 Deliverables Specific to TLM Surveys.

Deliverables specific to TLM surveys may include, but are not limited to:

- XYZI or XYZ RGB files in ASCII, CSV, XML, or other specified format. List all control points and targets with their coordinates and elevations, coordinate system, and units.
- Registered point clouds
- Current roadway design software files
- Current drafting software files

- Digital photo mosaic files
- 3D printing technology physical scale models of the subject
- Survey narrative report and QA/QC files
- FGDC-compliant metadata files conforming to the current FAA standard

7.5.2 Raw data formats.

Most scanners export data in proprietary, compressed data formats and are only useful if the FAA can access these files. Because they are small, these raw data files are useful for archiving the data from any TLM survey. The most portable format (but least efficient) is ASCII files. These files will generally contain three to seven columns of data and include XYZ point position data, intensity (I), and RGB color assignments. Unlike airborne terrestrial LIDAR mapping, TLM data does not usually collect full waveform data collection. In those projects where TLM, full wave form technology is used, the raw point cloud should include:

- All returns, all collected points, fully calibrated and adjusted to ground, by swath
- Fully compliant LAS V1.2 or V1.3, Point Record Format 1, 3, 4, or 5
- LAS V1.3 deliverables with waveform data are to use external “auxiliary” files with the extension “.wdp” for the storage of waveform packet data. See the LAS V1.3 Specification for additional information.
- Georeference information included in all LAS file headers
- GPS times recorded as Adjusted GPS Time, at a precision sufficient to allow unique timestamps for each pulse
- Intensity values (native radiometric resolution)
- One file per swath, file size not to exceed 2 GB

Newer systems provide the option to collect the full waveform, which results in raw data formats similar to ATLM or MCLM systems.

7.5.3 Modeled data.

Similar to MCLM systems, TLM produces more data than most clients are prepared to computationally handle. Therefore, generating modeled meshes suitable for display in CADD systems or geographic information systems is a common data deliverable. In most cases, with sufficient sampling density and spot overlap, the modeled surface data can be more accurate than the original LIDAR data.

CHAPTER 8. DATA REVIEW AND ACCEPTANCE

8.1 Data Review and Acceptance Requirement

The FAA Airport Surveying-GIS Program Manager (see contact list at https://www.faa.gov/about/office_org/headquarters_offices/arp/offices/aas/aas100/) is responsible for conducting the data validation and verification of work products performed in accordance with this advisory circular.

8.1.1 Review Process.

8.1.1.1 Receipt Acknowledgment.

The FAA will acknowledge the date the imagery and associated deliverables were received in the Airports GIS project file within two working days of receipt. They will include a note in the comments section of the project when they expect to start and finish the review for the information collected from the FAA, contractor and airport sponsor.

8.1.1.2 Imagery Acceptance Criteria.

The imagery will be evaluated by the criteria listed below:

- Ground Sample Distance (GSD). Between 0.25 feet and 1.0 feet (7.5 – 30 centimeters).
- Stereo Coverage. Imagery must have sufficient overlap to permit stereo coverage of the entire area for analysis.
- Geometric Fidelity. Collection and processing of the image data will maintain, within accuracy requirements, the relationship between measurements made in the image model and real world coordinates.
- Georeferencing. The imagery is georeferenced and the source data used for completing the georeferencing is provided.
- Positional Accuracy. As a minimum, positions of well-defined points determined from the stereo imagery must be within one meter relative to the NSRS referenced to North American Datum of 1983 (NAD83) and the North American Vertical Datum of 1988 (NAVD88) at the 95 percent confidence level for Easting, Northing and Orthometric Height. Positional accuracies must meet minimum requirements but should ultimately meet project defined positional accuracy requirements.
- Resolution. Imagery must be sufficiently sharp to allow identification, analysis, and measurement of airport features and obstructions.
- Image Quality. Imagery must be clear, sharp, and evenly exposed across the format. The imagery must be free from clouds, cloud

shadows, smoke, haze, scratches, and other blemishes interfering with the intended use of the imagery.

- Acquisition Date. Imagery should be acquired within the 6-month period prior to the airport ground survey unless authorized by FAA (see paragraph 3.1.).
- Foliage. Imagery should be collected at time of full leaf foliage if appropriate.

8.1.1.3 **Imagery Acceptance Review.**

The FAA will formally accept the imagery by providing an imagery usability report in the project file of Airport GIS. This review typically is completed within five days of the start date, but could take longer depending on workload. Once a favorable usability determination is made by the FAA, submit the airport ground survey data. If the FAA determines the imagery is unacceptable, re-submit new imagery as soon as possible for review. This is the primary reason for submitting the imagery well in advance of the airport ground survey portion.

CHAPTER 9. POINTS OF CONTACT

9.1 Advisory Circular Questions/Comments

Contact the following offices for additional information or clarification.

- For questions regarding these specifications, contact:
FAA Airport Surveying-GIS Program Manager
Email: 9-AWA-ARP-AirportSurveyingGIS@faa.gov
- For questions regarding imagery review, usability, or review times, contact:
FAA Airport Engineering Division (AAS-100)
Airport Surveying-GIS Program Manager
800 Independence Avenue, Suite 621
Washington DC, 20591
Front Office Phone: 202-267-7669

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APPENDIX A. LIDAR USABILITY TABLE

- A.1 The following table presents each of the features within AC 150/5300-18 and defines the usability of the different LIDAR sensors to capture the information. The table also provides any special capture requirements for the collection of each feature by sensor.
- A.2 For example, in the first entry, it is not acceptable to use ATLM to define this feature. However, by adding reference targets, it is acceptable to use MCLM and TLM sensors as a capture method.
- A.3 “Requires a reference target on” means the data provider must locate a reference target at or on the point specified.
- A.4 “Requires interpretation from data” means the data provider uses the data to identify, classify and geolocate an object.
- A.5 In this table, the abbreviation “NA” indicates that using LIDAR to capture the feature using the sensor is not acceptable.

Table A-1. Sensor Specific Capture Requirements

Feature Group	Feature Name	ATLM	MCLM	TLM
Airfield	AirfieldGateStand	NA	Requires a LIDAR reference target	Requires a LIDAR reference target
Airfield	AircraftNonMovementArea	NA	Requires a LIDAR reference target at line endpoints	Requires a LIDAR reference target at line endpoints
Airfield	AirOperationsArea	Requires a LIDAR reference target on vertices	Requires a LIDAR reference target on vertices	Requires a LIDAR reference target on vertices
Airfield	AirfieldLight	Requires a LIDAR reference target	Acceptable	Acceptable
Airfield	ArrestingGear	NA	NA	NA

Feature Group	Feature Name	ATLM	MCLM	TLM
Airfield	FrequencyArea	Requires a LIDAR reference target on vertices	Requires a LIDAR reference target on vertices	Requires a LIDAR reference target on vertices
Airfield	PassengerLoadingBridge	Requires Multiple Look Angles	Requires Multiple Look Angles	Requires Multiple Look Angles
Airfield	RunwayCenterline	NA	Requires a LIDAR Reference Target at line endpoints	Requires a LIDAR Reference Target at line endpoints
Airfield	RunwayHelipadDesignSurface	NA	NA	NA
Airfield	RunwayIntersection	Acceptable	Requires a LIDAR reference target at line endpoints	Requires a LIDAR Reference Target at line endpoints
Airfield	RunwayLAHSO	Acceptable	Acceptable	Acceptable
Airfield	RunwayElement	Acceptable	Acceptable	Acceptable
Airfield	Stopway	Acceptable	Acceptable	Acceptable
Airfield	TaxiwayHoldingPosition	Requires a LIDAR reference target at line endpoints	Requires a LIDAR reference target at line endpoints	Requires a LIDAR reference target at line endpoints
Airfield	AirportSign	Requires a LIDAR reference target	Requires a LIDAR reference target	Requires a LIDAR reference target
Airfield	Apron	Requires interpretation from data	Requires interpretation from data	Requires interpretation from data
Airfield	DeicingArea	Requires LIDAR reference target on vertices	Requires LIDAR reference target on vertices	Requires LIDAR reference target on vertices and sufficient view angle

Feature Group	Feature Name	ATLM	MCLM	TLM
Airfield	TouchDownLiftOff	NA	Requires interpretation from data	Requires interpretation from data
Airfield	MarkingArea	Requires interpretation from data	Requires interpretation from data	Requires interpretation from data
Airfield	MarkingLine	Requires interpretation from data	Requires interpretation from data	Requires interpretation from data
Airfield	MovementArea	Requires LIDAR reference target on vertices	Requires LIDAR reference target on vertices	Requires LIDAR reference target on vertices
Airfield	Runway	Requires interpretation from data	Requires interpretation from data	Requires interpretation from data
Airfield	RestrictedAccessBoundary	Requires LIDAR reference targets on line endpoints and interpretation from data	Requires LIDAR reference targets on line endpoints and interpretation from data	Requires LIDAR reference targets on endpoints and interpretation from data
Airfield	RunwayArrestingArea	Interpreted from data	Requires multiple look angles and interpretation from data	Requires multiple look angles and interpretation from data
Airfield	RunwayBlastPad	Requires interpretation from data	Requires interpretation from data	Requires interpretation from data
Airfield	RunwayEnd	Not acceptable	Requires LIDAR reference target	Requires LIDAR reference target
Airfield	RunwayLabel	Requires LIDAR reference target at location	Requires LIDAR reference target at location	Requires LIDAR reference target at location

Feature Group	Feature Name	ATLM	MCLM	TLM
Airfield	RunwaySafetyAreaBoundary	Requires LIDAR reference targets on vertices	Requires LIDAR reference target on vertices	Requires LIDAR reference target on vertices
Airfield	Shoulder	Requires interpretation from data	Requires Interpretation from data	Requires interpretation from data
Airfield	TaxiwayIntersection	Requires interpretation from data	Requires interpretation from data	Requires sufficient look angle and interpretation from data
Airfield	TaxiwayElement	Requires interpretation from data	Requires interpretation from data	Requires sufficient look angle and interpretation from data
Airspace	LandmarkSegment	Requires interpretation from data	Requires interpretation from data	Requires multiple look angles
Airspace	Obstacle	Requires complimentary imagery, sufficient look angle and interpretation from data	Requires complimentary imagery and interpretation from data	Requires complimentary imagery and interpretation from data
Airspace	ObstructionArea	Requires complimentary imagery, sufficient look angle and interpretation from data	Requires complimentary imagery, sufficient look angle and interpretation from data	Requires complimentary imagery, sufficient look angle and interpretation from data
Airspace	ObstructionIdSurface	NA	NA	NA
Airspace	RunwayProtectArea	NA	NA	NA
Cadastral	AirportBoundary	NA	NA	NA
Cadastral	AirportParcel	NA	NA	NA
Cadastral	County	NA	NA	NA

Feature Group	Feature Name	ATLM	MCLM	TLM
Cadastral	EasementsAndRightsofWay	NA	NA	NA
Cadastral	FAARegionArea	NA	NA	NA
Cadastral	LandUse	NA	NA	NA
Cadastral	LeaseZone	NA	NA	NA
Cadastral	Municipality	NA	NA	NA
Cadastral	Parcel	NA	NA	NA
Cadastral	State	NA	NA	NA
Cadastral	Zoning	NA	NA	NA
Environmental	EnvironmentalContaminationArea	NA	NA	NA
Environmental	FaunaHazardArea	NA	NA	NA
Environmental	Floodzone	NA	NA	NA
Environmental	FloraSpeciesSite	Requires LIDAR reference targets on vertices	Requires LIDAR reference targets on vertices	Requires LIDAR reference targets on vertices
Environmental	ForestStandArea	Requires LIDAR reference targets on vertices	Requires LIDAR reference targets on vertices	Requires LIDAR reference targets on vertices
Environmental	HazardousMaterialStorageSite	NA	NA	NA
Environmental	NoiseContour	NA	NA	NA
Environmental	NoiseIncident	NA	NA	NA
Environmental	NoiseMonitoringPoint	NA	NA	NA
Environmental	SampleCollectionPoint	NA	NA	NA
Environmental	Shoreline	Requires interpretation from data	Requires interpretation from data	Requires interpretation from data
Environmental	Wetland	NA	NA	NA

Feature Group	Feature Name	ATLM	MCLM	TLM
Geospatial	AirportControlPoint ¹	Requires LIDAR reference target on point and interpretation from data	Requires LIDAR reference target on point and interpretation from data	Requires LIDAR reference target on point and interpretation from data
Geospatial	CoordinateGridArea	NA	NA	NA
Geospatial	ElevationContour	Requires interpretation from data	Requires interpretation from data	Requires interpretation from data
Geospatial	ImageArea	NA	NA	NA
Manmade Structures	Building	Requires interpretation from data	Requires multiple look angles and interpretation from data	Requires multiple look angles and interpretation from data
Manmade Structures	ConstructionArea	NA	NA	NA
Manmade Structures	Roof	Requires interpretation from data	Requires multiple look angles and interpretation from data	Requires multiple look angles and interpretation from data
Manmade Structures	Fence	NA	Requires multiple look angles	Requires multiple look angles
Manmade Structures	Gate	Requires interpretation from data	Requires interpretation from data	Requires interpretation from data
Manmade Structures	Tower ²	Interpreted from data	Interpreted from data	Interpreted from data
NavigationalAids	NavaidCriticalArea	NA	NA	NA
NavigationalAids	NavaidEquipment - Airport Beacon	Requires interpretation from data	Requires interpretation from data	Requires interpretation from data

¹ Exception: Data providers may use any LIDAR sensor without targets in modeling the runway profile points, however it does require data interpretation.

² Data providers should use caution in collecting towers using LIDAR. Very tall and slender towers are difficult to capture using LIDAR. If used to support obstacle analysis, the data provider must use complimentary imagery to support the analysis.

Feature Group	Feature Name	ATLM	MCLM	TLM
NavigationalAids	NavaidEquipment - Air Route Surveillance Radar (ARSR) or Airport Surveillance Radar (ASR)	Requires interpretation from data	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NavaidEquipment - Approach Light System ³	NA	Requires LIDAR reference targets at line endpoints	Requires LIDAR reference targets at line endpoints
NavigationalAids	NavaidEquipment - Back Course Marker (BCM)	NA	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NavaidEquipment - Distance Measuring Equipment (DME)	NA	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NavaidEquipment - Glideslope - End Fire (GS-EF)	Requires a LIDAR reference target on each end of each antenna array and interpretation from data	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NavaidEquipment - Fan Marker (FM)	NA	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NavaidEquipment - Glideslope (GS)	NA	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NavaidEquipment - ground controlled approach touchdown reflectors	NA	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NavaidEquipment - Inner Marker	NA	Requires interpretation from data	Requires interpretation from data

³ A reference target is required on each end of each light bar or individual fixture to adequately capture this feature.

Feature Group	Feature Name	ATLM	MCLM	TLM
NavigationalAids	NavaidEquipment - Localizer (LOC)	Requires a LIDAR reference target on each end of the antenna array, horizontal survey point and interpretation from data	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NavaidEquipment - Localizer Type Directional Aid (LDA)	NA, insufficient resolution capabilities	Sufficient resolution, interpreted from data	Sufficient resolution, interpreted from data
NavigationalAids	NavaidEquipment - Middle Marker (MM)	NA	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NavaidEquipment - MLS Azimuth Antenna (MLSAZ)	NA	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NavaidEquipment - MLS Elevation Antenna (MLSEA)	NA	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NavaidEquipment - Non-Directional Beacon (NDB)	NA	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NavaidEquipment - Outer Marker (OM)	NA	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NavaidEquipment - Precision Approach Path Indicator (PAPI) System	Requires interpretation from data	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NavaidEquipment - Precision Approach Radar (PAR) Touchdown Reflectors	NA	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NavaidEquipment - Pulse Light Approach Slope Indicator (PLASI)	Requires interpretation from data	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NavaidEquipment - Pulsating Visual Approach Slope Indicator (PVASI)	Requires interpretation from data	Requires interpretation from data	Requires interpretation from data

Feature Group	Feature Name	ATLM	MCLM	TLM
NavigationalAids	NavaidEquipment - Runway End Identifier Lights (REIL)	NA	Requires interpretation of data	Requires interpretation of data
NavigationalAids	NavaidEquipment - Simplified Directional Facility (SDF)	NA	Requires interpretation of data	Requires interpretation of data
NavigationalAids	NavaidEquipment - Tactical Air Navigation (TACAN)	Requires interpretation from data	Requires interpretation of data	Requires interpretation of data
NavigationalAids	NavaidEquipment - Tricolor Visual Approach Slope Indicator System (TRCV)	NA	Requires interpretation of data	Requires interpretation of data
NavigationalAids	NavaidEquipment - “T” Visual Approach Slope Indicator System (T-VASI)	Requires interpretation of data	Requires interpretation of data	Requires interpretation of data
NavigationalAids	NavaidEquipment - VHF Omni Directional Range (VOR)	Requires interpretation from data	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NavaidEquipment - Visual Approach Slope Indicator System (VASI)	Requires LIDAR reference target at horizontal reference point and interpretation from data	Requires LIDAR reference target at horizontal reference point and interpretation from data	Requires LIDAR reference target at horizontal reference point and interpretation from data
NavigationalAids	NavaidEquipment - VOR/TACAN (VORTAC)	Requires interpretation from data	Requires interpretation from data	Requires interpretation from data
NavigationalAids	NAVAIDSite	Requires LIDAR reference targets on vertices	Requires LIDAR reference targets on vertices	Requires LIDAR reference targets on vertices
SeaPlane	WaterOperatingArea	NA	NA	NA
SeaPlane	WaterLaneEnd	NA	NA	NA
SeaPlane	TaxiChannel	NA	NA	NA
SeaPlane	TurningBasin	NA	NA	NA
SeaPlane	NavigationBuoy	NA	NA	NA

Feature Group	Feature Name	ATLM	MCLM	TLM
SeaPlane	SeaplaneRampCenterline	Requires interpretation from data	Requires interpretation from data	Requires sufficient look angle and interpretation from data
SeaPlane	SeaplaneRampSite	Requires interpretation from data	Requires interpretation from data	Requires sufficient look angle and interpretation from data
SeaPlane	DockArea	Requires interpretation from data	Requires multiple look angles	Requires multiple look angles
SeaPlane	AnchorageArea	Water/buoy	Water/buoy	Water/buoy
Security	SecurityArea	NA	NA	NA
Security	SecurityIdDisplayArea	NA	NA	NA
Security	SecurityPerimeterLine	NA	NA	NA
Security	SterileArea	NA	NA	NA
Surface Transportation	Bridge	NA	Requires interpretation from data	Requires interpretation from data
Surface Transportation	DrivewayArea	Requires LIDAR reference targets on vertices	Requires LIDAR reference targets on vertices or interpretation from data	Requires sufficient look angle, LIDAR reference targets on vertices, or interpretation from data
Surface Transportation	DrivewayCenterline	NA	NA	NA
Surface Transportation	ParkingLot	Requires LIDAR reference targets on vertices	Requires LIDAR reference targets on vertices or interpretation from data	Requires sufficient look angle, LIDAR reference targets on vertices, or interpretation from data

Feature Group	Feature Name	ATLM	MCLM	TLM
Surface Transportation	RailroadCenterline	Requires interpretation from data	Requires interpretation from data	Requires sufficient look angle and interpretation from data
Surface Transportation	RailroadYard	Requires interpretation from data	Requires multiple look angles	Requires multiple look angles
Surface Transportation	RoadCenterline	Requires interpretation from data	Requires interpretation from data	Requires interpretation from data
Surface Transportation	RoadPoint	Requires LIDAR reference target on point	Requires LIDAR reference target on point	Requires LIDAR reference target on point
Surface Transportation	RoadSegment	Requires interpretation from data	Requires interpretation from data	Requires sufficient look angle and interpretation from data
Surface Transportation	Sidewalk	Requires interpretation from data	Requires interpretation from data	Requires sufficient look angle and interpretation from data
Surface Transportation	Tunnel	NA	NA	NA
Utilities	TankSite	Requires interpretation from data	Requires interpretation from data	Requires interpretation from data
Utilities	UtilityLineA	NA	NA	NA
	UtilityLineB	NA	NA	NA
	UtilityLineC	NA	NA	NA
	UtilityLineD	NA	NA	NA
Utilities	UtilityPointA	NA	NA	NA
	UtilityPointB	NA	NA	NA
	UtilityPointC	NA	NA	NA
	UtilityPointD	NA	NA	NA

Feature Group	Feature Name	ATLM	MCLM	TLM
Utilities	UtilityPolygonA	NA	NA	NA
	UtilityPolygonB	NA	NA	NA
	UtilityPolygonC	NA	NA	NA
	UtilityPolygonD	NA	NA	NA

APPENDIX B. GLOSSARY

The following terms are used in this AC and throughout the FAA.

Term	Definition
3D Imaging System	A non-contact measurement instrument used to produce a 3D representation (for example, a point cloud) of an object or site. (ASTM E 2544-09b)
AC	Advisory Circular
Accuracy	The degree of conformity with a standard, or a value accepted as correct. Precision is the degree of uniformity of repeated measurements or events. For example, repeat measurements of the distance between two points may exhibit a high degree of precision by virtue of the relative uniformity of the measurements. However, if a “short” tape were used in the measurements, accuracy would be poor in that the measured distance would not conform to the true distance between the points. Surveying and mapping accuracy standards should include three elements: (1) a stated variation from a true value or a value accepted as correct, (2) the point to which the new value is relative, and (3) the probability that the new value will be within the stated variation. For example, “Horizontal accuracy will be 10 centimeters relative to the nearest Continuously Operating Reference Station (CORS) at the 95 percent confidence level.” (AC 150/5300-18)
ADO	Airport District Offices
AGL	Above Ground Level, flying height
AIP	Airport Improvement Program
ALM	Airborne LIDAR Mapping
AP	Aerial Photogrammetric
ATLM	Airborne Terrestrial LIDAR mapping
AWAR	Area-Weighted Average Resolution

Term	Definition
Beam Diameter ($d\sigma$)	<p>For a laser beam with a circular irradiance pattern, the beam diameter is the extent of the irradiance distribution in a cross-section of the laser beam (in a plane orthogonal to its propagation path) at a distance z and is given by:</p> $d_{\sigma}(z) = 4\sigma(z)$ <p>where:</p> $\sigma(z) = \sigma_x(z) = \sigma_y(z)$ <p>$\sigma_x(z), \sigma_y(z)$ = the square roots of the second order moments</p> <p>For a laser beam with a Gaussian distribution of irradiance, the beam diameter is often defined as the distance across the center of the beam for which the irradiance, I equals $1/e^2$ of the maximum irradiance (where e is the base of the natural logarithm. The area inside a circle with this diameter and centered at the beam center will contain 86.5 percent of the total beam irradiance. (ASTM E 2544-09b)</p>
Beam Divergence Angles ($\theta_{\sigma x}, \theta_{\sigma y}$)	<p>Measure for the asymptotic increase of the beam widths, $d_{\sigma x}(z)$ and $d_{\sigma y}(z)$, with increasing distance, z, from the beam waist locations, z_{0x} and z_{0y}, given by:</p> $\theta_{\sigma x} = (z - z_{0x}^{lim}) \rightarrow \infty \frac{d_{\sigma x}(z)}{z - z_{0x}}$ $\theta_{\sigma y} = (z - z_{0y}^{lim}) \rightarrow \infty \frac{d_{\sigma y}(z)}{z - z_{0y}}$ <p>(ASTM E 2544-09b)</p>
Bench Mark	A relatively permanent natural or artificial material object bearing a marked point whose elevation above or below an adopted surface (datum) is known. (AC 150/5300-18)
CADD	Computer-Aided Drafting and Design
CN	Color Negative film
Consolidated Vertical Accuracy	<p>The result of a test of the accuracy of 40 or more check points (z-values) consolidated for two or more of the major land cover categories, representing both the open terrain and other land cover categories. Computed using a nonparametric testing method (95th Percentile), a consolidated vertical accuracy is always accompanied by a fundamental vertical accuracy. (Refer to the sections “Fundamental Accuracy” and “Supplemental and Consolidated Vertical Accuracies” in <i>Guidelines for Digital Elevation Data</i> available at https://nationalmap.gov/standards/.)</p>
Control Station	A point on the ground whose position and/or elevation is used as a basis for obtaining positions and/or elevations of other points. (AC 150/5300-18)

Term	Definition
Datum	In general, a point, line, surface, or set of values used as a reference. A “geodetic datum” is a set of constants specifying the coordinate system and reference used for geodetic control (refer to Control Station), i.e. for calculating coordinates of points on the earth. At least eight constants are needed to form a complete datum: three to specify the location of the origin of the coordinate system; three to specify the orientation of the coordinate system; and two to specify the dimensions of the reference ellipsoid. Any point has a unique X, Y, Z datum coordinate which can be transformed into latitude, longitude, and ellipsoid height (height relative to the ellipsoid). A “horizontal control datum” is a geodetic datum specified by two coordinates (latitude and longitude) on the ellipsoid surface, to which horizontal control points are referenced. A “vertical datum” is a theoretical equipotential surface with an assigned value of zero to which elevations are referenced. (Refer to geoid.) (AC 150/5300-18)
DEM	Digital Elevation Model. The digital cartographic representation of the elevation of the land at regularly spaced intervals in x and y directions, using z-values referenced to a common vertical datum. (Refer to <i>Guidelines for Digital Elevation Data</i> available at https://nationalmap.gov/standards/ .)
DSM	Digital Surface Model. Similar to DEM or DTM, except that they depict the elevations of the top surfaces of buildings, trees, towers, and other features elevated above the bare earth. DSM are especially relevant for telecommunications management, forest management, air safety, 3D modeling and simulation. (Refer to <i>Guidelines for Digital Elevation Data</i> available at https://nationalmap.gov/standards/ .)
DTM	Digital Terrain Model. DTM may be identical to DEM, but they may also incorporate the elevation of significant topographic features on the land and change points and break lines that are irregularly spaced so as to better characterize the true shape of the bare earth terrain. The net result of DTM is that the distinctive terrain features are more clearly defined, and contours generated from DTM more closely approximate the real shape of the terrain. Such DTM are normally more expensive and time consuming to produce than uniformly spaced DEM because break lines are ill suited for automation; but the DTM results are technically superior to standard DEM for many applications. (Refer to <i>Guidelines for Digital Elevation Data</i> available at https://nationalmap.gov/standards/ .)
Ellipsoid Height	The distance between a point and the reference ellipsoid taken along the perpendicular to the ellipsoid. Ellipsoid heights are the heights resulting from GPS observations. Ellipsoid heights are positive if the point is above the ellipsoid. Ellipsoid Height = Geoid Height + Orthometric Height. (AC 150/5300-18)
FAA	Federal Aviation Administration

Term	Definition
Feature	A manmade or natural object that appears in the real world such as a building, runway, navigational aid or river. (AC 150/5300-18)
Feature Type	A collection of all features of a given type such as all runways or all buildings. Feature Types are analogous to layers in many GIS applications and are also referred to as Entity Types and Feature Classes in other standards. (AC 150/5300-18)
Feature Instance	A specific feature such as runway 10/28 at Baltimore Washington International Airport. (AC 150/5300-18)
FGDC	Federal Geographic Data Committee
First Return	For a given emitted pulse, it is the first reflected signal that is detected by a 3D imaging system, time-of-flight type (TOF), for a given sampling position, that is, azimuth and elevation angle. (ASTM E 2544-09b)
FOV	Field of View
Fundamental Vertical Accuracy	The fundamental vertical accuracy is the value by which vertical accuracy can be equitably assessed and compared among datasets. The fundamental vertical accuracy of a dataset must be determined with check points located only in open terrain where there is a very high probability that the sensor will have detected the ground surface. It is obtained utilizing standard tests for RMSE. (Refer to Consolidated and Supplemental Vertical Accuracies.) (Refer to <i>Guidelines for Digital Elevation Data</i> available at https://nationalmap.gov/standards/ .)
GBL	Ground Based LIDAR
Geoid	The theoretical surface of the earth that coincides everywhere with approximate mean sea-level. The geoid is an equipotential surface to which, at every point, the plumb line is perpendicular. Because of local disturbances of gravity, the geoid is irregular in shape. (AC 150/5300-18)
Geoid Height	The distance, taken along a perpendicular to the reference ellipsoid, between the reference ellipsoid and the geoid. The geoid height is positive if the geoid is above the reference ellipsoid. (Geoid height is negative for the conterminous United States.) Geoid Height = Ellipsoidal Height - Orthometric Height. (AC 150/5300-18)
GNSS	Global Navigation Satellite System
GPS	Global Positioning System
GSD	Ground Sampled Distance
IMU	Inertial Measurement Unit

Term	Definition
Independent Source of Higher Accuracy	Data acquired independently of procedures to generate the dataset that is used to test the positional accuracy of a dataset. The independent source of higher accuracy will be of the highest accuracy feasible and practicable to evaluate the accuracy of the dataset. (Refer to <i>Guidelines for Digital Elevation Data</i> available at https://nationalmap.gov/standards/ .)
KML	Keyhole Markup Language
LAS	LIDAR Data Exchange File
Last Return	For a given emitted pulse, it is the last reflected signal that is detected by a 3D imaging system, time-of-flight type (TOF), for a given sampling position, that is, azimuth and elevation angle. (ASTM E 2544-09b)
LIDAR	Light Detection and Ranging. An instrument that measures distance to a reflecting object by emitting timed pulses of light and measuring the time between emission and reception of reflected pulses. The measured time interval is converted to distance. (Refer to <i>Guidelines for Digital Elevation Data</i> available at https://nationalmap.gov/standards/ .)
Local Control	A control station or network of control stations in a local area used for referencing local surveys. Local control may or may not be tied to the National Spatial Reference System. (See Control Station) (<u>AC 150/5300-18</u>)
MCLM	Mobile Compensated LIDAR Mapping
Metadata	Information about the data itself such as source, accuracy, dates for which the data are valid, security classification, etc. Metadata is essential in helping users determine the extent on which they can rely on a given data item to make decisions. (<u>AC 150/5300-18</u>)
Multiple Returns	For a given emitted pulse, a laser beam hitting multiple objects separated in range is split and multiple signals are returned and detected. (ASTM E 2544-09b)
NAD	North American Datum
NAVD	North American Vertical Datum
NDEP	National Digital Elevation Program
NGS	National Geodetic Survey
NOAA	National Oceanic and Atmospheric Administration

Term	Definition
NSRS	National Spatial Reference System. A network of permanent survey monuments located throughout the United States with accurately determined positions (horizontal network) and/or elevations (vertical network). Gravity values, not always monumented, are also part of NSRS. Responsibility for establishing and maintaining NSRS rests with the National Geodetic Survey under the U.S. Department of Commerce. Current authority is contained in United States Code, Title 33, USC 883a as amended, and specifically defined by Executive Directive, Bureau of the Budget (now Office of Management and Budget) Circular No. A-16 Revised. (AC 150/5300-18)
NSSDA	National Standard for Spatial Data Accuracy
OIS	Obstruction Identification Surfaces
OPUS	Online Positioning User Service
Orthometric Height	The distance taken along the plumb line between a point and the geoid. Orthometric heights are positive if the point is above the geoid. Orthometric Height = Ellipsoid Height - geoid Height. (AC 150/5300-18)
QA/QC	Quality Assurance and Quality Control
PDF	Adobe's Portable Document Format
PDOP	Position Dilution of Precision
PFC	Passenger Facility Charges
Point Cloud	A collection of data points in 3D space (frequently in the hundreds of thousands), for example as obtained using a 3D imaging system. (ASTM E 2544-09b)
POS	Position Orientation System
Positional Accuracy	The difference between a geospatial feature's displayed position and its actual position. Absolute positional accuracy is the difference between a geospatial feature's displayed position and its actual position on the face of the earth. Relative positional accuracy is the difference between a geospatial feature's displayed position and that of other geospatial features in the same data set. (AC 150/5300-18)
Post Spacing	The smallest distance between two discrete points that can be explicitly represented in a gridded elevation dataset. It is important to note that features of a size equal to, or even greater than the post spacing, may not be detected or explicitly represented in a gridded model. For gridded elevation data, the horizontal post spacing may be referenced as the cell size, the grid spacing, the posting interval, or the ground sample distance. Horizontal post spacing should be documented in the metadata file. (Refer to <i>Guidelines for Digital Elevation Data</i> available at https://nationalmap.gov/standards/ .)

Term	Definition
Precision	The smallest separation that can be represented by the method employed to make the positional statement which is the number of units or digits to which a measured or calculated value is expressed and used. (AC 150/5300-18)
PRF	Pulse Repetition Frequency
Reference Ellipsoid	A geometric figure comprising one component of a geodetic datum, usually determined by rotating an ellipse about its shorter (polar) axis, and used as a surface of reference for geodetic surveys. The reference ellipsoid closely approximates the dimensions of the geoid. Certain ellipsoids fit the geoid more closely for various areas of the earth. Elevations derived directly from satellite observations are relative to the ellipsoid and are called ellipsoid heights. (AC 150/5300-18)
Registration	The process of determining and applying to two or more datasets the transformations that locate each dataset in a common coordinate system so that the datasets are aligned relative to each other. (ASTM E 2544-09b)
RGB	Red, Green, Blue
RMSE	Root Mean Square Error. The square root of the mean of squared errors for a sample. (Refer to <i>Guidelines for Digital Elevation Data</i> available at https://nationalmap.gov/standards/ .)
RTK	Real Time Kinematic satellite navigation
Second Order Moments (σ_x^2 , σ_y^2)	<p>The second order moments of an irradiance distribution of a simple astigmatic laser beam at a given range, z, $\sigma_x^2(z)$, $\sigma_y^2(z)$, along the principle axes, x and y, are defined as</p> $\sigma_x^2(z) = \frac{\int_{-\infty}^0 \int_{-\infty}^0 I(x, y, z) (x - \bar{x})^2 dx dy}{\int_{-\infty}^0 \int_{-\infty}^0 I(x, y, z) dx dy}$ $\sigma_y^2(z) = \frac{\int_{-\infty}^0 \int_{-\infty}^0 I(x, y, z) (y - \bar{y})^2 dx dy}{\int_{-\infty}^0 \int_{-\infty}^0 I(x, y, z) dx dy}$ <p>Where $I(x, y, z)$ is the irradiance or optical power per unit area at point (x, y, z), and \bar{x} and \bar{y} are the coordinates of the centroid (also referred to as the first order moments) of the beam in the $x y$ plane, respectively. (ASTM E 2544-09b)</p>
Simple Astigmatic Beam	A beam having non-circular power density distributions and whose principle axes retain constant orientation under free propagation. (ASTM E 2544-09b)

Term	Definition
Supplemental Vertical Accuracy	The result of a test of the accuracy of z-values over areas with ground cover categories or combinations of categories other than open terrain. Obtained utilizing the 95th percentile method, a supplemental vertical accuracy is always accompanied by a fundamental vertical accuracy. (Refer to Fundamental and Consolidated Vertical Accuracies.)
TIFF	Tagged Image File Format
TLM	Terrestrial LIDAR Mapping
UoM	Unit of Measure
USGS	U.S. Geologic Survey
UTM	Universal Transverse Mercator
Well-Defined Point	A point that represents a feature for which the horizontal position is known to a high degree of accuracy and position with respect to the geodetic datum. (Refer to <i>Guidelines for Digital Elevation Data</i> available at https://nationalmap.gov/standards/ .)

Advisory Circular Feedback

If you find an error in this AC, have recommendations for improving it, or have suggestions for new items/subjects to be added, you may let us know by (1) mailing this form to Manager, Airport Engineering Division, Federal Aviation Administration ATTN: AAS-100, 800 Independence Avenue SW, Washington DC 20591 or (2) faxing it to the attention of the Office of Airport Safety and Standards at (202) 267-5383.

Subject: AC 150/5300-17C, Change 1

Date: _____

Please check all appropriate line items:

- ☐ An error (procedural or typographical) has been noted in paragraph _____ on page _____.
- ☐ Recommend paragraph _____ on page _____ be changed as follows:
- _____
- _____
- _____
- ☐ In a future change to this AC, please cover the following subject:
(Briefly describe what you want added.)
- _____
- _____
- _____
- ☐ Other comments:
- _____
- _____
- _____
- ☐ I would like to discuss the above. Please contact me at (phone number, email address).
- _____

Submitted by: _____

Date: _____

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